STATE OF CALIFORNIA GOODWIN J. KNIGHT GOVERNOR

221

vol 2

PUBLICATION OF STATE WATER RESOURCES BOARD

Bulletin No. 12

VENTURA COUNTY INVESTIGATION

Volume II APPENDIXES AND PLATES

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STATE OF CALIFORNIA GOODWIN J. KNIGHT GOVERNOR

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VENTURA COUNTY INVESTIGATION

Volume II APPENDIXES AND PLATES



October, 1953 Revised April, 1956

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AGREEMENT, AND ITS SUPPLEMENT, BETWEEN THE STATE WATER RESOURCES BOARD, THE COUNTY OF VENTURA, AND THE DEPARTMENT OF PUBLIC WORKS

MEMORANDA OF UNDERSTANDING BETWEEN THE DIVISION OF WATER RESOURCES, UNITED WATER CONSERVATION DISTRICT, AND THE VENTURA COUNTY FLOOD CONTROL DISTRICT

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APPENDIX A

AGREEMENT BETWEEN THE STATE WATER RESOURCES BOARD, THE VENTURA COUNTY FLOOD CONTROL DISTRICT, AND THE DEPARTMENT OF PUBLIC WORKS

THIS AGREEMENT, entered into as of the 15th day of April, 1951, by and between the State Water Resources Board, hereinafter referred to as the "Board", the Ventura County Flood Control District, hereinafter referred to as the "District", and the Department of Public Works, acting through the agency of the State Engineer, hereinafter referred to as the "State Engineer":

$\underline{W \ I \ T \ N \ E \ S \ S \ E \ T \ H}$

WHEREAS, by the State Water Resources Act of 1945, as amended, the Board is authorized to make investigations, studies, surveys, hold hearings, prepare plans and estimates, and make recommendations to the Legislature in regard to water development projects, including flood control plans and projects; and

WHEREAS, by said act, the State Engineer is authorized to cooperate with any county, city, state agency or public district on flood control and other water problems and when requested by any thereof may enter into a cooperative agreement to expend money on behalf of any thereof to accomplish the purposes of said act; and

WHEREAS, the District has requested the Board to enter into a cooperative agreement to conduct a comprehensive investigation of the water resources of Ventura County; and

WHEREAS, the State Engineer has reported to the Board that, as a result of a preliminary investigation it was concluded that emergency water problems exist in Ventura County and that a cooperative investigation is

warranted and within the scope and intent for which matching funds are allocated by the State Water Resources Board; and

WHEREAS, the Board has requested the State Engineer to cooperate in conducting a comprehensive investigation of the water resources of Ventura County and to formulate a report thereon;

NOW THEREFORE, in consideration of the premises and of the several promises to be performed by each as hereinafter set forth, the Board, the District, and the State Engineer do hereby mutually agree as follows:

ARTICLE I - WORK TO BE PERFORMED:

The work to be performed under this agreement shall consist of (1) a complete review of reports of prior investigations concerning the water resources of Ventura County; (2) field investigations and office studies to determine (a) the location, occurrence, and condition of water resources of the County, both surface and underground, (b) present water utilization including its nature, extent, and a survey of water service agencies, (c) ultimate water requirements, (d) preliminary general plans and estimates of cost for development and utilization of local water resources of the County to the maximum practicable extent, (e) required supplemental water supply from outside sources, (f) possible outside sources for required supplemental supply including preliminary plans for importation and estimates of costs; and (3) the formulation of a report thereon.

The work shall include the following:

(a) A plan for the development and utilization of the water resources of Sespe Creek, including features for their use in the Santa Clara River Valley and Oxnard Plain.

- (b) A plan for the development and utilization of the water resources of Piru Creek, including features for their use in the Santa Clara Valley and Oxnard Plain.
- (c) A plan for meeting the present and future needs of
 Calleguas Creek Basin either from Santa Clara River Basin or from other sources.
- (d) A plan for the development and utilization of the water resources of the Ventura River and tributaries including their use in the Ventura River Basin.

In connection with the study of dam and reservoir sites on streams of Ventura County, the following work shall be undertaken initially: (a) Devil's Canyon site on Piru Creck--water testing of dam foundation; (b) Topa Topa site on Sespe Creek--exploration of dam site and borrow areas; (c) Fillmore site on Sespe Creek--exploration of dam site and borrow areas and topographic survey of dam and reservoir sites.

The Board by this agreement authorizes and directs the State Engineer to cooperate by conducting said investigation and formulating said report and by otherwise advising and assisting in formulating solutions to the water problems in Ventura County.

During the progress of said investigation, all maps, plans, information, data and records pertaining thereto which are in the possession of any party hereto, shall be made fully available to any other party hereto for the due and proper accomplishments of the objectives hereof.

The work to be done under this agreement shall be diligently prosecuted with the objective of completing the investigation and report on or before June 30, 1953. A progress report containing findings on possible development of surface water supplies of Sespe and Piru Creeks shall be completed by February 1, 1952, or as soon thereafter as possible.

ARTICLE II - FUNDS:

On execution of this agreement, the District shall transmit the sum of Five Thousand Dollars (\$5,000) to the State Engineer for deposit, subject to the approval of the Director of Finance, into the Water Resources Revolving Fund in the State Treasury, for expenditure by the State Engineer in performance of the work provided for in this agreement. Also upon execution of this agreement, the Board shall request the Director of Finance to approve the transfer of the sum of Five Thousand Dollars (\$5,000) from funds appropriated to the Board by Item 257 of the Budget Act of 1950 to the said Water Resources Revolving Fund for expenditure by the State Engineer in performance of work provided for in this agreement.

On or before July 1, 1951, the District shall transmit the further sum of Fifteen Thousand Dollars (\$15,000) to the State Engineer for deposit, subject to the approval of the Director of Finance, into the Water Resources Revolving Fund in the State Treasury, for expenditure by the State Engineer in performance of the work provided for in this agreement. Following July 1, 1951, as soon as practicable the Board shall request the Director of Finance to approve the transfer of the further sum of Fifteen Thousand Dollars (\$15,000) to said Water Resources Revolving Fund from any funds which may be made available for such purposes, for expenditure by the State Engineer in performance of the work provided for in this agreement.

In the event that on or before July 1, 1952, the District shall transmit the sum of Ten Thousand Dollars (\$10,000) to the State Engineer for deposit, subject to the approval of the Director of Finance, into the Water Resources Revolving Fund in the State Treasury, for expenditure by the State Engineer in performance of the work provided for in this agreement,

thereupon as soon as practicable the Board shall request the Director of Finance to approve the transfer of the sum of Ten Thousand Dollars (\$10,000) to said Water Resources Revolving Fund from any funds which may be made available for such purposes, for expenditure by the State Engineer in performance of the work provided for in this agreement.

Of the funds made available under this agreement, not more than Ten Thousand Dollars (\$10,000) shall be expended on exploration work and surveys at dam and reservoir sites.

Notwithstanding anything contained in this agreement contrary hereto or in conflict herewith, this agreement is made contingent upon the funds being deposited in or transferred to the Water Resources Revolving Fund as provided herein for expenditure by the State Engineer in performance of the work provided for in this agreement. In the event any of the funds are not transferred to the Water Resources Revolving Fund by the Director of Finance as provided for herein within 30 days after the Board requests such transfer, this agreement shall terminate and the unexpended balance of any funds deposited by the County shall be returned, provided that neither the Board nor the State Engineer shall be obligated to the County for any portion of the funds already expended.

The Board and the State Engineer shall under no circumstances be obligated to expend for or on account of the work provided for under this agreement any amount in excess of the funds made available hereunder.

Upon completion and final payment for the work provided for in this agreement, the State Engineer shall furnish to the Board and to the County a statement of all expenditures made under this agreement. One-half of the total amount of all said expenditures shall be deducted from the sum advanced from funds appropriated to the Board and one-half of the total

amount of all said expenditures shall be deducted from the sum advanced by the County and any balance which may remain shall be returned to the Board and to the County in equal amounts.

Notwithstanding anything herein contained to the contrary, this agreement may be terminated and the provisions of this agreement may be altered, changed, or amended, by mutual consent of the parties hereto.

IN WITNESS WHEREOF, the parties hereunto have executed this agreement as of the date first herein written.

Approved as to Form:

/s/ Roy A. Gustafson Attorney, Ventura County Flood Control District

Approved as to Form and Procedure

/s/ Henry Holsinger Attorney for Division of Water Resources VENTURA COUNTY FLOOD CONTROL DISTRICT S E By /s/ R. E. Barrett Chairman, Board of Supervisors

STATE WATER RESOURCES BOARD

By /s/ Royal Miller Member

STATE OF CALIFORNIA DEPARTMENT OF PUBLIC WORKS

C. H. PURCELL

Director of Public Works

| By | | | | E |
|-----|-------|----|--------|---|
| /s/ | Frank | В. | Durkee | A |
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S

Frank B. Durkee Deputy Director

APPROVED:

/s/ James S. Dean Director of Finance /s/ A. D. Edmonston A. D. Edmonston State Engineer

| :Ī | E.J.R.:F.J.M.: : | |
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| : | Form :Budget:Value:Descript. | : |
| : | DEPARTMENT OF FINANCE | |
| : | APPROVED | : |
| : | May 7 1951 | : |

SUPPLEMENTAL AGREEMENT BETWEEN THE STATE WATER RESOURCES BOARD, THE VENTURA COUNTY FLOOD CONTROL DISTRICT, AND THE DEPARTMENT OF PUBLIC WORKS

THIS AGREEMENT, executed in quintuplicate, entered into as of May 1, 1952, by the State Water Resources Board, hereinafter referred to as the "Board"; the Ventura County Flood Control District, hereinafter referred to as the "District"; and the Department of Public Works of the State of California, acting through the agency of the State Engineer, hereinafter referred to as the "State Engineer".

<u>WITNESSETH</u>:

WHEREAS, by agreement heretofore entered into as of April 15, 1951, by and between the District, the Board, and the State Engineer, it was provided that the work to be performed by the State Engineer thereunder shall consist of (1) a complete review of reports of prior investigations concerning the water resources of Ventura County; (2) field investigations and office studies to determine (a) the location, occurrence, and condition of water resources of the County, both surface and underground, (b) present water utilization including its nature, extent, and a survey of water service agencies, (c) ultimate water requirements, (d) preliminary general plans and estimates of cost for development and utilization of local water resources of the County to the maximum practicable extent, (e) required supplemental water supply from outside sources, (f) possible outside sources for required supplemental supply including preliminary plans for importation and estimates of costs; and (3) the formulation of a report thereon; and

WHEREAS, under said agreement the District made available the sum of Five Thousand Dollars (\$5,000) and Fifteen Thousand Dollars (\$15,000) which were matched in equal amounts by the Board for expenditure by the State Engineer in the performance of the work provided for in said agreement; and

WHEREAS, it was the expressed intention in said agreement that at the commencement of the second year of said investigation the District would make available a further sum of Ten Thousand Dollars (\$10,000) subject to a matching or contribution in an equal sum by the Board for the completion of said investigation and report; and

WHEREAS, the funds provided for under said prior agreement, to which this agreement is supplemental, have been exhausted and additional funds are now required to complete said investigation and report, and it is the desire of the parties hereto that an additional sum of Twenty Thousand Dollars (\$20,000) shall be provided, Ten Thousand Dollars (\$10,000) by the District and Ten Thousand Dollars (\$10,000) by the Board;

NOW THEREFORE, in consideration of the premises and of the several promises to be faithfully performed by each as hereinafter set forth, the Board, the District, and the State Engineer do hereby mutually agree as follows:

1. The County, upon execution by it of this agreement, shall transmit to the State Engineer the sum of Ten Thousand Dollars (\$10,000) for deposit, subject to the approval of the Director of Finance, into the Water Resources Revolving Fund in the State Treasury for expenditure by the State Engineer in continuing performance of the work provided for in said prior agreement to which this agreement is supplemental.

2. Upon execution of this agreement by the Board, the Director of Finance will be requested to approve the transfer of the sum of Ten Thousand Dollars (\$10,000) from funds appropriated to the Board by Item 269 of the Budget Act of 1952 for expenditure by the State Engineer in continuing performance of the work provided for in said prior agreement to which this agreement is supplemental, and the State Controller will be requested to make such transfer.

3. The Board and the State Engineer shall under no circumstances be obligated to expend for or on account of the work provided for in said prior agreement to which this agreement is supplemental any amount in excess of the sum of Sixty Thousand Dollars (\$60,000) as made available under said prior agreement and this supplemental agreement and if funds are exhausted before completion of said work the Board and the State Engineer may discontinue said work and shall not be liable or responsible for the completion thereof.

4. In so far as consistent herewith and to the extent adaptable hereto, all of the terms and provisions of said prior agreement to which this agreement is supplemental are hereby made applicable to this agreement and are hereby confirmed, ratified, and continued in effect.

IN WITNESS WHEREOF, the parties hereto have executed this agreement to be effective as of the date hereinabove first written.

Approved as to Form and Procedure

/s/ Donald H. Benton Attorney, Ventura County Flood Control District

Approved as to Form and Procedure

/s/ Henry Holsinger Attorney for Division of Water Resources

Approved as to Form and Procedure

Director of Public Works

Attorney, Department of Public Works

APPROVED:

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/s/ James S. Dean Director of Finance

:L.E.Z.:F.J.M.: :

: Form : Budget: Value: Descript.: DEPARTMENT OF FINANCE

APPROVED

May 19 1952

/s/ A. D. Edmonston

A. D. Edmonston State Engineer

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VENTURA COUNTY FLOOD CONTROL DISTRICT

By /s/ R. E. Barrett Chairman, Board of Supervisors

STATE WATER RESOURCES BOARD

By /s/ B. A. Etcheverry Vice Chairman

> State of California Department of Public Works

FRANK B. DURKEE

By /s/ Russell S. Munro

MEMORANDUM OF UNDERSTANDING

WITH REFERENCE TO WATER RESOURCES INVESTIGATION OF VENTURA COUNTY

The objective of this memorandum of understanding is to coordinate the work of the State of California, Ventura County Flood Control District, and the United Water Conservation District in the investigation of the water problems in the County of Ventura and the formulation of plans for their solution.

It is contemplated that an agreement will be executed between the State Water Resources Board, the Ventura County Flood Control District, and the Department of Public Works acting through the State Engineer for the purpose of conducting a county-wide comprehensive investigation of the water resources of Ventura County.

This memorandum is a prerequisite of the execution of the aforesaid agreement.

This memorandum sets forth only that part of the county-wide investigation in which all three of the named agencies are interested. Other parts of the county-wide study will go forward simultaneously under plans approved by the State and the District, and work primarily of interest to the Conservation District will be done and/or paid for by that agency. However the work of all agencies shall be closely coordinated, and information shall be freely exchanged. Any work done by the Conservation District with funds furnished by the Flood Control District shall be of sufficient scope to serve the purposes of the county-wide survey, as determined by the State Engineer.

This memorandum is preliminary in nature and shall be revised as necessary as the work proceeds, and all revisions shall be approved by representatives of the State, Flood Control District and Conservation District.

1. General Water Supply Studies

- a. State
 - Geologic investigation of Santa Clara River Basin and Oxnard Plain, including Pleasant Valley. Any geologic work beyond that which the State does shall be paid for by whatever agencies request additional work.
 - 2. Water supply available from all streams in Santa Clara Basin.
 - 3. Method of utilization of conserved water.
- b. United Water Conservation District
 - Make an independent check on previously reported values on water supply available at various reservoir sites on Sespe and Piru Creeks.
 - Make yield studies of reservoirs under consideration on Sespe and Piru Creeks in conjunction with utilization of ground water storage, up to maximum practicable capacity at each site.
 - 3. Determine a method of conservation and determine location, capacity and size of spreading grounds in the Santa Clara River Basin.
- 2. Dam and Reservoir Site Investigations
 - a. Sespe Creek
 - 1. Fillmore Site, approximately two miles above Telegraph Road.
 - (a) State

Locate drill holes and obtain rig and drill churn drill holes, obtain and operate geophysical rig for determining seismic profile and make a geological report.

Study availability of materials suitable for an earthfill dam of maximum planned height.

(b) Ventura County Flood Control District

Topographic survey and map of dam site, Scale 1" = 200', contour interval 5', to a height of 400' above stream-bed. Survey to be tied into U.S.G.S. surveys. Contour at an elevation of approximately 200' above stream-bed at dam site, to be located and mapped in order to check new U.S.G.S. sheet of this area.

Locate all cultures in dam and reservoir site and estimate cost of acquisition of lands. Calculate height, capacity and surface area curves for reservoir using new U.S.G.S. sheet. Obtain permissions to enter on lands for purpose of drilling holes or seismic work. (c) United Water Conservation District

If this site is found feasible geologically, make preliminary designs and cost estimates for several heights of dam.

2. Hammel Site

(a) State

Make geological investigation of dam site. Investigate availability of materials.

(b) Ventura County Flood Control District

Provide topographic maps of reservoir and dam sites. Make estimate of cost of acquisition of lands and right of way.

(c) United Water Conservation District

Make preliminary designs and cost estimates for several heights of dams.

- 3. Topa Topa Site
 - (a) State

Investigate geology and availability of materials.

(b) Ventura County Flood Control District

Make estimate of cost of acquisition of lands and/or right of way.

(c) United Water Conservation District

Make preliminary designs and cost estimates for several heights of dam.

- 4. Cold Spring Site
 - (a) State

Investigate geology and availability of materials.

(b) Ventura County Flood Control District

Make estimate of cost of acquisition and right of

way.

(c) United Water Conservation District

Make preliminary designs and estimates of cost for several heights of dam.

- b. Piru Creek
 - 1. Devil Canyon Site
 - (a) State

Investigate geology and availability of materials.

(b) Ventura County Flood Control District

Provide maps of reservoir and dam site. Make estimate of cost of acquisition of lands and right of way. (c) United Water Conservation District

Make preliminary designs and estimates of cost for several heights of dam.

- c. Santa Paula Creek
 - 1. Ferndale site
 - (a) State

Make estimate of flood control benefits. Make estimate of conservation benefits. Investigate geology and availability of materials.

(b) Ventura County Flood Control District

Make topographic survey and map of dam site, scale 1" = 100', contour interval 5', to a height of 300' above stream-bed. The new U.S.G.S. sheet will be used to calculate a height, capacity and surface area curve for this dam. Make estimate of cost of acquisition of lands and right of way.

(c) United Water Conservation District

Make preliminary designs and estimates of costs for several heights of dam.

As soon as the most satisfactory dam site or dam sites are decided upon in the Santa Clara River Basin, United Water Conservation District will proceed with detailed investigation of said sites. Such investigation, if paid for with funds advanced by the Ventura County Flood Control District, shall be sufficiently broad to cover the needs of the county-wide investigation.

All work outlined herein to be done by the State of California and by the Ventura County Flood Control District is to be paid from funds to be

made available under the aforementioned cooperative agreement between the State Water Resources Board, the Ventura County Flood Control and the State Engineer. Work to be done by United Water Conservation District will be paid for from funds provided by that agency.

April 23, 1951

/s/ A. D. Edmonston A. D. Edmonston, State Engineer

April 24, 1951

/s/ Robert L. Ryan, Robert L. Ryan, County Surveyor Ventura County

April 23, 1951

/s/ Julian Hinds Julian Hinds,

Julian Hinds, Consulting Engineer United Water Conservation District MEMORANDUM OF UNDERSTANDING With Reference to Water Resources Investigation of Ventura County

the

A memorandum of understanding was entered into on April 23 and 24, 1951, between representatives of the State Division of Water Resources, the Ventura County Flood Control District, and the United Water Conservation District, with the objective of coordinating the work of the three agencies in the investigation and study of the water problems of Ventura County. The Memorandum set forth the part of the coordinated program each agency would perform in the investigation and the cooperation that would be effected in the exchange of data so that there would be a minimum of duplication of effort and so that completion of the work would be effected as expeditiously as possible. Such coordination has been effectively carried out.

A meeting of the representatives of the foregoing three agencies was held in the office of the State Engineer on September 29 and 30, 1952, for the purpose of reviewing the results so far accomplished by the three agencies and to program certain further work to be done. At the conclusion of said meeting it was mutually agreed that both the United Water Conservation District and the State Division of Water Resources would:

1. Prepare independent cost estimates for concrete arch dams at the Topa Topa site on Sespe Creek to create gross storage of water in the amounts of 50,000 acre-feet, 75,000 acre-feet, and 100,000 acre-feet respectively.

2. Review existing yield studies for reservoirs created by the foregoing sizes of dams on the Topa Topa site on Sespe Creek.

3. Review existing yield studies for a reservoir of 100,000 acre-foot storage capacity at the Santa Felicia site on Piru Creek.

Said work to be accomplished by October 15, 1952, the results thereof to be submitted to the other parties to this agreement on or about that date, and a further meeting between the parties to be held in Santa Paula on October 24, 1952.

It was also mutually agreed, based on preliminary information available, that a rolled earth-fill type of dam is suitable and appropriate for the Santa Felicia site on Piru Creek, and that the capacity of Santa Felicia Reservoir should be limited to a maximum storage capacity of about 100,000 acre-feet in order to preserve from inundation the upstream Blue Point Dam and Reservoir site and to permit future development of said Blue Point site.

It was further mutually agreed that any dam constructed at the Topa Topa site on Sespe Creek should preferably be built initially to the maximum practicable size without provision for future enlargement.

> /s/ A. D. Edmonston A. D. Edmonston, State Engineer

> /s/ Robert L. Ryan Robert L. Ryan, County Surveyor Ventura County

/s/ Julian Hinds Julian Hinds, Consulting Engineer United Water Conservation District

Sacramento, California October 1, 1952

MEMORANDUM OF UNDERSTANDING With Reference to Water Resources Investigation of Ventura County November, 1952

Pursuant to a memorandum of understanding entered into on October 1, 1952, by representatives of the State Division of Water Resources, the Ventura County Flood Control District, and the United Water Conservation District, a meeting was held in the offices of the United Water Conservation District in Santa Paula on October 24, 1952, among representatives of the foregoing agencies. Under terms of the aforementioned memorandum of understanding, both the United Water Conservation District and the State Division of Water Resources were to prepare estimates of cost for concrete arch dams at the Topa Topa site on Sespe Creek creating reservoir capacities of 50,000, 75,000, and 100,000 acre-feet, respectively, and to review existing yield studies for reservoirs of these capacities. In addition, existing yield studies for a reservoir of 100,000 acre-feet capacity at the Santa Felicia site on Piru Creek were also to be reviewed.

As a result of these studies and in accordance with the conclusions derived and concurred in by the attendant parties at the meeting of October 24, 1952, it is mutually agreed that:

1. The reservoir yields independently determined under terms of the memorandum of understanding, dated October 1, 1952, are in agreement.

2. A fill type dam will be constructed at the Santa Felicia site on Piru Creek creating a gross reservoir capacity of not less than 90,000 acre-feet nor more than 100,000 acre-feet.

3. A concrete arch dam, creating a gross reservoir capacity of not less than 50,000 acre-feet, will be constructed at the Topa Topa site on Sespe Creek.

4. The United Water Conservation District will prepare plans and call for bids for construction of concrete arch dams at the Topa Topa site, which would create reservoir capacities of 50,000 and 60,000 acre-feet, with the objective of constructing as large a capacity reservoir as financial limitations will permit.

5. The State Division of Water Resources will study and report on the feasibility of an overpour spillway for concrete arch dams at the Topa Topa site for gross reservoir capacities of 50,000 acre-feet and larger.

6. Construction of not less than 90,000 acre-feet nor more than 100,000 acre-feet of gross reservoir storage capacity at the Santa Felicia site on Piru Creek, and of not less than 50,000 acre-feet of gross reservoir storage capacity at the Topa Topa site on Sespe Creek, is consistent with an overall plan for the conservation and utilization of the water resources of Ventura County.

> /s/ A. D. Edmonston A. D. Edmonston, State Engineer

/s/ Robert L. Ryan Robert L. Ryan, Engineer Ventura County Flood Control District

/s/ Julian Hinds Julian Hinds, Consulting Engineer United Water Conservation District

APPENDIX B

GEOLOGY AND GROUND WATER OF VENTURA COUNTY, CALIFORNIA

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CHAPTER B-I. INTRODUCTION

Ventura County includes an area of approximately 1,857 square miles inch is bounded by surveyed lines chosen irrespective of watershed boundaries orgeologic features, except along its southwest and in part its west sides. The southwestern boundary is the Pacific Ocean.

This appendix includes a description of the geology of Ventura County an adjacent areas with particular emphasis placed upon those geologic features which influence the occurrence and movement of ground water. Its purpose is threefold, namely:

1. To describe the geology and water-bearing properties of the rocks.

2. To discuss the effects of geologic structure upon the movement of pound water and the infiltration of sea water, and to describe briefly the ustory of events involving the evolution of the principal structures.

3. To describe the procedures followed in estimating the changes in yound water storage and estimating sub-surface ground water movement that ocurred within the principal basins during selected periods of study.

The older less permeable formations which yield little water are ceated briefly. These rocks are mentioned because: they are the parent source sediments which fill the ground water basins, in certain localities they effect the chemical character of the ground water, their position in part controls the movement and occurrence of ground water, and they form or delimit the ground water basins.

The permeable water-bearing formations are described in greater detail. uese deposits comprise the fill of the ground water basins, the principal sources ground water supply in the County.

Subsurface ground water geology was interpreted largely from the logs : some 1,534 water wells, most of which were obtained from the Ventura County ater Survey and some from field canvass. Considerable shallow subsurface data

B--6

in the form of 138 electric logs, drillers logs, and core descriptions were obtained from the State Division of Oil and Gas and from oil companies operating in various areas. Ground water level data and water analyses were amassed and in certain areas the transmissibility of sediments was estimated by pump testin of wells. All of these data were drawn upon freely in interpreting the geology

A perusal of geologic literature revealed a number of maps and report prepared by earlier investigators covering various parts of the County. These existing data were drawn upon freely in the preparation of this report and are listed in the accompanying bibliography.

In the course of this investigation, two geologic maps were prepared by compiling data from existing geologic maps, from aerial photographs and from field mapping by the Division of Water Resources in areas where existing data were insufficient. One of these maps is of small scale and depicts the entire County and tributary drainage areas (Plate 10); the second is a detailed map covering the area containing the ground water basins (Plates B-1A, B-1B, and B-1C).

CHAPTER B-II. PHYSIOGRAPHY

The southern portion of Ventura County is to a large extent located in he Transverse Ranges geomorphic province, while that portion north of the Santa nez fault (see Areal Geology, Plate 10) is in the southerly section of the Coast anges province (Jenkins 1943). The mountains and valleys in the southern porion of the County trend nearly east-west, while in the northern portion of the county they trend more in a northwest-southeast direction. The physiography of he County can best be described by covering the following features: mountains, valley areas, the coastal plain, and submarine topography.

Mountains

The principal mountains in Ventura County include the Piru Mountains (named by Axelrod, 1950), the Santa Ynez Mountains, the Topatopa Mountains and the Santa Monica Mountains. Smaller mountain areas include Oak Ridge, the Santa Susana Mountains north of the Simi Valley area, and the Simi Hills south of Simi Valley. All mountain areas are generally maturely dissected and rugged, with relief ranging from 500 to 2,000 feet. Soil cover is generally thin, but some areas of rolling topography are found where soil cover becomes quite thick. Such areas are generally found in the higher parts of the ranges and may represent remnants of one or more old erosion surfaces.

Valley Areas

In general, many of the valleys were formed under earlier geologic conditions when the area was nearer base level (in most cases sea level). Since being formed most of the valleys have been uplifted and have undergone additional erosion. Generally, cycles of erosion and alluviation have been repeated. A few of the valleys are largely the result of structural movements.

The Santa Clara River Valley is the most prominent valley in Ventura

County and trends east-west. It is controlled essentially by structural feature: In general, it is a downfolded and faulted trough between mountains to the north and south. Deposition by the Santa Clara River and by smaller tributaries has been fairly continuous in most of the valley, while terraces on the side slopes may be due in part to periodic uplift of the sides of the valley with respect to the valley floor.

Physiography of the Ventura River drainage area has been discussed in considerable detail by Putnam (1942). Streams in Ojai Valley and the Coyote Creek drainage evidently originally drained westward and eastward respectively toward the Ventura River. Headward erosion of San Antonio and Coyote Creeks has captured those drainages so that they now drain in a southerly direction. Ojai Valley is in a structural depression in which over 700 feet of fluviatile sediments have been deposited. The Coyote Creek drainage area is also in a structural depression, but apparently the area has not been as active as the Ojai Valley area and only thin deposits of alluvium are found there.

The small Upper Ojai Valley, according to Putnam (1942), originally drained westward and the headwaters included the upper portion of the present Santa Paula Creek. Most of the alluvial material now found in Upper Ojai Valley was deposited at that time. Subsequent headward erosion of Santa Paula Creek, I possibly aided by tectonic movements, has resulted in the capture of the present drainage system, so that Upper Ojai Valley now drains both to the west and to the east.

The north-south trending Ventura River Valley has been essentially an erosional feature, a relatively small thickness of alluvial fill being found in the valley at the present time. Terrace deposits indicate that the valley has undergone at least two cycles of erosion. Putnam (1942) presents evidence that the most prominent of the terraces has been gently warped upward, with the axis of the upwarping near Casitas Springs.

The Las Posas upland area extends eastward from Oxnard Plain almost to imi Valley, and lies between Oak Ridge to the north and the Camarillo and Las bass Hills to the south. This broad upland area slopes generally to the south. bth erosion and deposition are occurring in places within the area at the present tme. It is possible that Arroyo Las Posas once flowed westward from the vicinity 'Somis, north of the Camarillo Hills, to the Oxnard Plain. If so, it was proboly diverted south at Somis into the valley it presently occupies by the buildingp of alluvial fans extending from Oak Ridge in the area northwest of Somis.

Simi Valley is located in a structurally depressed area in which over D0 feet of alluvial sediments has accumulated. Simi Valley has been through more han one cycle of erosion as indicated by the exposure and present dissection of he older alluvium on the southwest side of the valley. The Simi fault, extending long the north side of the valley, has apparently been active during deposition f most of the alluvial fill. This is inferred from the 60-foot thickness of the lluvium in the valley just west of where the Simi fault crosses the alluvium. his area is north of the Simi fault or on the uplifted side. The bulk of Simi alley and the greatest thickness of sediments is south of the Simi fault.

Conejo Valley is a broad valley which has been a part of a larger genrally east-west valley system lying in part in Los Angeles County. This old alley system was evidently originally developed by a through stream flowing ither east or west. The former drainage system has been captured by headward rosion of Conejo Creek in Conejo Creek Canyon north of Newbury Park, aided by robable northward tilting of the Conejo Valley area and fracturing of rocks.

Tierra Rejada and Santa Rosa Valley are both essentially erosional eatures, although up to 200 feet of alluvium has been deposited in Santa Rosa falley. The harder portions of the existing volcanic rocks have created temporary base levels resulting in erosion of Tierra Rejada and another smaller

valley in the upper drainage of Santa Rosa Valley.

There are several other valley areas in Ventura County which are not described here since they lie outside that portion of the County with which thi report is principally concerned.

Coastal Plain

The Coastal Plain has been formed by deposition of sediments from the Santa Clara River and from the Calleguas-Conejo drainage area. The land surface resembles a large compound alluvial fan having one apex near Saticoy and another near Somis. A group of smaller, but steeper, alluvial fans has been deposited by the smaller creeks draining the hills north of the area, forming an alluvial piedmont. Remnants of terraces along the northern edge of the coastal plain indicate uplift of this part of the plain. This rise might be du to relatively recent upwarping in the Ventura River drainage area (Putnam 1942) possibly augmented by changes of sea level.

Submarine Topography

Principal features of offshore topography are shown on Plate B-3 whic is taken from U. S. Coast and Geodetic Survey chart number 5202 (Point Dume to Purisima Point). These features are also discussed in papers by Emery and Shepard (1945) and Emery and Rittenberg (1952). Santa Rosa, Santa Cruz and Anacapa Islands are extensions of the Santa Monica Mountains. A scarp-like feature extends in an east-west direction on the south side of the islands and the Santa Monica Mountains. This scarp is cut by the southerly trending Huenem and Mugu submarine canyons which end in the floor of the Santa Monica submarine basin lying south of the Santa Monica Mountains. In addition to these major canyons, three poorly developed or incipient canyons exist between the two majo canyons. The Santa Barbara basin lies due west of the Coastal Plain of Ventura County. This basin is relatively shallow and the slope of the surface west of

the Oxnard Plain is gentle, being as low as five to fifteen feet per mile. In comparison, the slopes of the east-west scarp and the heads of the submarine canyhs are quite steep. Hueneme Canyon cuts across the southeastern corner of the gitly sloping Santa Barbara basin.

The heads of the submarine canyons are within a quarter of a mile of the sore, and water-bearing materials are probably exposed along the canyon walls, alowing free contact between sea water and permeable formations. This contact is wry important in considering movement of ground water, since, depending on the drection of the gradient, fresh water can be discharged into the ocean or sea ter may move into the aquifers.

CHAPTER B-III. GEOLOGIC FORMATIONS

General descriptions of all geologic formations and a short discussio of their role in the hydrology of Ventura County are included in this section.

The detailed geologic maps of the southern part of Ventura County (Plates B-1A, B-1B, and B-1C) show the areal extent and distribution of formations based on lithology. Plate 10 is a generalized geologic map of the entir Ventura County and Santa Clara River drainage area. The stratigraphic columns Plate B-2 indicate the age relationship between the various formations by area.

The complex geology of Ventura County has been mapped by many people over a period of more than forty years. As a result, present terminology is somewhat confusing to those not familiar with the problems of the area. An attempt has been made in this report to use names of formations which are commonly accepted by local geologists. Wherever lack of agreement seemed to exist the formation names which appeared to be most familiar to local geologist were adopted.

Basement Complex

The basement complex is composed of granitic and metamorphic rocks of pre-Cretaceous age. The metamorphic rock types include gneiss, schist, hornfel, quartzite, and limestone, indicating that the rocks, before they were metamorphosed, were mostly sediments. These metamorphic rocks were intruded in Jurassic (?) time by granitic and dioritic rocks (Kew 1924, Wallace 1949, and Crowell 1950 and August and October, 1952 and others).

Large areas of the Piru Creek-Santa Clara River drainage area are underlain by deeply weathered granitic and dioritic rocks. Both the igneous an metamorphic rocks are fractured and jointed, being more extensively fractured near large faults.

The basement complex is essentially nonwater-bearing, but scattered

mestic and stock wells obtain small quantities of water from weathered residual terials and from fractures. Small springs are fairly common in areas of baseint complex, especially during wet periods.

Surface water derived from areas of basement complex is generally a Licium bicarbonate type with moderately low dissolved solids.

Cretaceous System

Consolidated sediments of Cretaceous age in the Ventura Region have een called the Chico formation by early workers, but oil company geologists in the last few years have generally dropped this term. Since the formations have of been assigned local names, they are generally called Upper Cretaceous rocks, • simply Cretaceous rocks. The rocks consist of marine shales, siltstones, indstones, and conglomerates having an aggregate thickness in excess of 7,000 feet. the shales and siltstones are generally well bedded and dark gray to black in plor. Upon weathering, they splinter, ultimately disintegrating to gray clayey pils. The sandstones are mostly medium to coarse grained, arkosic, locally iteaceous, dark gray in color, and wherever weathered they are usually gray or uite. Most exposures show the rocks to be well cemented and indurated; although a some areas the degree of cementation is slight.

The Upper Cretaceous rocks in the Simi Hills are essentially massive andstones with thin interbedded shales (Kew 1924; Stipp 1943). In the Santa nez Mountains and in a small area of the Piru Mountains, they consist of thin eds of shale, siltstone, sandstone, and lenticular conglomerates. Conglomerates in the Wheeler Springs area contain well rounded, well cemented sandstone, ranitic and metamorphic pebbles (Merrill 1952). The Upper Cretaceous rocks re generally folded and faulted and exhibit complex fracture and joint systems.

Fossils found in this series include <u>Baculites</u> chicoensis and other arine invertebrates indicative of the age of the rocks.

The Upper Cretaceous rocks are generally nonwater-bearing. In the hill south and east of Simi Valley, however, water wells obtain domestic and limited irrigation supplies from the massive poorly cemented and fractured sandstones. *F* well drilled into one of the sandstones has yielded over 1,000 gallons per minute probably from fracture systems. Small springs generally occur near fractures and faults in the Santa Ynez Mountains. Most of the springs yield cool, fresh water; but some of them which may be associated with faults such as Wheeler Hot Springs, produce warm water of good quality which is generally low in mineral constituents except boron. Surface water from areas of outcrop of these rocks is generally of good quality.

Paleocene-Eocene Series

Several formations are included in this series in Ventura County. The Martinez formation comprises the oldest rocks of the series. This formation is found in the Piru Creek area (Clements 1937), the Castaic Creek area (Clements 1937 and Dehlinger 1952), and in the Santa Monica Mountains, but has not been differentiated on Plate 10. In these areas, the Martinez formation consists of marine shale, sandstone, and conglomerate.

The Santa Susana-Martinez formation of Paleocene and middle and lower Eocene age (Stipp 1943) in the Simi Hills and on the south side of Simi Valley consists of up to 3,500 feet of light colored sandstone, some interbedded shale and a massive basal conglomerate. The lower Llajas formation (Meganos of Kew 1924) consists of about 2,000 to 3,000 feet of olive drab and blue shale, minor interbedded sandstone, and a basal conglomerate. The upper Llajas formation (Tein formation of Kew 1924) consists of about 2,000 feet of brown to gray micaceous sandstone, siltstone, and some conglomerate (Stipp 1943). Both the upper and lower Llajas formations are exposed in the Simi Hills, and are of middle Eocene age.

The formations described below are all exposed north of the Santa Clare

Hver Valley and are of upper Eocene age. The Juncal formation consists of cout 5,000 feet of marine olive drab to grey siltstone and shale grading downrd into concretionary dark grey siltstone and shale. Near the base of the incal formation, there is a black calcareous shale which is the equivalent of te Sierra Blanca limestone in Santa Barbara County (Merrill 1952). The Matilija indstone lies conformably on the Juncal and consists of about 2,400 feet of trine light colored sandstone with minor interbedded grey siltstone. The Cozy hill shale lies conformably over the Matilija sandstone and consists of about 800 feet of olive drab to grey siltstone and shale. North of the Santa Ynez uult, a prominent mapable white sandstone member has been included within the bzy Dell (Merrill 1952). Coldwater sandstone conformably overlies the Cozy Dell vale. It consists of about 2,200 feet of white massive sandstone with interbded red and green silty shale and lenticular conglomerate.

In the extreme northwest part of Ventura County a series of brackish ster and marine sandstones and shales of Eocene age have been named the Pattiay formation (Carlson 1952, Dibblee 1952).

Index fossils commonly found in the Paleocene-Eocene formations are isted below:

| Santa Susana-Martinez formation | <u>Turritella pachecoensis</u> Venericardia venturensis |
|--|--|
| Upper and lower Llajas formations | <u>Turritella lawsoni</u> <u>Galeodea susanae</u> |
| Juncal formation, Matilija sandstone, Cozy Dell shale, and Coldwater sandstone | <u>Turritella</u> <u>andersoni</u> <u>Turritella</u> <u>uvasana</u> |

Shales of the Paleocene-Eocene series are generally well indurated, and the sandstones are fairly well cemented. The entire series generally forms rugged topography with thin sandy soils. Sandstone beds form particularly massive outcrops and very rugged topography. Rocks of this series are strongly fractured, faulted, and folded. They are generally nonwater-bearing. However, in the hills south and east of Simi Valley domestic and limited irrigation supplies are obtained from poorly cemented sandstones of the upper Llajas and the Santa Susana-Martinez formations. Occasional windmills in the Santa Ynez, Topatopa, and Firu Mountain regions obtain water from the sandstone members of this series. Generally, wells tapping these rocks at depths exceeding 300 to 400 feet encounter water of poor quality. Small springs are generally found in these rocks in the region north of the Santa Clara River.

Surface water derived from areas of outcrop of the Paleocene-Eocene series is generally of calcium sulphate type with about 600 to 1,500 and above parts per million (ppm) total dissolved solids.

Oligocene Series

The Oligocene series in the Ventura County region consists of continental lenticular interbedded conglomerate, sandstone, and shale. No Oligocene marine deposits are known to exist in the area covered by the geologic maps. Th sandstones are poorly to well cemented and generally buff to grey in color. Conglomerates generally contain granitic, metamorphic, sandstone, and chert peb bles and cobbles in a matrix of sandstone or siltstone. The siltstones and shall are generally micaceous and red, maroon, blue, or grey in color. The formation generally weather into red sandy clay soils which are characteristic of areas o outcrop of the series.

In most of Ventura County, the Oligocene series is called the Sespe formation. In the eastern Santa Clara River drainage area, it has been called

Wasquez formation (Sharp 1935). Beds of probable Oligocene age in the ex-

The Sespe formation varies in thickness from 1,200 to 7,300 feet and iges in age from upper Eccene to lower Miccene, as determined by vertebrate fasils (Bailey 1947).

The Vasquez formation is of lower Miocene and Oligocene age (Jahns L40) and is up to 9,000 feet thick with about 4,000 feet of interbedded basaltic fows near its base. The Simmler formation is about 3,000 feet thick and con-

The Oligocene series is essentially nonwater-bearing. Sandstones and inglomerates in the Ojai region and in the Simi Valley area usually yield from to 100 gallons per minute to wells. One well in the sandstones northwest of mi Valley, however, yields up to 700 gallons per minute. Wells deeper than 0 or 500 feet generally obtain brackish water unsuitable for irrigation.

Surface waters derived from outcrop areas of the Oligocene series are nerally of a calcium and bicarbonate-sulphate type with about 300 to 800 ppm tal dissolved solids. In Lockwood Valley and in the Tick Canyon area in Los geles County, a borax mineral, colemanite, is commonly interbedded with the diments, and as a result, surface runoff from these areas may be high in boron ntent especially during periods of low flow.

Miocene Series

The Miocene series in the Ventura County region consists of several arine and non-marine formations, and includes volcanic rocks. Reference may be de to the stratigraphic columns (Plate B-2) while reading the short descripons of the formations which follow.

Marine Formations

Sediments deposited under marine conditions include the Vaqueros, Rincon, Topanga, Modelo, and "Santa Margarita" formations. The Vaqueros formation of lower Miocene age consists of 100 to 1,800 feet of gray to brown marine sandstone and shale. The Vaqueros formation has been mapped by some geologists in the southwestern part of the Santa Monica Mountains where it consists of black calcareous shale and minor sandstone beds, but it is included with the Topanga formation on the geologic map of this report because of the extreme geologic complexity.

The Rincon formation consists of over 2,000 feet of dark gray to brow concretionary shale and is exposed only north of the Santa Clara River.

The Topanga formation is mapped only in the Santa Monica Mountains ar Conejo Valley area. It is possibly the equivalent of the Rincon formation and the Vaqueros. Vaqueros fossils are found near the ocean south of Boney Mountai, but the beds are included in the Topanga formation because of the complexity of the area. The Topanga formation is mostly black or gray shale in the area soutwest of Boney Mountain. It is composed of sandstone, conglomerate, and brown shale in the Conejo Valley area, and grades into sandstone and conglomerate in the easternmost portion of the area shown on the geologic map (Plate B-1C). Tr Topanga formation is closely associated with volcanic rocks. The sediments of the Topanga formation have an aggregate thickness of 6,000 to 9,000 feet, while the intercalated volcanics have a maximum aggregate thickness of about 13,000 feet. The Topanga formation is unconformably overlain by Modelo sandstone and shale.

The Modelo formation is variable in lithology but has been subdivided into sandstones and shales by Kew (1924). This subdivision has been followed and used on Plates B-1A, B-1B, and B-1C. Thickness of the Modelo formation variable from zero to 6,500 feet. The sandstones of the Modelo formation are generally

Ight grey to tan in color and are fine to medium grained, and they contain iterbedded clay shales, and conglomerates. The shales of the Modelo formation enerally consist of laminated diatomaceous and cherty shales and clay shales, th minor interbedded sandstones and conglomerates. Fish scales and foraminifera e usually abundant.

The "Santa Margarita" formation includes up to 2,000 feet of tan, Ity, diatomaceous shales, and sandstones of uppermost Miocene age. Some of the ecks have been called the Pico formation by Kew and other names by other workers. The status of some of the sediments included in the "Santa Margarita" formation "this report is somewhat uncertain. This term has been adopted because of its redominant usage in the western portion of the county. The quotation marks are cessary, since lithology is not consistent and may not resemble that of the pe locality.

Distinctive fossils of the Miocene marine formations are listed below:"Santa Margarita" formationModelo formationDelectopecten pedroanusForaminifera onlyTopanga formationTurritella ocoyana

Rincon formation

<u>Turritella ocoyana</u> <u>Turritella boesei</u>

Foraminifera only

For foraminifera in these formations, see Kleinpell (1938) and Hanna Manna Hertlein (1943).

ntinental Sediments

Continental sediments of Miocene age include the Quatal formation in ne Cuyama Valley area and the Mint Canyon formation in the Los Angeles County ortion of the Santa Clara River drainage area. The Quatal formation consists of ,500 feet of red gypsiferous clay, sand, and poorly cemented conglomerate underuin by 3,000 feet of red sandstone and poorly cemented fanglomerate (Dibblee 952). The Mint Canyon formation (Kew 1924) consists of 4,600 feet of lacustrine and fluviatile sandstone, conglomerate, clay, and some marl. Fresh water molluscs, plant remains, and land vertebrate remains have been found in the Min Canyon formation (Jahns, 1940).

Volcanic Rocks

The volcanic rocks, mostly of Miocene age, associated with the Topang formation include up to 13,000 feet of pyroclastics and basaltic flows. The pyroclastics include agglomerates, mud flows, and rhyolitic rocks. Andesitic ar basaltic dikes, sills, and plugs intrude the flows and the associated Topanga sediments. Fossiliferous marine shales, sandstones, and conglomerates occur from place to place interbedded with the flows. The volcanics are faulted, highly fractured, and moderately folded.

Hydrologic Properties

Most of the formations of the Miocene series are essentially nonwater bearing. Until 1953, the volcanics in the Conejo Valley and Tierra Rejada are constituted the principal formation in the Miocene series which was significant as far as water supply is concerned. Further discussion of the water-bearing properties of the volcanics is included in the description of the ground water basins.

Present information indicates that none of the Miocene marine sedimeter are potential major ground water reservoirs. That is, few wells could be drilled which would yield large quantities of water from these rocks. Deep wells in the Miocene marine formations generally obtain brackish or salty water. The Mint Canyon formation contains permeable beds which are potential sources of supply. As far as is known, no irrigation wells obtained water from the Mint Canyon formation up to 1953. The Quatal formation, of Miocene age, in the Cuyama Valley area contains extremely permeable gravels. It is possible that

rels yielding sufficient water for irrigation could be obtained if drilled in sutable locations. At present, only domestic wells are known to be supplied from this formation in Ventura County.

Surface water derived from areas of Miocene marine formations is generly a sodium-calcium-magnesium sulphate type containing from 400 to over 2,000 ppm cal dissolved solids. The runoff from areas of Miocene continental beds is generally a calcium sulphate type with from 200 to over 1,500 ppm total dissolved wids.

Pliocene Series

Formations of Pliocene age include the Pico, Saugus, and Morales forma-

The Pico formation consists of marine gray sandstone, blue gray shale, 1 lenticular conglomerate. These materials weather to a dull brown silty clay 1. Landslides are common in areas of outcrop. The Pico formation is unconmable with underlying and overlying formations in the Las Posas area but elsepre is generally conformable. It varies in thickness from 12,000 feet in the ntura area to zero in the Las Posas area. In the Ventura area, the Pico, Modelo, 1 Santa Barbara formations are conformable and interfinger. The Pico-Saugus ntact in the Saugus-Castaic area in Los Angeles County transgresses time so that 2 lower part of the Saugus formation, near the town of Saugus, is the age equivent of the upper portion of the Pico formation to the west. Typical fossils und in the Pico formation include <u>Turritella cooperi</u>, <u>Pecten healyi</u>, and <u>Chione</u> rnandoensis.

The Saugus formation consists of up to 2,500 feet of non-marine brown nd, slightly cemented gravel, and gray or tan clay. Kew (1924) originally cluded the San Pedro and portions of the Santa Barbara formation as described this report in the Saugus formation, but it is limited here to the eastern d of the Santa Clara River Valley in Los Angeles County. Vertebrate fossils

collected by W. H. Corey and identified by Chester Stock indicate that the Saugus formation in the Castiac-Saugus area is of upper and middle Pliocene age (verbal communication from W. H. Corey, April, 1953).

The Ridge Basin group of sediments is located between the San Andreas and San Gabriel faults south of Quail Lake, largely in Los Angeles County. This group consists of up to 18,000 feet of continental shale, sandstone, and conglomerate (Eaton 1939, Crowell 1950, August 1952, and Dehlinger 1952). Fresh water fish, vertebrate, and plant remains indicating Pliocene age are found in the Rid₁ Basin group (Axelrod 1950 and Crowell August, 1952).

The continental Morales formation is located in the northwest portion of Ventura County to the east and north of the Cuyama River. It consists of about 4,000 feet of gray to buff gravels and sands (Dibblee 1952).

Hydrologic Characteristics

The Pico formation is generally nonwater-bearing or yields salty water to wells. Some water wells in the Los Angeles County portion of the Santa Clara River drainage area probably obtain water from the permeable sands and gravels of the Saugus formation. A few water wells were observed which penetrate the Rive Basin group in Hungry Valley and Peace Valley, upper tributaries of Piru Creek nu U. S. Highway 99. A few irrigation wells are drilled into the Morales formation in Santa Barbara County. Both the Morales formation and the Ridge Basin group are possible potential ground water reservoirs. However, low rain-fall in these areas would probably result in limited replenishment of the formations after the ground water was depleted by pumping.

Surface water from the Pliocene formations is generally a sodium sulpt type with from about 400 to over 2,000 ppm total dissolved solids.

Lower Pleistocene Series

Sediments of the lower Pleistocene series comprise some of the most

anta Barbara Formation

The Santa Barbara formation is of lowermost Pleistocene and uppermost liocene age (Bailey 1935). It had been previously included in the Pico formaion by Kew (1924). It has been called upper Pico by Cartwright (1928), Driver 1928), and Waterfall (1929). The thickness and lithology of the Santa Barbara ormation varies considerably from about 4,000 feet of mudstone, shale, and minor andstone beds near the City of Ventura to about 1,000 feet of sand, gravel, and inor clay in the Tapo Canyon area and 800 feet of sand and clay in the southern art of the Oxnard Plain. Most of the clays and shales are blue in color when 'resh, and contain plant remains and distinctive foraminiferal faunas. The lightly cemented buff colored gravels and sands on Oak Ridge referred to in this report as the Grimes Canyon member of the Santa Barbara formation extend southwestward under the Las Posas area and into the Pleasant Valley area, where they pecome mostly fine to medium sand. Typical fossils found in the Santa Barbara formation are listed below:

| Vertebrates (Bailey 1935) | Equus cf. occidentalis |
|-----------------------------|--|
| Invertebrates (Bailey 1935) | Pecten <u>caurinus</u> Pecten <u>bellus</u> |
| Foraminifera (Natland 1952) | <u>Cassidulina</u> limbata |

The foraminifera found in the Santa Barbara formation in the Hall Canyon area indicate deposition at depths of 125 to 900 feet below sea level, while the lithology of the Grimes Canyon member near Grimes Canyon indicates beach or littoral deposition. The lithology and fossils indicate that the present area of the Santa Clara River Valley was under fairly deep water during deposition of sediments which comprise the Santa Barbara formation. A fluctuating shoreline extended from near the Santa Monica Mountains through Moorpark and eastward through the Tapo Canyon area. The northward extension of the shoreline is now concealed by structure or destroyed by erosion.

San Pedro Formation

The San Pedro formation is of lower Pleistocene age. North of the Santa Clara River, it interfingers with the underlying Santa Barbara formation. South of the Santa Clara River, it is generally unconformable on the Santa Barbara formation. In the Oxnard Plain-Pleasant Valley area, the available oil well logs indicate a conformable contact. The San Pedro formation consists of up to 4,000 feet of marine and continental gravel, sand, and clay. North of th Santa Clara River, the San Pedro formation consists of extremely lenticular bed with many scour and fill features. The base of the San Pedro in this area contains abundant marine fossils, but from near the middle of the formation to the top, marine fossils are rare except near the City of Ventura and the Ventura River. An oil well drilled through part of the San Pedro formation near the mouth of Aliso Canyon encountered mostly blue-green clay with abundant wood framents and plant remains, indicating fresh water swamp deposits. A prominent st and gravel zone up to 300 feet in thickness containing marine fossils on the soul side of Oak Ridge and beneath the Las Posas area is called the Fox Canyon membe in this report. The Fox Canyon member can be traced on the surface and in wate and oil well logs in the Las Posas area and is found at or near the base of the San Pedro formation. The Fox Canyon member extends into Pleasant Valley and th Oxnard Plain, but available well logs indicate that it is probably not as homogeneous there as it is in the Las Posas area.

During deposition of the San Pedro formation the Oxnard Plain was postly submerged to depths of about 125 feet and was being filled by deposition from the ancient Santa Clara River. The Santa Clara River Valley itself was subiding so that the thickness of sediments is now greater there than anywhere else in Ventura County. As the basin filled with sediments, the shoreline moved westard, and some of the San Pedro formation is, therefore, of continental origin. is a result, nearly all the San Pedro formation exposed near the Ventura River intains marine fossils, but only the base of the formation contains marine is just east of Santa Paula.

Nearly all the San Pedro formation in the Las Posas and Oxnard Plain reas was deposited in shallow water, probably partially in lagoons.

> Typical fossils in the San Pedro formation are listed below: Vertebrates (Bailey 1943) Equus cf. occident

Invertebrates (Bailey 1935)

Equus cf. occidentalis Chendytes lawi

<u>Crepidula princeps</u> <u>Cancellaria tritonidea</u> <u>Cantharus fortis</u> Pecten Circularis

Foraminifera (Natland 1952)

Elphidium hannai Rotalia becarrii

Some of the deeper sediments deposited in Simi Valley and Ojai Valley ay be of lower Pleistocene age but are described herein with the upper Pleistoene Series.

ydrologic Properties

Ground water occurs in sands and gravels of the Santa Barbara and San

Pedro formations. The Santa Barbara formation probably contains water of poor quality in the Santa Clara River area and portions of the Oxnard Plain-Pleasant Valley area as is indicated by electric logs of a few oil wells. The Grimes Canyon member contains fresh water of good quality in the Las Posas area and in the Pleasant Valley area. At the time of this investigation, only a few wells were obtaining water from the Grimes Canyon member or other sands of the Santa Barbara formation alone. A few other wells obtained water from the Santa Barbar as well as the overlying San Pedro formation.

The San Pedro formation yields water to wells in the Santa Clara River Valley and in the Las Posas, Oxnard Plain, and Pleasant Valley areas. As far as is now known, all water in the San Pedro gravels and sands is of good quality except that below about 2,000 feet in the Santa Clara River area, where a few electric logs indicate that the water may be slightly brackish.

Surface runoff from the Santa Barbara and San Pedro formations is geneally of fair quality.

Upper Pleistocene and Recent Series

Sediments of upper Pleistocene age include most of the gravels, sands, and clays younger than the San Pedro formation. In general, they are all undisturbed or only gently folded in contrast to the San Pedro and older formation.

The upper Pleistocene series in the Oxnard Plain-Pleasant Valley area extends from the top of the San Pedro formation to within about 20 to 50 feet of the surface. It consists of up to 500 feet of interbedded marine blue clay and sand, alluvial silt, and stream deposited sand and gravel. The principal aquifer on the Oxnard Plain is a stream deposited gravel of upper Pleistocene age which is called the Oxnard aquifer in this report. In the Pleasant Valley area, the upper Pleistocene series contains lenticular gravels which yield water wells. From available well log data, the base of the upper Pleistocene series

apears to lie unconformably on the San Pedro formation in Pleasant Valley and i parts of the Oxnard Plain.

The alluvium in Simi and Ojai Valleys is probably largely of upper and lwer Pleistocene age, approximately the upper 50 feet being Recent. Most of te alluvium in the Santa Clara River Valley is also Pleistocene, and the total tickness of alluvium in the river bottom ranges from five or six feet at Blue (t to over 200 feet elsewhere. Most of the terrace deposits in Ventura County re probably upper Pleistocene.

Recent Alluvium is quite thin over most of Ventura County, probably no pre than 60 or 70 feet thick. It consists of sand, gravel, and clay. Most uter wells obtain water from materials underlying the Recent alluvium except 1 those areas where the water table is high.

Both upper Pleistocene and Recent sediments are more fully discussed ander the description of ground water basins. The purpose of this chapter is to discuss the geologic structure of Ventura County, placing particular emphasis on those features which affect the occurrence and movement of ground water.

Ventura County is located in two regions of fairly distinct structural characteristics which coincide with the geomorphic provinces mentioned in Chapte: B-II. The portion of the County north of the Santa Ynez fault (see Plate 10) is in the southern Coast Range province, and the portion south of this fault is generally included in the Transverse Range province. At the extreme northeast corner of Ventura County, the Sierra Nevada and Mojave Desert provinces meet bot the Coast Range and Transverse Range provinces near Lebec.

Many major structural features in Ventura County trend east-west, although considerable variation in direction exists. Principal structural feature of Ventura County and adjacent areas are shown on Plate 10. Plates B-1A, B-1B al B-1C show additional details of structure of the southern portion of the County. Geologic cross-sections are also included on these latter plates to illustrate structural features.

Faults

Faults in the Ventura County region may be divided into northwestsoutheast trending, northeast-southwest trending, and east-west trending faults. Some faults have been displaced horizontally and some vertically, while both com ponents of movement have occurred on others. Aside from the major or prominent faults shown on the geologic maps, there are minor faults and fractures too numerous to indicate. Nearly all faults are actually zones of faulting, the width of the zone generally being greatest on the larger faults. Relative direc tions of movement along the faults, where known, are shown on the geologic maps.

cthwest-Southeast Trending Faults

Major faults trending in a northwest-southeast direction include the m Andreas, Nacimiento, Pine Mountain, Hot Springs, San Gabriel, and Santa mana faults. The San Andreas fault is well known and is the longest fault in a faults. The San Andreas fault is well known and is the longest fault in a faults. Horizontal or more correctly right lateral movement has been preminant, with over 300 miles displacement possibly occurring since Jurassic time suggested by Hill and Dibblee (1953). The Nacimiento fault extends into Ventura inty near the Cuyama River. The Hot Springs fault, which is located southwest Lockwood Valley, may be an extension of the Nacimiento fault, the two having an displaced by the Big Pine fault. The San Gabriel fault is another major llt of California. Like the San Andreas fault, the predominant movement has in horizontal, and Crowell (Oct. 1952) has presented evidence for about 25 miles rizontal displacement along the fault since Miocene time. The Santa Susana ult, in the Santa Susana Mountains of Ventura and Los Angeles Counties, is a rust fault which dips to the north and appears to override the east end of the k Ridge fault (Herron 1952 and Sheller and Bien 1947).

rtheast-Southwest Trending Faults

The most prominent northeast-southwest trending faults are the Big Pine d Sycamore Canyon faults. According to Hill and Dibbleo (1953), the Big Pine ult is probably an extension of the Garlock fault (not shown), the two having en displaced by movement on the San Andreas fault. Hill and Dibblee report a ssible horizontal displacement on the Big Pine fault of 14 miles. The Sycamore nyon fault and associated faults in the Santa Monica Mountains are about 15 les long, and as far as can be determined have had mostly a vertical component movement.

It is possible that a major northeast-southwest trending fault exists ong the southeast edge of Pleasant Valley, but no evidence could be found that

such a fault affects water-bearing materials, and it has not been shown on the geologic maps.

East-West Trending Faults

Most of the remaining major faults in Ventura County are essentially east-west trending. The northernmost of these faults will be discussed first, and those on the south side of the county last.

The Pine Mountain fault is a north dipping reverse fault, and is appar ently a branch fault connecting the Big Pine and Hot Springs faults. The Tule Creek fault, although more than 20 miles long, is apparently not directly connect with other major faults. The Santa Ynez fault is another of the major Californi faults which have probably had both horizontal and vertical components of movement.

The Santa Ana fault, which crosses the Ventura River drainage area, he had fairly recent movement, and probably has been an important factor in the accumulation of the alluvial fill in Ojai Valley.

The San Cayetano fault is a north dipping reverse or thrust fault with a known low dip of about 20 degrees near Fillmore, and of about 60 degrees north of Santa Paula. The San Cayetano fault actually consists of two or more branche often with soft, easily deformed sediments between them (Sheller and Bien 1947) Nonwater-bearing formations have been thrust over the San Pedro formation along he San Cayetano fault in the area northeast of Fillmore. North of Santa Paula, Eocene sediments have been thrust over Pliocene sediments as shown on Section C-1 Plate B-1B.

The Oak Ridge fault extends along the south edge of the Santa Clara River Valley from Saticoy to a point about two miles southeast of Piru, where if turns southward into Oak Ridge and is cut off by the Santa Susana fault. The Saticoy fault may be a westward extension of the Oak Ridge fault or a branch of the The Oak Ridge fault has been penetrated by several oil wells drilled on Oak

dge, and it has been found that the fault dips southward about 60 degrees. der formations have been thrust up from the south over younger San Pedro sedients (see Section C-C', Plate B-1B).

Evidence for the location of the Saticoy fault shown on Plate B-lB was rund in logs of oil wells and in differences in water level elevation across it. Is not certain what happens to this fault at depth, but it does appear to die it westward and cannot be detected in well logs north of Montalvo or along the each south of Ventura. The south side of the Saticoy fault has been uplifted elative to the north side.

The Simi-Santa Rosa fault system consists of several branches and tensions. The north side of this fault system has been generally uplifted alative to the south and has apparently been one of the causes for the accumulaon of the thick alluvium in Simi Valley. East of Simi Valley the relative rection is reversed, with the south side being uplifted, suggesting a hinge or issors type fault.

It could not be determined during this investigation whether either the pringville fault zone or the Camarillo fault are extensions of the Simi-Santa as fault system. The Springville fault zone is located on the south side of the Camarillo Hills and consists of at least two parallel faults, portions of which are well exposed on the surface. It is possible that the fault zone coninues eastward south of Somis, but outcrops and well log data are not available or show this. This fault zone does not appear to affect the Oxnard aquifer in the Oxnard Plain although it does affect the San Pedro and older formations, at east near the Camarillo Hills. The Springville fault zone dips steeply to the orth with the north side being uplifted relative to the south.

The Camarillo fault extends along the south side of a low hill near town of Camarillo and curves northeastward toward Santa Rosa Valley. Evince for this fault is found in well logs, physiographic features and water level

data. The fault is probably nearly vertical and the morth side is uplifted. The fault appears to fade out eastward and cannot be detected beyond the Camarill Airport.

Faults of Hydrologic Significance

The major faults in Ventura County which are known to have a barrier effect on ground water are the Saticoy and Springville faults and a portion of th Camarillo fault. Some of the other major faults described above with accompanyin folding generally affect ground water indirectly by deformation of the water-bear ing materials. This deformation in some cases has resulted in changes of crosssection area of water-bearing formations, and in exposure and erosion of portions of the formations so that they can be recharged by surface waters.

Some of the faults of Ventura County may also be avenues of escape for deeper waters of poor quality. Faults which may be in this category include the Hot Springs and Santa Ynez and possibly the San Cayetano and Oak Ridge faults. Evidence for escape of deeper waters appears in the analyses of spring water, and in some cases in analyses of ground water in alluvium near the faults.

The Saticoy fault affects the San Pedro formation and possibly the over lying alluvium. It appears to act as a partial barrier to flow of ground water with a water level differential across it of up to 100 feet (see Plates 14B, 15B, and 16B). The barrier effect of the fault seems to be most pronounced near the town of Saticoy. Its effect on ground water in the area of the Santa Clara River bed is not known due to lack of well control. The fault also appears to die out or become less effective so that no prominent differentials in water levels can be detected across the fault two or three miles westward of Saticoy. The barrier portion of the Saticoy fault also results in the deflection of a part of the underflow from Santa Paula Basin westward into Mound Basin, the remainder flowing through or over the fault into Comard Forebay Basin.

The Springville fault zone has displaced the San Pedro and underlying rmations. This displacement and the reduction of permeability caused by moveit as shown on surface exposures of the fault has caused a barrier to southrd movement of ground water from beneath the Camarillo Hills into Pleasant Valr Basin. The barrier effect is evidenced by up to 60 feet water level differrial across the fault in aquifers of the San Pedro and Santa Barbara formations. is probable that the Springville fault zone has not affected alluvium in the hard Plain area.

Study of well logs indicates that the Camarillo fault displaces the Pedro formation and probably some of the alluvium near the town of Camarillo. und water contours generally do not indicate that this fault acts as a barrier flow of ground water in the San Pedro formation. Field observations indicate, rever, that local drawdown of pumping wells does not extend across the fault. er levels in the lenticular aquifers of the alluvium do appear to be higher the north side of this fault, suggesting that it may act as a barrier to the thward movement of ground water in the alluvium.

Folds

All rocks and sediments in Ventura County, except those most recently sosited, have been folded. Most of the folds, like the faults, trend in an it-west direction. In general, there are three prominent anticlinal areas which cually consist of several smaller folds. These are the Topatopa Mountains near it Santa Ynez fault, Oak Ridge, and the Simi Hills south of Simi Valley. The it Monica Mountains in the Ventura County area are essentially a north dipig homocline. The principal areas which are essentially synclinal in structure it the Cuyama drainage area northwest of the Big Pine fault, the area between in near the intersection of the San Andreas and San Gabriel faults which has in called the "Ridge Basin" in geologic literature, the Santa Clara River syn-

cline, and the area south of Oak Ridge.

Some of the folded formations in the high hills and mountains may form limited ground water basins under certain conditions, but since most of these structures are not explored by wells, they are not discussed further.

Folds of Hydrologic Significance

The most significant folds from a ground water standpoint are the Sant Clara River syncline, the Montalvo anticline, and the series of folds in the synclinal area south of Oak Ridge, all of which affect water-bearing materials.

Santa Clara River Syncline. The Santa Clara River syncline extends from the ocean south of Ventura up the Santa Clara River into Los Angeles County The origin of this syncline was closely related to movement of the Oak Ridge and San Cayetano faults, as well as the Ventura Avenue anticline (see Plates B-1 and B-1B). It is probable that the Santa Clara River syncline was initially folded without faulting, and faulting occurred later when the sides of the fold became fairly steep (Reed, 1933). Of interest here is the fact that the waterbearing San Fedro formation has been folded in the Santa Clara River syncline, resulting in erosion and exposure of the upturned edges so that ground water-ts now replenished by surface water . The north flank of the folded San Pedro formtion is exposed from the ocean to a point about three miles east of Santa Paula and may be recharged by rainfall penetration and stream percolation. The south flank of the syncline may be partially eroded and covered by alluvium along the so of the Santa Clara River and partially covered by older formations which have been thrust up along the Oak Ridge fault.

Montalvo Anticline. The Montalvo anticline, which affects the San Pero formation, extends from the ocean up the south side of the Santa Clara River, crosses the river near Montalvo, and continues eastward south of the Saticoy fau. The Montalvo anticline appears to be cut off by the Saticoy or Oak Ridge fault

ar the west end of Oak Ridge. This anticline is shown on the geologic map late B-lB) as a single continuous fold, but may consist of two or more inditual anticlines. The area in the vicinity of the anticline is complicated by nor faults and folds and it is difficult to determine details of the structure i their effect on the movement and occurrence of ground water. It seems clear at the Montalvo anticline has folded the San Pedro formation so that it has en eroded and covered by alluvial gravels in Oxnard Forebay Basin. As a reit, some aquifers of the San Pedro formation are most likely in hydrologic ntinuity with ground water in the overlying alluvium.

Folds South of Oak Ridge. The folds in the area south of Oak Ridge in a Las Posas area (see Plate B-1C) affect the principal aquifers there. These lds result in the aquifers being exposed in certain areas where they can be harged by surface waters and buried in other areas where water wells can be illed into them. The upturned edges of the aquifers and in some cases the ests of anticlines serve as ground water storage reservoirs for the deeper porms of the aquifers. Change of storage probably occurs in the Fox Canyon aqui-' in portions of the Long Canyon, Moorpark, and Camarillo anticlines. In the eper synclinal areas ground water within the aquifers is usually confined by "rlying silts and clays.

CHAPTER B-V. GEOLOGIC HISTORY

The geologic history of Ventura County has been very complex. Portion of the area have been repeatedly covered by the sea and then uplifted, while othportions have been below sea level nearly all the time. A few areas have been generally above sea level so that sediments were not deposited on them. The Ter tiary history of Ventura County has been closely related with the history of a larger region which includes much of Santa Barbara County, the Channel Islands, and a portion of Los Angeles County, and is designated Ventura Basin in this report. The term basin as used here means geologic basin. The northern portion of this large area has been called the Santa Barbara embayment. The Channel Islands and parts of the Santa Monica Mountains have been called Anacapia by Reed (1933) and Reed and Hollister (1936), but for purposes of discussion the two area are discussed here as one. Ventura Basin during most of Tertiary time was a brol east-west trending downfolded belt, with gentle uplifting occurring to the north and south. History of events prior to Tertiary time is obscure due to lack of eposures of pre-Tertiary rocks. The axis or deepest portion of the basin where t; thickest sediments are found varied during Tertiary time. During Eocene time th axis was located in the northern portion of the County. In Oligocene time, the axis appears to have been along the central part of the County, and in Miocene time the major axis appears to have been in the south portion in the Santa Monic Mountains, with perhaps a secondary basin existing in the central portion of the County. During Pliocene and Pleistocene time the axis near the central part of the County remained the most prominent. It appears to have migrated slowly sout. ward, so that at the present time the deepest part of Ventura Basin coincides wil the axis of the Santa Clara River syncline (see Plate B-1B).

Since Eocene time, much of the northern portion of Ventura County has been eroded. The eroded area has grown larger as the axis moved southward and the northerly areas were uplifted. In the Santa Monica Mountains great thick-

isses of volcanics and marine sediments accumulated during Miocene time, but e area was apparently uplifted in the Pliocene since no Pliocene sediments have en found there. These flexures in the earth's crust were accompanied by faultis and gentle folding of the sediments. In middle Pleistocene time, however, Ilding and faulting was accelerated during the Santa Barbaran orogeny. This ddle Pleistocene orogeny resulted in overturning of folds, subsequent breaking the folds into thrust fault, (Reed 1933), and the development of geologic feares essentially as they are today. Evidence is available that thrusting occurd first from the south and later from the north (Herron 1953). After the mideistocene orogeny the land was eroded into gently rounded hills and mountains, ile upper Pleistocene deposits were still being deposited in the valleys. Durg upper Pleistocene time, however, fluctuations in sea level, possibly related world wide glaciation, caused changes in base level of the streams. During riods of low sea level, renewed erosion of portions of the valley fills and the lling uplands occurred. As the water level rose after the last glacial period. reams deposited their loads and filled the valleys once more.

The latest events appear to include the following: (1) continued tivity along certain faults; (2) continued folding such as the folded terraces the Ventura River drainage area (Putnam 1942); (3) renewed downcutting and adward erosion of streams, which may be due to a combination of man's alternation natural conditions and climatic changes.

The events since the mid-Pleistocene orogeny have had considerable effect development of the submarine canyons. It is not known if the canyons existed fore that period. Whether they did or not, however, it is possible that the ad of sediments carried by the ancient Santa Clara River may have been of conderable importance in present development of the canyons. During the upper eistocene, the Santa Clara River continually shifted its course so that at varus times all parts of the Oxnard Plain were covered, and the river probably

discharged at various times into the ocean at all points along the coast from the Santa Monica Mountains to its present position. Discharge of sediments onto the ocean floor in the area of the steep east-west submarine scarp south of the Coastal Plain may have resulted in submarine landslides, mudflows, and turbidity currents, which once started would continue from time to time as river sediments and transported beach sediments were dumped into the head of the conyons. The ocean floor west of Oxnard has a gentle slope, and it is probable that caryons have not been started there due to low velocities of turbidity currents and other transporting agents. It would appear that the submarine area west of Oxnard has been aggrading while the area to the south of Oxnard has been undergoing degradation and dissection which has been at least in part responsible for the formation of the submarine canyons.

CHAPTER B-VI. GROUND WATER STORAGE AND SUBSURFACE FLOW

The purpose of this chapter is to explain the procedures used to deter-

Ground Water Storage

Ground water is stored within the interstices of sediments and in racks or fractures of solid rocks. The changes in ground water storage occuring over selected periods of study were estimated for the more important ground ater basins within the County. Results of these studies are discussed in Chapter I. In general, the procedures of estimation required first a determination of he change in the volume of saturated sediments that occurred over a selected tudy period and second an estimate of the percentage of this volume that conained extractable ground water. The first factor was obtained by computing the olume of sediments that lay between the water tables that existed at the start nd close of the study period; the second factor from evaluating the average eighted specific yield of the sediments between water tables from available ell logs. Storage changes over the periods of study were computed by multiplyng changes in volume of saturated sediments by average weighted specific yield.

pecific Yield

The specific yield of a sedimentary deposit is the ratio of the volume of water which it will yield by gravity after being saturated, to its own volume, mustomarily expressed in per cent. In its South Coastal Basin Investigation, the Division of Water Resources conducted extensive field and laboratory investigations for the purpose of assigning specific yield values to various types of material appearing in well logs. These procedures are described in Bulletin No. 5 "Geology and Ground Water Storage Capacity of Valley Fill" (Division of Water Mesources, 1934). With slight variations, the values determined in this earlier

work and Bulletin 46 "Ventura County Investigation" were adopted for compiling the change of storage estimates presented here.

The task of assigning specific yield values to the sediments appearing in logs was simplified by dividing all basin sediments into eight general categories. These included soil, clay, clay-sand, clay-gravel, tight sand, sand tight gravel, and gravel. Sand, gravel, and clay, which constitute the bulk of the basin sediments, were generally found to be well differentiated on the driller's logs. Combinations of these materials, however, were frequently described by such unique terms as "ooze", "muck", "cement", etc. Materials so described were placed, based on the judgment of a geologist, into one of the above eight categories. Table B-1 indicates specific yield values assigned to the general categories. In certain instances, these values were altered slightly whenever field observations indicated the advisability of changes.

TABLE B-1

SPECIFIC YIELDS OF SEDIMENTS

| Material | Specific Yield (Per Cent) | |
|---|---------------------------|--|
| Soil, including silty clay Clay, including adobe and hard pan | 3 | |
| Clayey sand, including sandy silt Clayey gravel Sand | 5 7 25 | |
| Tight sand, including cemented sand Gravel, including gravel and sand Tight gravel, including cemented gravel | 18 21 14 | |

Selection of Increments

Each ground water basin was subdivided into smaller areas. Units of 100 acres were adopted in the larger basins where well logs were abundant and larger areas were used in basins for which little data were available. The sediments underlying each such subarea were separated at selected depth intervals. In this manner, each basin was divided into zones, the storage capacity of which
culd be conveniently estimated. The change in ground water storage for each etire basin was then computed as the sum of the changes occurring within the znes.

Subsurface Flow

Two methods were used to determine subsurface flow. These were the sope-area method and the rising water method.

Sope-Area Method

The slope-area method is based on the commonly used form of Darcy's law, PAI, where Q equals subsurface flow in gallons per day passing through the coss-sectional area A in square feet; P is permeability in gallons per day per suare foot; and I is slope of water table at the cross-section in feet per foot. Lis method is fairly reliable in cases where cross-sectional area can be deterned from well logs, permeability can be estimated by pump tests, and the ground iter slope can be accurately measured.

sing Water Method

The rising water method of computing subsurface flow is applicable were rising water occurs perennially and where the cross-sectional area of sturated sediments is unknown. The method has been mentioned by Tolman (1937) and a variation of the method was used by Kimble (1936). The rising water method also based on Darcy's law.

Let Qt equal total subsurface flow past a cross-section of the valley 11 at the point of zero rising water, X, upstream from the point of maximum .sing water, Y (see diagram below). Let Qu be the subsurface flow at the point 'maximum rising water and Qr the maximum rising water at any time. Assuming nat little or no water is lost by diversion or evapo-transpiration between X and Y, and that steady flow conditions exist it follows that:

Qt = Qu + Qr

Qt = PAI

At point X the total subsurface flow may be expressed as

and

Qu + Qr = (PA) I

where

P = permeability

A = cross-sectional area

I = slope of water table at point X, as measured in

wells just upstream from X.

It is assumed that:

1. Flow is horizontal.

- 2. The materials are essentially homogeneous.
- The change in cross-sectional area due to change in water levels is negligible.
- 4. Permeability is constant throughout the section.

Under these assumptions, the variations in Qr must be dependent only upon I.

Two periods t_1 and t_2 may be taken so that

and

 $Qu + Qr_1 = (PA)I_1$ (1) $Qu + Qr_2 = (PA)I_2$ (2)

Subsurface flow or Qu is nearly constant as long as any rising water occurs, since I at Y is essentially determined by the surface of the stream. Dividing (1) by (2)

$$\frac{Qu + Qr_1}{Qu + Qr_2} = \frac{PA (I_1)}{PA (I_2)}$$

and solving for Qu

$$\frac{J_{u} = I_{1}(Qr_{2}) - I_{2}(Qr_{1})}{I_{2} - I_{1}}$$

The maximum rising water, Qr, must be measured. Error may be intronced if water losses are not taken into account or if the stream is not measured t the point of maximum rising water. Rising water measurements obtained by oth the Division of Water Resources and the Ventura County Water Survey were sed to compute subsurface flow in the basins of the Santa Clara River by this ethod.



Qt = PAIQt = Qr+Qu In order to complete the hydrologic studies of the Santa Clara River basins it was necessary to estimate the decrease in subsurface flow when water level of the basins were drawn down and rising water stopped. In order to do this the logarithm of rising water plus previously estimated subsurface flow wa plotted against basin storage depletion and found to be nearly a straight line. This line was then projected past the point where zero rising water occurred an the projected line was used to estimate subsurface flow for conditions of zero rising water. This method of estimating subsurface flow with zero rising water is based on the assumption that the storage depletion of the basin and the slop of the water table upstream from the area where rising water occurred are relati, and that the change of wetted cross-sectional area due to change in ground wate level is negligible. In general, it was found that these factors were related during the period of record, so that the assumption appears to be valid.

Permeability

Pump tests to determine permeability were conducted where possible. These data are summarized in Table B-2. Permeability was computed using nonequilibrium methods as outlined in the Division of Water Resources "Draft of Report of Referee" No. 506806, of the West Coast Basin, February, 1952. Slight variations in methods as suggested by Wenzel (1942) and Jacobs and Cooper (1944 were used where necessary. In general, the recovery and the drawdown methods were used depending on conditions. These methods depend on time-rate of recovery after pumping stops or time-rate of drawdown during pumping. The last column of Table B-2 indicates relative reliability of results due to conditions at the time of the tests.

TABLE B-2 SUMMARY OF PERMEABILITY PUMP TESTS

| MethodBashn and AquiferTransmissibilityof AquiferGailons per layStorageStorageConditionsdownPleasant Valley - San Fedro fin.54,900280220043plordownPleasant Valley - San Fedro fin.54,900280220043plordownPleasant Valley - San Fedro fin.54,900280200043plorveryOnmard Bain - San Fedro fin.193,0001001,990doodveryOnmard Bain - San Fedro fin.264,0001002,640doodveryOnmard Bain - San Pedro fin.120,000280200096FairveryBast Las Posas - Fox Canyon aquifer177,300426416dooddownMond - San Pedro fin.220,000280780.0096fairdownUpper Verture356,0002803383,600PoorveryPiru-Fillmore1,220,0003383,600PoorveryPiru-Fillmore1,220,0003383,600Poor | | | • | | Thickness : | Permeability : | | |
|---|---------|--------|---|---|-------------------------------------|---------------------------------------|------------------------|-----------------------------------|
| omPleasant Valley - Older alluvium, san Fedro fm., and volcanics54,300280220.043Poor outsryOnder alluvium, and volcanics193,0001001,980600sryOmard Bain - Omard Aquifer264,0001001,980600sryOmard Bain - Omard Aquifer264,0001002,640600sryOmard Aquifer120,000600200.096FairomTherra Rejada - Interna Rejada -177,300426416600dsryEast Las Posas - Fox Canyon aquifer220,000280780.0096ForomMound - Ran Fedro fm.220,000280780.00056(composite of t weills)omUpper Venture River - alluvium1,220,0003383,600PooreryFiru-Fillmore1,220,0003383,600PooreryFiru-Fillmore1,220,0003383,600Poor | t Mé | ethod | Basin and B | Transmissibility Gallons per Day per Foot | of Aquifer: from : Well Log : | Gallons per Day per Square Foot | Storage Coefficient | Conditions of Test |
| erry Omard Flain - Omard aquifer 193,000 100 1,980 Good erry Omard aquifer $264,000$ 100 $2,640$ 600 ferry Omard aquifer $264,000$ 100 $2,640$ 600 form Therra Rejada - $120,000$ 600 200 $.096$ Fair form Fast Las Posas - $177,300$ $4,26$ $4,16$ 600 for Fast Las Posas - $177,300$ $4,26$ $4,16$ 600 for Fast Las Posas - $177,300$ $4,26$ $4,16$ 600 for for $200,000$ 280 780 $.0007$ 600 down Upper Venture $36,000$ 200 $1,800$ $.00056$ $fair down Upper Venture 1,220,000 338 3,600 Poor san Pedro fm. 1,220,000 338 3,600 Poor $ | awd | имо | Pleasant Valley - Older alluvium, San Pedro fm., and volcanics | 54, 800 | 280 | 220 | •043 | Poor |
| veryOxnard Plain - Oxnard aquifer $264,000$ 100 $2,640$ $$ 600 downTierra Rejada - volcanics $120,000$ 600 200 $.096$ FairveryEast Las Posas - Fox Canyon aquifer $177,300$ 426 416 $$ 600 downMound - San Fedro fm. $220,000$ 280 780 $.0067$ $Foor$ downWourd - San Fedro fm. $220,000$ 280 780 $.0007$ $Foor$ downUpper Venture River - alluvium $36,000$ 20 $1,800$ $to 0.256$ $(composite of the vertage)$ veryPiru-Fillmore $1,220,000$ 338 $3,600$ $$ $Poor$ | Ö | very | Oxnard Plain - Oxnard aquifer | 198,000 | 100 | 1,980 | 1 | Good |
| down Tierra Rejada - 120,000 600 200 .096 Fair volcanics volcanics I 77,300 426 416 $Good$ $Good$ $Fox Canyon aquifer 220,000 280 780 .0007 Poor San Fedro fm. 36,000 20 1,800 1,900 1,800 1,800 1,800 1,800 1,800 1,800 1,800 1,800 1,800 1,800 1,900 $ | ະ ເ | very | Oxnard Plain - Oxnard aquifer | 264,000 | 100 | 2,640 | | Good |
| veryEast Las Posas - Fox Canyon aquifer177,300426416GooddownMound - San Fedro fm.220,000280780.0007PoordownUpper Ventura River - alluvium36,000201,800.00056Fair 4, wells)veryPiru-Fillmore1,220,0003383,600Poor | ģ | иморм | Tierra Rejada - volcanics | 120,000 | 600 | 200 | 960• | Fair |
| down Mound - 220,000 280 780 .0007 Poor San Pedro fm. 36,000 20 1,800 .00956 Fair Niver - alluvium (average) 20 1,800 .0025 (Composite of to 0.25 (Composite of the wells) San Pedro fm Poor | 0 | overy | East Las Posas - Fox Canyon aquifer | 177,300 | 426 | 914 | | Good |
| down Upper Ventura 36,000 20 1,800 .00956 Fair River - alluvium (average) 20 1,800 to 0.25 (Composite of h wells) very Piru-Fillmore 1,220,000 338 3,600 Poor San Pedro fm. | ត្ត | mobw | Mound - San Pedro fm. | 220,000 | 280 | 780 | 4,000. | Poor |
| very Piru-Fillmore 1,220,000 338 3,600 Poor San Pedro fm. | e E | uwdown | Upper Ventura River - alluvium | 36,000 (average) | S | 1,800 | .00056 to 0.25 | Fair (Composite of 4 wells) |
| | 0 | covery | Piru-Fillmore San Pedro fm. | 1,220,000 | 338 | 3,600 | r . | Poor |

CHAPTER B-VII. DESCRIPTION OF GROUND WATER BASINS

Seventeen ground water basins delineated in Ventura County in the course of this investigation are discussed in the following paragraphs. The boundaries of these basins are shown on Plate 11. In addition, there is discussed herein an additional basin known as Eastern Basin, which is located in Los Angeles County but which affects the regimen of flow in the Santa Clara Rive in Ventura County. Several further ground water bodies which presently yield bu little water, are discussed under the name of the locality in which they occur, namely, Malibu hydrologic unit, Rincon subunit and Rincon Creek drainage area, Cuyama River drainage area, and upper portions of Piru Creek drainage.

The boundaries of ground water basins in most instances conform with geologic features such as contacts between permeable and impermeable formations, fault zones of low permeability, or changes in subsurface lithology which affect movement or mode of occurrence of ground water. These boundaries were establish from available data, including well logs, areal geology, and hydrologic observations.

In general, there are three types of ground water basins in the County These include (1) basins composed of unconsolidated sediments or alluvium, (2) volcanic rock basins, and (3) basins composed of consolidated rocks. The first type of basin comprising unconsolidated sediments or alluvium has been further divided into two sub-types: the simple type basin in which ground water occurs in a single unconfined body, and the complex basin in which ground water occurs in more than one aquifer. The characteristics of these types of basins are summarized in Table B-3.

| BASINS |
|-----------------|
| WATER |
| GROUND |
| OF |
| TYPES |
| OF |
| CHARACTERISTICS |

TABLE B-3

| : Water table conditions t at wells | ly heterogen- rmeable zones de to form one | or more aqui- ently in well ih zones. Frequently confined. May be part- fy uncon- fined or both. | cure may be Generally ater occurs unconfined. . Highly nes produce |
|---|--|--|--|
| Nature of | Lithological eous, but pe interconnect water body. | Usually two fers, freque defined dept Some free g areas. | Basin struct complex. We in fractures fractured zo |
| Type of wells and general characteristics | Irrigation mainly. Many wells, yields generally high. | Irrigation mainly. Many wells, yields generally high. Com- monly show pressure effects. | Domestic and limited irrigation. Wells numerous with moder- ate vields. |
| Basin or subunit | Ojai Upper Ojai Ventura River Basins Simi Piru Fillmore Santa Paula Oxnard Forebay Miscellaneous Areas | Mound Oxmard Plain Pleasant Valley East and West Las Posas Santa Rosa Miscellaneous Areas | Tierra Rejada Parts of Conejo Basin Parts of Malibu Hydro- logic Unit. |
| | Simple | Complex | IS |
| Type of basin | Unconsolidated Sedimentary or | Alluvial Basins | Volcanic Rock Basin |

TABLE B-3 (Continued)

CHARACTERISTICS OF TYPES OF GROUND WATER BASINS

| : Water table : conditions : at wells | Ly Confined, unconfined, or both. | |
|---|---|---|
| Nature of basin fill | Structure may be extremel complex. Water occurs in fractures and/or inter- stices. | |
| Type of wells and general characteristics | Domestic, stock watering, and limited irrigation. Few wells-yields gener- ally low. | |
| : Basin or subunit | Parts of Ventura River Subunits Simi Subunit (Hill areas) Parts of Conejo Basin Parts of Malibu Hydro- logic Unit Miscellaneous Areas | |
| Type of basin | Consolidated Rock Basins | a |

Ground water basins of most economic importance in Ventura County are tose possessing a combination of sufficient replenishment, sufficient storage vlume to regulate the water supply, and materials which will readily yield wter to wells. In general, the unconsolidated sedimentary basins are of geatest economic significance.

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Ground Water Basins Within Ventura Hydrologic Unit

Ground water basins in the Ventura Hydrologic Unit include Upper Ojai, Ojai, Upper Ventura River, and Lower Ventura River Basins. Locations of the basins are shown on Plate 11, and physical characteristics are summarized in Table 9. Other portions of the Ventura Hydrologic Unit contain relatively small areas of shallow water-bearing materials. Water wells in such areas are usually shallo and have low yields. Some wells in these areas obtain water from the fractures and pervious zones within consolidated Tertiary sediments.

Plate B-1A (Areal Geology) depicts the details of structure and extent of formations within this unit.

Upper Ojai Basin

This basin lies within the Upper Ojai Subunit. It comprises a surface area of about 1,950 acres situated in the northeasterly portion of Ventura Hydrologic Unit. Elevations vary from 1,200 feet to 1,600 feet above sea level. Surface waters drain westerly through Lion Canyon into San Antonio Creek and easterl along Sisar Creek to Santa Paula Creek.

<u>Geology</u>. The water-bearing materials comprising the ground water basir include Recent and Pleistocene stream deposited gravel, sand and clay, and to a limited extent deeply weathered older formations. The alluvial materials are known to exceed 300 feet in thickness at a well near Sisar Creek, though their average thickness is estimated to be about 60 feet. The Tertiary sediments that flank the basin consist of consolidated marine and continental sandstone, conglomerate, and shale, and contain water of poor quality in some areas. These Tertiary sediments include the Pico, "Santa Margarita", Modelo, Rincon, Vaqueros, and Sespe formations, all of which are folded and faulted.

Occurrence of Ground Water. The alluvium comprises the principal surce of ground water. In general, ground water is unconfined and surface ters percolate relatively freely to the water table. Ground water also occurs i fractures in the consolidated Tertiary sediments which flank and underlie the aluvium. There are a few springs supplied from such fractures in the rocks. Is twells in the basin penetrate the alluvium and bottom in the older rocks. e alluvium underlying much of the basin drains rapidly to Lion Canyon and Sisar eek and consequently only a few feet of water accumulate above the base of ese materials.

Movement of Ground Water. The ground water in Upper Ojai Basin divides, rt flowing eastward to Sisar Creek and the remainder flowing westward into Lion nyon. The location of the ground water divide and the directions of movement e depicted by contours on Plate 16-A.

Replenishment and Depletion of Ground Water. The principal sources of ound water replenishment are deep penetration of precipitation, percolation of rface water in streams, and percolation of the unconsumed portion of water plied for irrigation and other uses. Ground water is depleted by pumped exactions, consumptive use of phreatophytes, and drainage of springs into Sisar eek and Lion Canyon.

<u>Subsurface Inflow and Outflow</u>. Consolidated Tertiary formations flank e alluvial deposits, preventing appreciable subsurface outflow to Ojai Basin. rings exist near the base of the alluvium in both Sisar Creek and Lion Canyon, ough this water leaves the basin as surface flow.

Ground Water Storage Capacity and Specific Yield. Alluvium in this sin is relatively shallow, with an average estimated depth of about 60 feet. ter in these sediments drains rapidly to springs situated at lower elevations,

as indicated in the preceding paragraphs. Though no estimates were made, the usable storage capacity is believed to be small.

<u>Yield of Wells</u>. The yield of wells ranges from about 10 to 200 gallons per minute, with an estimated average yield of about 50 gallons per minute.

Ojai Basin

This basin, with an areal extent of about 6,040 acres, lies in the northerly portion of the Ventura Hydrologic Unit and to the northwest of Upper Ojai Basin. Elevations vary from about 700 feet to over 1,200 feet above sea level. Surface waters within this basin drain to the southwest in San Antonio Creek toward the Ventura River.

<u>Geology</u>. Rock types of Ojai Basin consist of permeable stream deposite of Recent and Pleistocene age flanked and underlain by older consolidated sedimer tary formations as shown on Geologic Sections E-E' and F-F' on Plate 12-A.

The alluvium consists chiefly of gravelly clay and gravel up to 700 fee in thickness. Boulders and pebbles in the gravel consist chiefly of sandstone or conglomerate derived from the surrounding drainage area. Well logs show less clay in the easterly end of the valley than in the vicinity of the City of Ojai. Only a few well logs are available in the area west of the city, but here the low yield of wells indicates that the clay content is probably high. Samples of the deeper alluvium obtained during the drilling of wells appeared to be strongly weathered. The alluvium of Recent age is composed of about 50 to 100 feet of sediments similar to those occurring in the underlying Pleistocene alluvium thou usually less weathered. Distinction between the two formations is usually difficult.

The older Tertiary formations including the Modelo, Rincon, Vaqueros, Sespe, Coldwater, and Cozy Dell are usually consolidated or cemented, and may contain water of poor quality.

Ojai Valley is essentially a down-faulted and folded area, which has been filed with permeable stream deposits consisting of the Recent and Pleistoone alluvium. Surface exposures of the Pleistocene alluvium indicate that the bis dip from 10 to 30 degrees.

<u>Occurrence of Ground Water</u>. Ground water occurs within alluvial deposits at to some extent in fractures and interstices of the flanking Tertiary formatons. Ground water is generally unconfined except in the southwestern portion the basin where flowing wells during periods of high water level indicate local enfined conditions. Percolation of surface water to the water table appears to relatively unrestricted in the balance of the area.

<u>Movement of Ground Water</u>. Directions of ground water movement are shown water level contours on Plates 14-A, 15-A, and 16-A. In general the movement toward the south and west converging on San Antonio Creek near its outlet from ai Valley. During periods of low water levels (Plate 16-A) the water table slope reversed in the southwest portion of the basin.

There are no known barriers to movement of ground water in this basin.

Replenishment and Depletion of Ground Water. Ojai Basin is replenished percolation of surface water in stream channels, by water diverted from Matija Dam into the Ojai spreading grounds, by deep penetration of precipitation and rcolation of the unconsumed portion of water applied for irrigation, and to a ight extent by subsurface inflow from the surrounding Tertiary formations. The sin is depleted by pumped extractions, by consumptive use of phreatophytes, and reffluent discharge into San Antonio Creek.

<u>Subsurface Inflow and Outflow</u>. Ojai Basin is isolated from Upper Ojai isin by nonwater-bearing Tertiary formations which prevent the movement of appreable quantities of ground water between these basins. At the westerly end of

Ojai Valley near the outlet of San Antonio Creek, bedruk is exposed along the creek bottom. A short distance northeast of this outl., normwater-bearing sedi-1 ments of the Sespe formation are exposed in a small hil. North of this hill there are a few deep wells in the valley floor which yield cly sall amounts of water although their logs indicate only sedimentary materia. These observations suggest that at the westerly end of Ojai Valley, a relat ely thin mantle of allu-3.0 vium overlies essentially nonwater-bearing Sespe form ion. Subsurface outflow · 1 from Ojai Valley to Ventura River Basin is therefore obably insignificant. 1

2. Ground Water Storage Capacity and Specific inld. Specific yield and 250 storage capacity of the water-bearing materials and canges in storage occurring over selected study periods were computed in the man r described in Chapter B-VI, 127 and results are discussed in Chapter II. Total usab storage capacity of Ojai Basin is estimated to be about 70,000 acre-feet.

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Yield of Wells. Wells in the alluvial bash generally yield from 100 to 600 gallons per minute with a range in specific cpacity of from three to twenty. Wells tapping the older surrounding format ns usually yield from two to five gallons per minute but occasionally yield as much as 50 gallons per minute. The specific capacity of wells in the older formations is usually very small.

Upper Ventura River Basin

Upper Ventura River Basin lies within the Upper Ventura River subunit ind of which includes the Ventura River Valley, the Coyot Creek drainage area, and that portion of the San Antonio Creek drainage area tha lies downstream from Ojai S' mi y Valley. The basin ranges in elevation from 200 to ore than 800 feet above sea 1 level, and consists of about 4,990 acres underlai by alluvium. Water-bearing Ziz alluvial deposits of very limited areal extent als occur along Coyote Creek and 120 San Antonio Creek, but these deposits are not consdered to be of sufficient size E: 55 to be regarded as ground water basins.

Geology. The al vium consists of Upper Pleistocene and Recent depos-TEZ ts of gravel, sand, and cla. Well logs indicate the alluvium of Ventura River 10:0 alley to vary from 60 to 10 feet in depth. In the San Antonio and Coyote Creek EL C reas, it apparently varies rom 5 to 30 feet in depth. These deposits are lanked by Tertiary sediment consisting of marine and continental sandstone, conlomerate, and shale, and inlude the Rincon, Modelo, Vaqueros, Sespe, Coldwater, a nd Cozy Dell formations. The Sespe formation underlies a large portion of the loyote Creek drainage area and is composed chiefly of well cemented sandstone with intercalated lenses of congluerate. The sandstone is locally fractured. The onglomerates occur as both porly and well cemented deposits. It is often diffi-- ult to distinguish the conglmerates from overlying alluvium in well logs. Some ater wells penetrating the Mdelo formation obtain water of inferior quality.

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The Ventura River i an antecedent stream that has cut across the rejional structure and does notflow along a structural trough. The Tertiary rocks are generally folded and fauled, but the Recent deposits are relatively undisurbed.

Occurrence of Grount Water. Ground water occurs in alluvium and to some watent in the fractures and iterstices of the Tertiary formations. In general, free ground water conditions pevail throughout the entire subunit. However, locally confined bodies of ground water may exist. While wells in the Sespe formation are being drilled, the waer level occasionally rises, indicating the existince of localized confined bodies of ground water.

211 Movement of Ground W.er. Ground water moves through the alluvium following the slopes of the surfa drainage, ultimately g Lower ntura River Basin below Foster Park. Direcions of sely re mor by ground water contours on Plates 14-A, and 16 ated le f as crossing this basin in an est-we ce is. I ctior

suggesting that they cut the alluvium or form barriers to movement of ground water.

Replenishment and Depletion of Ground Water. Ground water is replenished chiefly by percolation from the Ventura River and to a lesser extent by per colation of direct rainfall and the unconsumed portion of water applied for irrigation and other uses. A slight amount of recharge is probably derived from subsurface inflow through the flanking Tertiary formations. Ground water is deplete by pumped extractions, consumptive use of phreatophytes, effluent discharge, and subsurface outflow.

<u>Subsurface Inflow and Outflow</u>. Subsurface inflow is practically negligible being limited to seepage through fissures and pores in the Tertiary formations. In 1906, the City of Ventura constructed a partial subsurface barrier in the alluvium of the Ventura River near Foster Park. The purpose of the barrier was to create rising water to be diverted for domestic and irrigation purposes. The easternmost end of this barrier was not completed, and a perennial subsurfac' flow exists around this end. This flow was estimated by the slope area method ' described in Chapter B-VI and was found to vary between 75,000 and 100,000 gallo) per day or about 100 acre-feet per year.

Ground Water Storage Capacity and Specific Yield. Total storage capacy of the basin is estimated to be about 10,000 acre-feet. The average specific yield of the contained sediments is estimated to be about eight per cent.

<u>Yield of Wells</u>. Irrigation wells in the alluvium yield about 600 gall per minute with specific capacities ranging from 10 to 200. Both well yield and specific capacity are influenced by the regimen of the Ventura River. Following cessation of surface flow in the river, both yields and specific capacities fall below the above values.

cer Ventura River Basin

Lower Ventura River Basin includes the alluvial deposits of the Ventura for that lie between Foster Park and the ocean, and the basin ranges in elevaon from sea level to about 200 feet above sea level. It has a surface area of but 2,670 acres. The valley floor of Canada Larga has been excluded from this sin, since the alluvial deposits in this valley are shallow and contain little mer. In the southern end of the basin, alluvium overlies and is probably hynulically isolated from the San Pedro formation. The San Pedro formation in is area appears to be hydraulically connected with the Mound Basin and is conlered herein a part of that basin.

<u>Geology</u>. The alluvial fill of the basin consists of gravel, sand, and y. It is surrounded and underlain by older, generally less permeable sedimenry formations. These include the San Pedro, Santa Barbara, Pico, Modelo, Rincon, queros, and Sespe formations. At its north end, the basin is connected to Upper ntura River Basin near Foster Park, and at the south end it is bounded by the ean.

Within this basin, Ventura River is an antecedent stream. It crosses e axis of the Ventura Avenue anticline near the center of the basin. Downstream om this axis, formations older than the alluvium dip southward and strike in an sterly direction. With the exception of the Pleistocene San Pedro formation, e flanking formations are generally nonwater-bearing. The San Pedro formation nsists chiefly of gravel, sand, silt, and clay.

<u>Sea-Water Intrusion</u>. No evidence available conclusively indicates that ean water has invaded the basin. The electric log of one well near the river uth shows that at the time of drilling, the alluvial deposits contained highly neralized water. It is uncertain as to whether this condition was due to inusion of sea water or to pollution from local sources. Two wells near the

river mouth tap only the underlying San Pedro formation and provide no indication of the quality of water occurring in the alluvium.

Occurrence of Ground Water. The principal water-bearing formation is the late Quaternary alluvium of the Ventura River which varies from 60 to 100 fee in thickness. Terrace and alluvial deposits flanking the main body of alluvium, Plate B-IA, are shallow and would produce only minor quantities of water.

The San Pedro formation flanks and underlies the alluvium near the mou of Ventura River. It dips about 35 degrees toward the south and strikes to the east extending into Mound Basin. Available data suggest that the San Pedro forms tion near the river mouth is at least partially hydraulically isolated from the river alluvium by relatively impervious material, possibly of lagunal or Paludal origin. The alluvium is considered to be within the Lower Ventura River Basin overlapping the San Pedro formation which belongs hydrologically within Mound Basin. This conclusion is substantiated by the following observations:

1. Static water levels in wells tapping the San Pedro formation have been above the elevation of the bed of the Ventura River indicating that ground water in the San Pedro formation is confined.

2. The electric log of one of the above wells indicated water of poor quality in the river alluvium; yet this same well has continually produced fresh water from the underlying San Pedro formation.

3. Water levels in wells tapping the San Pedro formation fluctuate in rapid response to tidal fluctuations, further indicating that ground water in the San Pedro formation is confined.

Movement of Ground Water. There are few wells situated in Lower Ventu: River Basin; consequently, no ground water contour map was constructed of this area. In general, ground water moves in a downstream direction, ultimately discharging into the ocean. No barriers to ground water movement are known to exis

Replenishment and Depletion of Ground Water. The basin is replenished r percolation from the Ventura River, by percolation of rainfall and the unconmed portion of water applied for irrigation and other uses, as well as by suberface inflow from Upper Ventura River Basin. Inflow from flanking Tertiary diments is probably small. Depletion occurs by surface and subsurface outflow, mited pumped extractions, and consumptive use of phreatophytes.

<u>Subsurface Inflow and Outflow</u>. Subsurface flow in the alluvium of the ntura River near Foster Park constitutes one of the principal sources of supply Lower Ventura River Basin. Subsurface outflow from the basin probably disarges to the ocean during periods of high ground water level.

Ground Water Storage Capacity and Specific Yield. Pumping draft on wer Ventura River Basin is negligible. There are no irrigation wells, a few andoned domestic wells, and one sump pump. For this reason, no estimates of ange in storage were compiled for this basin.

<u>Yield of Wells</u>. There are no known irrigation or domestic wells opera-.ng in Lower Ventura River Basin which obtain water from the alluvial fill.

Ground Water Basins Within Santa Clara River Hydrologic Unit

Ground water basins within the Santa Clara River Hydrologic Unit inclu Piru, Fillmore, Santa Paula, Mound, Oxnard Forebay, Oxnard Plain, and Pleasant Valley Basins. These basins are the most productive in Ventura County. Plate 1 shows the location of the ground water basins, and their physical characteristic are summarized in Table 11. Plate B-1B shows details of geologic structure and extent of formations in this hydrologic unit.

Eastern Basin

Eastern Basin lies within Los Angeles County; more explicitly it comprises the water-bearing formations of that part of the Santa Clara River Valley lying east of Ventura County. Since it is not in Ventura County, it was not studied in detail during the course of this investigation. It is discussed here because it is tributary to other basins in Ventura County. Pumping from this basin effects the regimen of both surface and subsurface flow in the Santa Clara River. The boundaries of Eastern Basin were not determined exactly, except for the boundary with Piru Basin on the west.

<u>Geology</u>. The watershed tributary to Eastern Basin contains sedimentar formations of marine and non-marine origin, some volcanic rocks, and large areas of granitic and metamorphic rocks. These formations are shown on Plate 10 and include alluvium, Saugus and Pico formations, Ridge Basin group, "Santa Margarit" Modelo, Vaqueros, Mint Canyon and Martinez formations. Of these formations, the Saugus and Quaternary alluvium are of principal interest. Water derived from

asas of non-marine formations may be fairly high in boron due to occurrence of bron minerals in these sediments.

The Saugus formation in this area consists of continental sand, clay, ad poorly cemented gravel and attains a maximum thickness of 2,500 feet. These mterials have been faulted, folded, and eroded. In the valley areas the Saugus frmation is overlain by up to 100 feet of alluvial sand and gravel with some cay and silt.

Occurrence of Ground Water. Ground water is derived principally from Alls tapping the Saugus formation and the Quaternary alluvium. It is known tat ground water within the deeper aquifers often occurs as confined water wherethat within the alluvium is usually unconfined. No attempt was made in this ivestigation to delimit the extent of the different aquifers in Eastern Basin.

<u>Movement of Ground Water</u>. No ground water contour maps were constructed 'Eastern Basin by this Division during this investigation. Ground water contour ups of this area are published in "The Annual Report on Hydrologic Data" by the os Angeles County Flood Control District.

Replenishment and Depletion of Ground Water. Ground water is repleished by stream percolation, penetration of direct precipitation and the unconumed portions of water applied for irrigation and other uses as well as imported ater released by the City of Los Angeles from Bouquet Canyon Reservoir, and also be a minor extent by subsurface inflow from the older semi-permeable formations nat flank the basin. The ground water basin is depleted by pumped extractions, consumptive use of phreatophytes, and effluent discharge.

<u>Subsurface Inflow and Outflow</u>. An undetermined amount of inflow enters ne basin from older semi-permeable formations that flank the water-bearing sedients. Subsurface outflow from Eastern Basin is negligible and is discussed in ne paragraph on subsurface inflow to Piru Basin.

Ground Water Storage Capacity and Specific Yield. No attempt was made to estimate change of storage or total storage capacity within Eastern Basin during this investigation.

<u>Yield of Wells</u>. No measurements of well discharge were made in this basin, although both irrigation and domestic wells exist. Several highly productive wells tap the alluvial sands near the river and are supplied directly by percolation from that source. One such well reportedly yields 2,000 gallons per minute. Irrigation wells are also known to tap the Saugus formation, but the yields of such wells are not known.

Piru Basin

Piru Basin ranges in elevation from 470 feet to 800 feet. The highest elevation within the watershed is 4,592 feet attained at Hopper Mountain. It comprises a surface area of about 6,520 acres. The greater portion of the ground water basin underlies the alluvial area of the Santa Clara River Valley. The eastern boundary of the basin is at Blue Cut which is located about one mile west of the Los Angeles County line. The western boundary is arbitrarily located as shown on Plate B-1B.

<u>Geology</u>. The principal water-bearing formations of Piru Basin are alluvium of Recent and upper Pleistocene age and the San Pedro formation. Rocks adjacent to the valley floor include the Sespe, Vaqueros, Modelo, "Santa Margarita", and Pico formations, all of which are essentially nonwater-bearing in this area but which may contribute water of poor quality to the water-bearing materia.

Alluvium in Piru Basin is about 85 to 200 feet thick and consists of river deposited sand and gravel of Recent and upper Pleistocene age which is not readily differentiated from the San Pedro formation. In Blue Cut at the extreme east end of the basin, alluvium is about 6 to 15 feet thick. On the south side (

Pru Basin alluvium overlies a shelf-like feature of nonwater-bearing rocks (see Gologic Section J-J', Plate 12-A).

The San Pedro formation does not outcrop in this basin, but it is toped at depth by many wells.

Water wells reach depths of up to 1,000 feet in Piru Basin and their Iss indicate that the San Pedro formation is characterized by gravel, sand, and she clay. Several water wells which were in the process of being drilled were rsited in the course of the investigation. Samples of material taken from these wells indicate that most of the San Pedro formation, to depths of 800 feet, was apposited under conditions similar to those which prevail in the Santa Clara over today. The nature of the sediments deeper than 800 feet is not well known. Emples from an oil well drilled near Cavin Road on the north side of the river st west of the center of the basin are comprised of similar materials to depths 1,000 feet. The silt content increases appreciably to 1,200 feet, the maximum pth of the well. The electric log of an oil well about one mile east of the wn of Fillmore indicates that thick interbedded sands and clays extend to depths 4,000 feet.

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The San Pedro formation has been folded along the Santa Clara River ncline. At the Oak Ridge and San Cayetano faults older rocks have been thrust er the formation. Two oil wells in the mountains about one mile north of the lley first penetrate the Modelo sandstone and shale, then the San Cayetano mult, and finally the San Pedro formation which is only gently folded. The west angle of dip of the San Cayetano fault as shown by these wells is about degrees toward the north. The Oak Ridge fault dips about 60 degrees southward e evidenced in oil wells drilled on Oak Ridge south of the Santa Clara River alley.

The trough axis of the Santa Clara River syncline is folded upward southast of the town of Piru, where the San Pedro formation has apparently been

truncated by erosion. The San Pedro formation is reduced in cross-sectional art westward from Piru to the State Fish Hatchery, where the narrowest part of the valley is located. Well log data indicate that the cross-sectional area may al, be slightly reduced by a gentle upwarp of the base of the San Pedro formation near the fish hatchery.

Occurrence of Ground Water. Ground water is generally unconfined in both the San Pedro formation and the overlying alluvium.

<u>Movement of Ground Water</u>. Ground water generally moves westward, as shown on the water level contour maps, with minor contribution from the north ai south sides of the basin (see Plates 14-B, 15-B and 16-B). The water table sloe appears to be fairly steep just southeast of Piru, and it is possible that this steep slope may be related to the upturned edge of the San Pedro formation beneath the alluvium.

Slope of the water table decreases from an area south of Piru and continues to decrease toward the State Fish Hatchery which is located on the Piru-Fillmore Basin boundary. Cross-sectional area of the water-bearing materials i the least near the Fish Hatchery where it results in a steeper water table slop. Farther west the slope decreases as the cross-sectional area increases. The point where the steepening occurs cannot be accurately determined, but its loce tion appears to vary with water levels in Piru Basin. The boundary of Piru Basin was drawn arbitrarily near the Fish Hatchery where the maximum amount of rising water usually occurs, but could also be located to the east or to the wet of the assumed boundary. It is clear that as long as a westward gradient of th water table exists, subsurface outflow will continue. Historically, this westwro gradient has always existed; although it has become less as the basin was depleed and greater as the basin was replenished. Because of the considerable depth of the San Pedro formation at the State Fish Hatchery it is likely that subsurface

fow out of Piru Basin will continue as long as water levels are higher than in Filmore Basin. There is no evidence available indicating the presence of a fult which would function as a ground water barrier, as suggested by previous ivestigators.

Replenishment and Depletion of Ground Water. Ground water in Piru Isin is replenished by percolation in stream channels and in the Piru spreading rounds, by percolation of direct precipitation and the unconsumed portion of Iter applied for irrigation and other uses, and probably by a minor amount of Ibsurface inflow from adjacent semi-permeable formations. Ground water is deleted by pumped extractions, consumptive use of phreatophytes, effluent discharge, and by subsurface flow into Fillmore Basin.

<u>Subsurface Inflow and Outflow</u>. Subsurface flow into the Piru Basin from he Eastern Basin is probably negligible because of the thin alluvial cover at the asin boundary in Blue Cut. Subsurface flow out of Piru Basin into Fillmore Basin as been estimated by the rising water method (see p. B-42) to be thirty secondeet. Subsurface flow out of Piru Basin will occur whether there is rising water r not, but will be decreased somewhat after rising water stops and as the water able is lowered and its gradient decreased.

Ground Water Storage Capacity and Specific Yield. Results of change of ground water storage estimates are discussed in Chapter II. Forty-five well logs here used to determine specific yield of the sediments. The mean weighted specific yield of the interval between the highest water levels of 1914 and the lowest of 1951 is estimated to be 16 per cent. To estimate total storage capacity of the ground water basin, it was assumed that usable depth of the ground water basin is 1,000 feet, that the average area at depth is about 6,000 acres, and that the specific yield does not change appreciably with depth. By multiplying these figures,

the storage capacity for Piru Basin is estimated to be $6000 \times 1000 \times .16$ or 960,000 acre-feet, or on the order of one million acre-feet.

<u>Yield of Wells</u>. Yield of irrigation wells in Piru Basin varies from 600 to about 2,000 gallons per minute, with an estimated average of 800 gallons per minute. Specific capacity averages about 70. Yield of wells on the shelf on the south side of Piru Basin is fairly high when water levels are high, but when the water level falls and the alluvium is dewatered the wells go dry.

Fillmore Basin

Fillmore Basin which comprises a surface area of about 16,870 acres ranges in elevation from 280 to 470 feet in the Santa Clara River channel. Maximum elevation in the immediate drainage area is 4,959 feet at Santa Paula Peak. Two prominent features of this area include Timber Canyon which reaches an eleve tion of about 2,200 feet, and the Sespe uplands north of the Santa Clara River and west of Sespe Creek.

<u>Geology</u>. Water-bearing materials underlying the basin include alluviu of Recent and Pleistocene age and the San Pedro formation. Formations adjacent to Fillmore Basin include the Santa Barbara, Pico, Modelo, Vaqueros, and Sespe which are essentially nonwater-bearing, but may contribute limited amounts of water of poor quality to the water-bearing materials. See Geologic Section H-H Plate 12-A, for general relationships of the water-bearing materials.

The alluvium comprises gravel, sand, and some clay up to 250 feet in thickness. The alluvium is difficult to differentiate from the underlying San Pedro formation in the valley floor area of the basin. A study of water well logs in the Sespe uplands indicates that the alluvium is underlain, at least in part, by the San Pedro formation. The alluvium in the Sespe uplands consists o upper Pleistocene and Recent alluvial fans which have been deposited on the

uturned and eroded edges of the San Pedro and older formations (See Geologic Sction H-H', Plate 12-A). The upper Pleistocene (or older) alluvium has been artly dissected and somewhat folded. It is being dissected in some areas and evered by Recent alluvium in others. The alluvium in the Sespe uplands comprises avel, gravelly clay, and clay and is generally reddish in color.

Timber Canyon, located about four miles northeast of Santa Paula, is noored by Recent alluvium. A large alluvial cone extends from Santa Clara River alley up the floor of Timber Canyon. The surface material on this cone is tremely coarse, poorly sorted, subangular gravel. Water well logs indicate at this coarse material contains considerable clay and that the cone is up to 0 feet thick just north of Highway 126.

On the south side of Fillmore Basin, in the Bardsdale area, the alluvium erlies a shelf of semi-permeable rocks which have been thrust up by the Oak dge fault. The alluvium overlying this shelf is up to 180 feet thick. It is obable that this alluvium includes upper Pleistocene materials.

The San Pedro formation outcrops north of the valley floor from the wn of Santa Paula to the Sespe uplands, where it is concealed beneath alluvium. e San Pedro formation dips about 40 degrees to the south near Santa Paula and comes steeper eastward until it is overturned at Timber Canyon. Cores from an 1 well northwest of the town of Fillmore indicate that the San Pedro formation relatively flat lying at depth below the gently dipping San Cayetano fault and the valley area of Sespe Creek. The structure of the San Pedro formation in e Sespe uplands is concealed by alluvium, but available data suggest that addional unknown faults complicate the geology of this area.

The San Pedro formation is about 4,000 feet thick in this basin and it insists of gravel, sand, and clay. The uppermost portion of the formation is ream deposited, probably by the ancient equivalent of the Santa Clara River. We lower and some of the middle portion of the San Pedro formation contains

marine fossils which indicate deposition in a shallow marine environment. Field inspection of the San Pedro formation on the north side of the Fillmore Basin indicates that the beds are extremely lenticular and discontinuous. Clays are fairly common, but are also discontinuous.

Structurally Fillmore Basin is a part of the Santa Clara River synclim. Along the Oak Ridge fault on the south of the basin, the semi-permeable formations underlying Oak Ridge have been thrust up against the San Pedro formation. The San Cayetano fault swings northward near the town of Fillmore and does not directly affect the ground water geology west of Sespe Creek.

The cross-sectional area of the San Pedro formation is slightly reduce by local warping of the Santa Clara River syncline east of Santa Paula, where th assumed boundary of the basin is located. Water levels and available geologic data do not indicate faulting in this area.

A complex feature exists in Fillmore Basin on the south edge of the Sesre uplands just north of Highway 126. Inspection of the older alluvial sediments in the small hills in this area indicates that an anticlinal structure is present as shown on the geologic map (Plate P-1B). Cores from an oil well drilli here indicate the presence of faulted and folded sediments at shallow depths. I is possible that an east-west trending fault which may affect ground water exist in this area, but water level contours do not indicate the presence of such a fault.

Occurrence of Ground Water. Ground water occurs in the San Pedro formtion and in the overlying alluvium and is essentially unconfined.

Movement of Ground Water. Ground water moves westerly in Fillmore Bash with some minor contribution from the south and north sides. There is a decreas in cross-sectional area of water-bearing materials from the vicinity of Sespe Creek to the arbitrary boundary between Fillmore and Santa Paula Basins. Slope

the water table decreases westward toward this boundary. Near the narrowest bint which is taken as the western boundary of Fillmore Basin, the slope of the iter table increases, decreasing again as the cross-sectional area increases ito Santa Paula Basin. The point of steepest slope appears to be variable under fferent water level conditions, and is actually a fairly wide zone rather than sharp line. This is the reason that an arbitrary boundary is used, just as itween Piru and Fillmore Basins. As far as can be determined, no cross fault nich affects ground water exists at this boundary.

Replenishment and Depletion of Ground Water. Ground water in Fillmore usin is replenished by subsurface inflow, by stream percolation, by percolation direct precipitation and the unconsumed portion of water applied for irrigation dother uses, and probably by a minor amount of subsurface flow from the San edro formation and adjacent semi-permeable formations. Ground water is depleted v pumped extractions, consumptive use of phreatophytes, effluent discharge, and absurface outflow.

Subsurface Inflow and Outflow. Subsurface inflow into Fillmore Basin om Piru Basin has been estimated by the rising water method to be about thirty econd-feet. Well log data are not sufficient to accurately check this estimate of the slope-area method. Subsurface outflow from Fillmore Basin into Santa mula Basin has been estimated by the rising water method to be about sixteen econd-feet. Subsurface outflow from Fillmore Basin will continue, even after. sing water stops, but it is likely that the underflow would decrease somewhat.

Ground Water Storage Capacity and Specific Yield. Results of ground ater storage estimates are discussed in Chapter II. One hundred five well logs are used to estimate specific yield and change of storage. The change of storage stimate includes the Sespe upland area. The mean weighted specific yield of

the interval between the highest and lowest historic water table is estimated to be 12 per cent. The total storage capacity of the upper thousand feet of Fillmore Basin is probably on the same order of magnitude as Piru Basin, or about one million acre-feet using an estimated area of 12,000 acres. The average specific yield is smaller in Fillmore Basin, but the area is larger than Piru Basin. The maximum total storage capacity of the basin is unknown because the effective depth of the basin has not been determined. This depth may reach 4,000 feet, which is the base of the San Pedro formation as found in oil wells near the town of Fillmore.

<u>Yield of Wells</u>. Irrigation wells in Fillmore Basin yield up to 2,100 gallons per minute, and their average yield is about 700 gallons per minute. Spe cific capacity of the wells varies considerably but probably averages 50. Yield of wells on the shelf on the south side of the basin and on part of the Sespe uplands are generally smaller than in the valley floor, due to limited depth of alluvium.

Santa Paula Basin

Santa Paula Basin ranges in elevation from 140 to 280 feet, although the maximum elevation in the local drainage area is 2,750 feet on Sulphur Mountain. The ground water basin underlies the flat alluvial area of the Santa Clar River Valley and comprises a surface area of about 13,520 acres. The boundary between Santa Paula and Fillmore Basins has been discussed under the description of the latter basin. Between Santa Paula Basin and Mound Basin, the boundary is also an arbitrary line as discussed below.

<u>Geology</u>. Water-bearing materials in Santa Paula Basin include the alluvium of Recent and upper Pleistocene age and the San Pedro formation. Rocks underlying the San Pedro formation and adjacent to the basin include the Santa

Brbara, Pico, Modelo, Vaqueros, and Sespe formations, which are generally semiprmeable and may contain water of poor quality.

The alluvium consists of up to 200 feet of stream deposited gravel, snd, and some clay and cannot be easily differentiated from the underlying San Edro formation. The alluvium near Saticoy and in the northwestern portion of the tsin consists of yellow silty clay overlving and interbedded with stream gravels a shown on Geologic Section G-G' of Plate 12-A. Lenticular gravels interbedded with this clay may locally yield water of poor quality to wells. The clay forms accap over the gravels and pinches out to the south and to the east. This yellow clty clay was probably deposited as alluvial fans by streams draining the area with of the basin. Similar silts are still being deposited in the area.

The San Pedro formation consists of 4,000 feet of gravel, sand, and cay. The lower third of this formation contains fossils indicating a marine rigin. The upper two-thirds is generally devoid of fossils and consists prinurily of stream deposits. Exposures of the San Pedro formation exhibit irregular bdding. Scour and fill features are common with individual gravels often grading iterally into sands or silts within a very short distance. It is probable that «treme variations also exist in the San Pedro formation underlying the valley loor, but local changes cannot be detected from drillers' logs.

The deepest water well logs indicate that gravels of the San Pedro forntion persist to depths of at least 800 feet. Oil well logs and outcrops indinte that the total thickness of the San Pedro formation may be 4,000 feet. The otal effective thickness is unknown, as far as water-bearing characteristics are oncerned, but it is probably at least 1,000 feet as indicated by one oil well log.

The most significant structural features in Santa Paula Basin are the inta Clara River syncline, the Oak Ridge fault, and the Saticoy fault. As disissed in Chapter B-IV of this appendix, it is probable that the Saticoy fault is branch of the Oak Ridge fault, although it may simply be an extension. Where it can be traced, the Saticoy fault forms the boundary between Santa Paula and Oxnard Forebay Basins, but since it cannot be traced in the bed of the Santa Clar River, an arbitary boundary was used at that location. The San Pedro formation has been cut off on the south by the Oak Ridge fault. The upturned edge of the San Pedro formation is exposed on the north side of the basin. It has been folde into the Ventura Avenue anticline and the Canada Larga syncline as shown on Plate B-1B. These structures probably affect ground water in the outcrop area of the San Pedro, but they appear to die out to the east and probably do not affect ground water in Santa Paula Basin.

Occurrence of Ground Water. Ground water occurs in the San Pedro forme tion and in the overlying alluvium. The ground water body in most of the basin is unconfined, but where clay lenses exist, confined ground water is evident from water level records of wells.

<u>Movement of Ground Water</u>. Ground water generally moves in a westerly direction as shown by ground water elevation contour maps (Plates 14-B, 15-B, and 16-B). At the west end of the basin the Saticoy fault forms a barrier impeding movement of ground water into Oxnard Forebay Basin. The effectiveness of this fault as a ground water barrier is demonstrated by a pronounced difference in water level elevation on either side of the fault. On the upstream side of this fault, near Saticoy, water levels range from 50 to 100 feet higher than the level existing on the downstream side.

Between Santa Paula Basin and Mound Basin a relatively steep gradient in water levels exists. The cause of this steeper gradient is not readily apparent. There is no distinct and sudden drop in water level as characterized by a fault barrier, and it is believed that the steep gradient is due to a decrease in the permeability of the sediments underlying the area, although there may be some faulting in the San Pedro formation.

Replenishment and Depletion of Ground Water. Ground water is replenhed by stream percolation, by penetration of direct precipitation and the unmsumed portion of water applied for irrigation and other uses, by subsurface flow from Fillmore Basin, and probably by a minor amount of subsurface inflow con older formations on the south side of the basin.

Field inspection of outcrops of the San Pedro formation reveals great regularity in bedding, and also considerable amounts of silt and clay. It can assumed that the upturned edge of the San Pedro is in hydrologic continuity th the aquifers in the same formation from which pumping occurs underlying the lley floor. Water level data and geologic control are not available for a liable estimate of subsurface inflow to the basin from the outcrop.

The ground water basin is depleted by pumped extractions, consumptive e of phreatophytes, effluent discharge, and subsurface flow into Oxnard Forebay d Mound Basins.

Subsurface Inflow and Outflow. Ground water movement into and out of nta Paula Basin was estimated by the rising water method. Sixteen second-feet s estimated as subsurface inflow from Fillmore Basin. Ten second-feet was timated as outflow into the Oxnard Forebay Basin. Additional subsurface outow also occurs into Mound Basin through the San Pedro formation and could ount to more than ten second-feet. Geologic data necessary for an accurate timate of subsurface outflow into Mound Basin are lacking.

Ground Water Storage Capacity and Specific Yield. Estimates of change storage are discussed in Chapter II. Some 67 well logs were used to obtain ese estimates. Weighted average specific yield of the sediments in the interval tween the water table elevations of 1944 and 1951 is estimated to be ten per nt. Change of storage in the outcrop area of the San Pedro formation could not determined because water level and well log data were unavailable. Such anges could conceivably be large.

Using an estimated area of 10,000 acres and a depth of about 800 feet, total storage capacity of the ground water basin was estimated to be about 800,000 acre-feet. Additional storage capacity probably exists below this depth and in the outcrop area, but data to estimate its amount are lacking.

<u>Yield of Wells</u>. Irrigation wells in Santa Paula Basin yield from 300 to 1,500 gallons per minute and average about 700 gallons per minute.

Mound Basin

Mound Basin ranges in elevation from sea level to over 400 feet and has a surface area of about 12,300 acres. It is bounded by hills to the north, Santa Paula Basin to the east, and Oxnard Plain and Oxnard Forebay Basins to the south.

<u>Geology</u>. The principal water-bearing formation in Mound Basin is the San Fedro. Other formations include the overlying alluvial deposits and the undelying Santa Barbara formation. The Recent and upper Fleistocene alluvium is characterized by yellow silty clay containing occasional lenses of sand and gravel. It varies from 100 feet to 500 feet in thickness. The yellow silty clay has been deposited as alluvial fans by streams draining the area to the north. It appears that these yellow clays grade into and interfinger with deposits of Oxnard Plain Basin along the present course of the Santa Clara River. Gravel, sand, silt and clay of upper Pleistocene age outcrop along the north edge of the basin and dip southward as much as 12 degrees. These particular outcrops are at the base of the upper Pleistocene deposits and contain marine fossils which indicate littoral deposition.

The San Pedro formation lies unconformably beneath the alluvium and outcrops in the hills north of Mound Basin where it is 4,000 feet thick. It consists of gravel, sand, silt, and clay. Marine fossils are found throughout the

sction of outcrop, but previous investigators believe that most of the upper ort of the San Pedro formation is continental in origin and that the marine fosels are reworked. The upper 500 to 1,000 feet of the San Pedro formation contins many permeable sand and gravel members. Below these members are a series beds which are predominantly silts and clays and are in turn underlain by avels. The permeable members of the outcrop may be continuous with those betath the Mound Basin. Exposures show these sands and gravels to be extremely inticular. Scour and fill features are common, and individual beds cannot be aced more than several hundred feet.

From the outcrop, the San Pedro formation extends westward under the luvium of the Ventura River Valley and into the ocean. Contours of the ocean oor indicate small irregularities which parallel the trend of the San Pedro ormation and suggest that it outcrops there. The San Pedro formation west of he Ventura River is mostly coarse gravel, with minor sands and clays. On the lust side of the Ventura River, however, the formation is mostly fine sand, silt hd clay with only minor gravels. This rapid lateral change in lithology cannot sually be traced down the dip of the beds, but where suitable exposures are bund it appears that the down dip variations are as great as the lateral ariations.

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The structure of Mound Basin is essentially that of a syncline as nown by cross-section B'-B", Plate B-1B. The San Fedro formation is folded in the Santa Clara River syncline and in the Montalvo anticline, and is displaced to the Saticoy fault (see Plate B-1B). The north limb of the Santa Clara River function is exposed in the hills north of the basin. Well log data indicate that the Saticoy fault extends a short distance westward from Saticoy, but either dies at or cannot be detected westward from a point north of Montalvo. Water well togs south of Ventura do not indicate that the Saticoy fault extends to the beach. arface exposures and well logs indicate that complex folding and faulting has

affected the area north of the Santa Clara River and south of the Saticoy fault. Cores from oil wells in and near the Santa Clara River west of Montalvo indicate that the south flank of the Santa Clara syncline dips steeply to the north. The Montalvo anticline in the area west of Montalvo appears to be assymetrical and probably overturned to the north. North and east of Montalvo the Montalvo anticline appears to be nearly symmetrical but is probably displaced by the Saticoy fault.

An excavation on the small hill due north of Montalvo reportedly exposed sediments which dip sixty-five degrees southward. This outcrop is now covered, but the sediments in the area appear to be similar to the non-marine San Pedro formation. Cores from an oil well northwest of Montalvo indicate fault: and steeply dipping sediments below 200 feet. These examples of faults and steeply dipping sediments are in themselves of little significance, since they cannot be correlated. They are important because they indicate that the waterbearing sediments in this part of Mound Basin have been involved in complex folding and faulting.

Seaward Extension and Hydraulic Continuity of Aquifers With the Ocean. A question of some importance in Mound Basin is the possibility of hydrologic continuity of the aquifers of the San Pedro formation with the ocean. The only place where this continuity may exist is west of the Ventura River where the sout dipping beds strike westward into and beneath the ocean. Slope of the offshore topography is very gentle and it is unlikely that the San Pedro formation near the axis of the Santa Clara River syncline would outcrop on the ocean floor unles the syncline were folded upward. Offshore seismic data suggests that the synclin continues seaward without such upwarping. The gently sloping sea floor is underlain by silty clay in areas where samples have been taken.
When water levels are high in Mound Basin, a seaward gradient exists, agesting subsurface outflow. Water levels fall below sea level during dry eiods, suggesting that sea water intrusion occurs. Detailed measurements of ech wells and wells in and west of the Ventura River Valley perforated in the a Pedro formation show tidal fluctuations which lag behind the ocean tides by by a few minutes. This short time lag indicates that the aquifer is affected tidal loading but does not necessarily indicate hydraulic continuity with the can. A shallow abandoned well near one of the City of Ventura's deep wells on beach showed more than an hour's time lag, indicating possible hydraulic tinuity of the minor shallow aquifers with the ocean.

Available evidence indicates, therefore, that outflow of fresh water or low of sea water is possible, but data are not available to estimate the quany. Up to the time of writing this report no evidence of sea water intrusion been found in quality of water from wells in the San Pedro formation which closest to the ocean.

Occurence of Ground Water. Wells obtain water from sands and gravels the San Pedro formation and possibly from alluvium of upper Pleistocene age. has been necessary to drill water wells in Mound Basin to depths of 400 to 500 feet in order to obtain water from these gravels. The gravels of the n Pedro formation are overlain by up to 500 feet of confining silty clay. Ls near the beach south of Ventura flow when water levels are high.

<u>Movement of Ground Water</u>. Ground water in Mound Basin generally moves stward toward the ocean as shown by the water level contours on Plates 14-B, -B, and 16-B. Movement may occur from Oxnard Forebay Basin as well as Santa ala Basin. Some movement may possibly occur from Oxnard Plain Basin and from e outcrop area of the San Pedro formation north of the Mound Basin. A water vel recorder installed on a well about 60 feet south of Highway 101 and just

west of the Ventura River showed no fluctuations as a result of pumping a well about 500 feet to the south, which has a drawdown of about 60 feet. Both wells are perforated, however, in the San Pedro formation which dips about 35 degrees southward in this vicinity. The lack of reaction in the recorder well indicates that there is limited movement of ground water across the bedding of the San Pedr formation.

Replenishment and Depletion of Ground Water. Replenishment of Mound Basin occurs by subsurface inflow from adjacent basins and from the outcrop area of the San Pedro formation, which is in turn replenished by rainfall penetration and stream percolation. The basin is depleted by pumped extractions and possibly subsurface outflow.

Subsurface Inflow and Outflow. Subsurface inflow occurs from Santa Paula Basin, from the outcrop area of the San Pedro formation and possibly from Oxnard Forebay and Oxnard Plain Basins when water levels are favorable. As discussed in the paragraphs on Lower Ventura River Basin, inflow from the alluvium of that basin is probably negligible. Some subsurface inflow from the seaward extension of the aquifers probably occurs during periods of low water level.

Subsurface outflow from Mound Basin into Oxnard Plain Basin may occur through the San Pedro formation beneath Oxnard Plain Basin when water levels are suitable, but the degree of hydrologic continuity of the San Pedro formation between these two areas is uncertain and may be negligible. Some subsurface outflow toward the ocean may occur during periods of high water level.

Ground Water Storage Capacity and Specific Yield. It is estimated that very little, or no change of storage occurs within Mound Basin. Since water leve and well log data are lacking in the outcrop area, change in storage in the San Pedro formation north of the basin could not be estimated, although such changes may be fairly large.

<u>Yield of Wells</u>. Wells in Mound Basin yield from 300 to 1,500 gallons minute from the San Pedro formation. The estimated average yield is 700 lons per minute and the average specific capacity about 70.

ard Forebay Basin

Ground surface elevations within Oxnard Forebay Basin vary from about to 150 feet above sea level and the basin occupies an area of about 6,170 acres. water-bearing sediments of Oxnard Forebay Basin are similar in several respects those of Oxnard Plain Basin except that the Oxnard Forebay is a free ground er area. This basic difference is so important that the areas have been ferentiated and will be described separately.

<u>Geology</u>. Formations in Oxnard Forebay Basin include Recent and upper istocene alluvium underlain unconformably by the San Pedro formation and, in mall area, by the Santa Barbara formation.

Alluvium of Recent and upper Pleistocene age is the most important materin the Oxnard Forebay since it forms the ground water reservoir for most of water used in Oxnard Plain Basin. The alluvium consists of up to about 400 t of river deposited gravels, clays and sand being common below 200 feet. alluvium has been deposited unconformably upon the upturned San Pedro and ta Barbara formations. Geologic Section K-K', Plate 12-B, shows that the base the upper Pleistocene has also been folded, while the upper gravels have not n appreciably disturbed, resulting in a local unconformity within the materials e designated alluvium. This interpretation of the data would suggest that some the lower part of "upper Pleistocene" alluvium may be middle Pleistocene in age, ce parts of it would have been deposited while the unconformity was being formed the middle Pleistocene orogeny; or the lower part of the alluvium is in reality er Pleistocene in age, and folding has occurred in upper Pleistocene time. The

latter possibility is preferred, since upper Pleistocene vertebrate fossils have been found near Ventura in the terrace deposits which dip 10 to 15 degrees southward. It is probable that the sediments are still being actively folded.

The upper gravels are continuous with the Oxnard aquifers of Oxnard Plain Basin. The gravels are poorly sorted and consist of cobbles and pebbles of sandstone, conglomerate, and igneous rock. They occur in a matrix of medium to coarse sand and contain small, irregular beds of silt and clay. Oxnard Forebay Basin is the apex of the large Oxnard Plain alluvial fan where the coarser materials are found. The nature of the clay capping the oxnard aquifer in Oxnard Plain Basin as it approaches Oxnard Forebay Basin is rather complex, as might be expected. In general, the clay cap interfingers with the gravels of the Forebay, the percentage of clay decreasing to zero in the Forebay. The bottom and top of the clay cap also slope downward away from the Forebay. As a result of the slope of the bottom of the cap and the interfingering with gravels, the actual contact o the free water surface with the clay cap will vary over a wide zone depending on the water levels in the Forebay.

The San Pedro formation underlies the alluvium unconformably, as shown in Section K-K', Plate 12-B, but appears to become conformable near the south and west edges of Oxnard Forebay Basin. A medium to coarse grained sand is found near or at the base of the San Pedro formation. This sand is the equivalent of the Fox Canyon member in West Las Posas Basin. The surface outcrop of the Fox Canyon member on the south slope of South Mountain continues westward into Oxnard Forebay Basin. Its outline beneath the alluvium has been traced by the use of water and oil well logs and by inspection of materials from wells drilled during this investigation. The probable extent of the Fox Canyon member is shown on the geologic map (Plate B-1B). The Fox Canyon is folded into a westward plunging anticline, the anticlinal structure of which is confirmed by deeper oil well logs. A few oil well logs indicate that the Fox Canyon member of the San Pedro formation

scontinuous into and beneath Oxnard Plain Basin, although areas of low perebility may exist which could retard flow of water toward Oxnard Plain Basin.

The nature of the San Pedro formation above the Fox Canyon member and new the upper Pleistocene alluvium is not well known. Available oil well logs nicate that other permeable beds exist above the Fox Canyon member and possibly cerlie unconformably the alluvial gravels of the Forebay. These aquifers in San Pedro formation cannot be traced by well logs into Mound Basin, but there be some continuous beds since water levels can be contoured into Mound Basin om the west end of Oxnard Forebay Basin.

The oldest formation of interest is the Santa Barbara formation of lower distocene and upper Pliocene age which consists of impervious clay and silt. Is formation underlies the San Pedro formation and both have been folded and tially removed by erosion in Oxnard Forebay Basin prior to deposition of Luvium. As a result of this folding and erosion, the Santa Barbara formation i lies immediately under the alluvium in the area between the Saticoy spreading bunds and the westernmost exposures of the formation on Oak Ridge. Several well is indicate that the depth to the eroded surface of the Santa Barbara formation prages about 75 feet but ranges up to 140 feet.

The Saticoy fault separates Oxnard Forebay Basin from Santa Paula Basin. exact location of the fault beneath the Santa Clara River east of Saticoy is known, hence the boundary there is arbitrary, but is guided by the point where rface water from Santa Paula Basin begins to percolate into the river gravels. boundary between Oxnard Forebay Basin and Mound Basin is also somewhat arbiary and has been placed along the north edge of the Santa Clara River, where ll logs indicate the approximate limit of the permeable gravels of the alluvium. boundary between Oxnard Forebay Basin and Oxnard Plain Basin has been estabshed from well logs and is the probable limit of the area where rainfall penetraon and excess applied irrigation water returns to the aquifer. This does not

necessarily coincide with the actual pressure-nonpressure boundary which has been discussed above.

Occurrence of Ground Water. Ground water occurs in the Recent and upper Pleistocene alluvium and in permeable sands and gravels of the San Pedro formation. Oxnard Forebay Basin is essentially a free ground water area, with changes of water level and corresponding changes in ground water storage occurri: in the alluvium. Apparently, the permeable zones in the San Pedro formation underlie and are in hydrologic continuity with the alluvium. The Santa Barbara formation underlying the San Pedro consists of fine silt and clay and is general impervious.

<u>Movement of Ground Water</u>. Ground water moves southwesterly in Oxnard Forebay Basin toward Oxnard Plain Basin as shown on the water level contour maps (see Plates 14-B, 15-B, and 16-B). The shape of the water table contours in the upper portion of the Forebay resemble those of a ground water mound produced by an injection well. The Saticoy fault and the eastern boundary of Oxnard Plain Basin near the Forebay may be visualized as enclosing a segment of a circle. Slope of the water table decreases away from the apex in much the same way the hydraulic gradient decreases away from an injection well. A difference in eleva tion of 50 to 100 feet occurs across the Saticoy fault in a distance as short as 500 feet.

Movement of water into Oxnard Plain Basin may be complex when water levels in Oxnard Forebay Basin are low. For example, when the westerly end of the Forebay has been lowered greatly by pumping, some water may leave the upper part of the Forebay, where water levels are high, and move into the area just south of the spreading ground, then back into the Forebay in the vicinity of the junction of Highways 101 and 101A.

Replenishment and Depletion of Ground Water. Oxnard Forebay Basin is replenished by subsurface inflow, by percolation in the Santa Clara River channel and in the Saticoy spreading grounds, and by percolation of direct precipitation and the unconsumed portion of water applied for irrigation and other uses. The brebay is depleted by subsurface outflow, pumped extractions, and probably by offluent discharge over the clay cap and consuptive use of phreatophytes during priods of high water level.

<u>Subsurface Inflow and Outflow</u>. Subsurface inflow occurs from Santa ula Basin through and possibly over the Saticoy fault. Some subsurface inflow ty occur from Mound Basin when water levels there are higher than in the Forebay. Ibsurface outflow occurs into Oxnard Plain Basin through the aquifers of the lluvium and San Pedro formation. Subsurface outflow into Pleasant Valley Basin ossibly occurs through the aquifers of the San Pedro formation, primarily urough the Fox Canyon member. Some subsurface outflow into Mound Basin probably ccurs at various times through the San Pedro formation in the area near Montalvo. nen water levels in the Forebay are above the clay cap of Oxnard Plain Basin, ome subsurface outflow may occur into the semi-perched zone overlying the clay ap.

Ground Water Storage Capacity and Specific Yield. Estimated changes of torage in Oxnard Forebay Basin are discussed in Chapter II. Estimated weighted ean specific yield of sediments between the interval of the 1944 and 1951 water evels is 16 per cent. Estimated total storage capacity of the alluvium in xnard Forebay Basin is about 300,000 acre-feet. When water levels are lowered n Oxnard Forebay Basin, water levels also drop in Oxnard Plain Basin. If water evels were drawn down so the Oxnard aquifers were entirely dewatered, then total torage of the Oxnard aquifer in Oxnard Plain and Oxnard Forebay Basins is proably on the order of 800,000 acre-feet. <u>Yield of Wells</u>. Irrigation wells in the Oxnard Forebay Basin yield from 200 to 2,000 gallons per minute, the average being of about 1,100 gallons per minute. Specific capacity of wells averages about 200.

1.1 FE.M.

Oxnard Plain Basin

2.1

The Oxnard Plain Basin ranges from sea level to about 100 feet in elevation, and occupies an area of about 46,460 acres. Included in the basin is about one-fourth of the irrigated area of Ventura County. This basin is bounded on the west by the Pacific Ocean, on the north by Mound Basin, and on the east by West Las Posas and Pleasant Valley Basins. The boundary between Oxnard Plain and Mound Basins is arbitrarily placed along the Santa Clara River.

<u>Geology</u>. Water-bearing formations in Oxnard Plain Basin include alluvium of Recent and upper Pleistocene age, the San Pedro formation of lower Pleistocene age, and to a minor extent the Santa Barbara formation of lower Pleitocene and upper Pliocene age. Formations underlying the Santa Barbara are pene trated only by oil wells and include the Pico, "Santa Margarita", Modelo, Topan, and Sespe formations as well as volcanic rocks of Miocene age.

The principal aquifers underlying Oxnard Plain Basin are shown on Geologic Sections K-K' and M-M', Plate 12-B. The most important aquifer underlying Oxnard Plain Basin is the Oxnard aquifer, which is part of the upper Pleistocene alluvium. The Oxnard aquifer is a series of river deposited gravel and is continuous with gravels in Oxnard Forebay Basin. The east and northwest boundaries of Oxnard Plain Basin coincide with the extent of the Oxnard aquifer The Oxnard aquifer is characterized by medium to coarse gravel interbedded with lenticular deposits of coarse sand and some clay streaks. Well logs indicate considerable irregularity in areal extent and thickness of clay and sand lenses, as would be expected of river deposits. The Oxnard aquifer is capped by yellow

ad blue clay and silt, the base of which ranges in elevation from sea level nar the forebay to about 130 feet below sea level near the coast. The overlying cay cap varies from 50 to 150 feet in thickness, and well logs indicate that it entains lenticular sands and gravels, which causes some increase in permeability c the cap.

The clay cap is overlain by up to 50 feet of sand and gravel, which etends to the ground surface. These permeable sediments contain semi-perched gound water and are considered to be of Recent age.

The boundary between Oxnard Forebay and Oxnard Plain Basins is placed, a noted above, to include in Oxnard Plain Basin the area where applied water des not return to the principal aquifers. As also stated, above, the pressurennpressure line coincides with the intersection of the unconfined water table wth the base of the confining clay cap, which intersection shifts laterally as wter levels in Oxnard Forebay Basin fluctuate.

The base of the Oxnard aquifer is rather poorly defined since most wter wells do not penetrate more than a foot or two of clay below the gravel. I some cases, this basal clay may be only five or six feet thick and additional gavels may be beneath. In general, however, the Oxnard aquifer is about 75 to 20 feet in thickness, and the base varies in elevation from 180 feet to 250 feet tlow sea level.

Well log control of the sediments below the Oxnard aquifer is poor. Aailable logs indicate that the base of the upper Pleistocene sediments is about 10 to 500 feet below the surface (about 200 to 300 feet below the Oxnard aquifer) ad that other aquifers of unknown areal extent and hydrologic continuity exist. I the southeast portion of Oxnard Plain Basin, a fairly continuous gravel sratum about 70 feet in thickness occurs at depths of 400 feet and extends at

least partly into Pleasant Valley Basin. Scattered well logs in other parts of Oxnard Plain Basin also indicate gravels at this depth. Near the City of Port Hueneme, however, a 600 foot water well penetrated only fine silt and clay from 300 to 600 feet, indicating that the 400 foot gravels do not underlie the entire Oxnard Plain Basin.

The Recent and upper Pleistocene alluvium is underlain by the San Pedro formation which varies from about 600 feet in thickness in the southern part of the Oxnard Plain to about 1,800 feet just south of the Santa Clara River. Only two water wells on the Oxnard Plain are known to completely penetrate the San Pedro formation. Several oil wells also penetrate it, but logs of these wells a generally poor. From a study of the available oil well logs, electric logs, and water well logs, it appears that the basal 100 to 300 feet of the San Pedro formation consists of sand and some gravel which is most likely continuous with the Fo Canyon aquifer in Pleasant Valley and West Las Posas Basins and is a potentially important aquifer in Oxnard Plain Basin. Electric logs of oil wells in the Oxna oil field, about four miles due east of the City of Oxnard, indicate that the For Canyon member consists of a series of sands containing irregular interbedded sil and clay layers. Fossils indicating shallow marine or lagoonal conditions of deposition have been found in the few wells which have been inspected by geologist: during drilling. Oil well logs suggest a thickening of the Fox Canyon northward toward the Santa Clara River, but it is extremely difficult to determine whether this member continues into Mound Basin, Some electric logs suggest that the basi part of the San Pedro formation near the Santa Clara River may contain water of poor quality. All sources of information indicate that sands and gravels occur : the San Pedro formation above the basal Fox Canyon member. The degree of hydrologic continuity of these additional pervious zones with areas of recharge, or w.h the Fox Canyon member, cannot presently be determined.

The Santa Barbara formation underlies the San Fedro formation in the Omard Plain and varies from about 2,000 feet in thickness near the Santa Clara Rier to about 800 feet in the southern part of the area. Electric logs of two where wells near Port Hueneme and Mugu Lagoon indicate that thick permeable zones my exist here and also that the lower half or two-thirds of the formation probaby contains water of poor quality. Electric logs in the Oxnard oil field area indicate fairly fresh or slightly brackish water in the formation, while electric logs in the Santa Clara River area indicate that nearly all the Santa Barbara proboly contains water of poor quality. These electric logs also indicate that the Sata Barbara formation contains few permeable zones near the Oxnard oil field and the Santa Clara River areas. The outcrop of the Santa Barbara formation on the south slope of Oak Ridge adjacent to this basin similarly contains few permuble zones.

Structure of Oxnard Plain Basin is relatively simple. The water-bearing meerials are generally nearly flat lying, and are not known to be affected by fulting. Total thickness of alluvium and the San Pedro formation south of the Suta Clara River is about 2,000 feet, while north of the river in the Santa Clara Rver syncline the total thickness is over 4,000 feet. See Geologic Section B-B", Pute B-1B, for structure of the area.

Seaward Extension and Hydraulic Continuity of Aquifers With the Ocean. The relationship of the Oxnard and Fox Canyon aquifers with the offshore topoguphy and geology is of considerable interest in arriving at an understanding of the ground water hydrology of Oxnard Plain, Oxnard Forebay, and Pleasant Valley Bains. The pertinent submarine topographic features are discussed in Chapter B'I and are shown on Plate B-3.

It is apparent that both the Oxnard and Fox Canyon aquifers extend for she unknown distance seaward beneath the ocean as illustrated by Geologic Sectoms K-K' and M-M' on Plate 12-B.

The seaward extension of the aquifers presents two important problems in the consideration of the ground water hydrology of the Oxnard Plain Basin. While data are lacking in many respects, the operation of the ground water basins is partly dependent on the conditions seaward of the coastline. The two problems are that the aquifers may outcrop at some unknown distance seaward on the ocean floor resulting in hydraulic continuity of the ground water with sea water, and that the seaward extension of these aquifers may act as ground water storage rese. voirs not inventoried in the hydrologic study whose utilization is dependent on to seaward distance of the outcrop.

Seaward extend of the Oxnard aquifer is unknown, though the presence of two sharply defined submarine canyons a short distance southerly of the coastline offers reasonable possibilities for the outcropping of the aquifer close to shore thereby limiting utilization of the storage capacity, at least in the vicinity of the canyons. Evidence suggesting that the Oxnard aquifer outcrops in the vicinit of the head of Hueneme Canyon includes the following: (1) the development of a landward gradient during periods of low water levels of the piezometric surface near Port Hueneme, the contours having a roughly circular shape with the canyon : the center, (2) historic reports of fresh water outflow in the Hueneme Canyon are at times of high water levels, (3) fluctuations of water levels in wells correspiding to but lagging up to three hours behind tidal fluctuations, and (4) water quaity indicating possible sea-water intrusion in 1951 near Port Hueneme. The only indications for seaward extension of the Oxnard aquifer or connection with the ora in the vicinity of Mugu Canyon are a landward gradient in the ground water surface and a response to tidal fluctuations in the wells. These canyon outcrops, as sha on Plate B-3, may be within a quarter of a mile of the shoreline.

Well log control on the Fox Canyon aquifer of the San Pedro formation 1 Oxnard Plain Basin is poor, but available data indicates that this aquifer thins southward, rising gently, so that it would be closer to the surface near Mugu

Cayon. It is possible that the Fox Canyon outcrops in both the submarine canyons, by available geologic evidence is not conclusive.

The offshore extensions of aquifers constitute additional ground water syrage space, the volume of which is undeterminable at this time and has not ben considered quantitatively in the hydrologic balances compiled in the course outhis investigation. It is conceivable that such storage could be considerable. Fe purposes of speculation, the Oxnard aquifer was projected westward beneath the ocan as indicated on the sections of Plate B-3. Projection of the Oxnard aquifer t the ocean floor indicates that it would outcrop in the vicinity of the 120 to 10 foot depth contours as shown on Plate B-3. Postulating further from the geolor of Oxnard Plain Basin it was estimated that the average thickness of the Oxnard auifer offshore is about 100 feet and that the specific yield is about 10 per cat. The maximum area underlain by the offshore extension was estimated to be abut 51,000 acres. These values are based upon assumptions that may be consideraly in error; however, they conform with and are believed to be the most reasonale values that can be obtained with available information. An estimate of offsore storage based on these values indicates the Oxnard aquifer may contain up to 9,000 acre-feet of ground water. The possible capacities of offshore extensions o the Fox Canyon and other aquifers do not lend themselves even to such approximtions. It is conceivable, however, that offshore storage in these aquifers can b great.

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It is obvious that data are not available to determine accurately either te degree of hydraulic continuity with the ocean or the offshore storage capacity c the aquifers. Full utilization of offshore storage is improbable because of te outcrop of the aquifers within the submarine canyons. Wells situated in the vcinity of these canyons may become polluted by inflow through the canyon walls tfore offshore storage in more remote submarine areas is exhausted. Water levels

in the Oxnard Plain Basin should, therefore, be maintained at such levels as to prevent accumulative intrusion of sea water.

Occurrence of Ground Water. Ground water occurs in a semi-perched zone in the Oxnard aquifer which is the principal aquifer, in the Fox Canyon aquifer, and in less easily traced gravels between these aquifers. At the present time, only two wells in this basin obtain water from the Santa Barbara formation. The semi-perched zone is unconfined, has no wells withdrawing water from it, and contains water of poor quality. All aquifers underlying the semi-perched zone contain confined ground water. Wells along the coast in the Oxnard aquifer flowed prior to 1927 and during the period from 19h2 to 19hh. The area of flowing well: in the spring of 19hh is shown on Plate 15-B.

Movement of Ground Water. Movement of ground water in the Oxnard aquifer is shown by the ground water elevation contour maps (Plates 14-B, 15-B, and 16-B). When Oxnard Forebay Basin is filled as in 1944, water moves southwestwar from the Forebay toward Hueneme and Mugu Canyons. When the water levels are low ered in the Forebay by pumping in Oxnard Plain and Oxnard Forebay Basins, the hy draulic gradient toward the ocean decreases until no outflow to the ocean occurs Further pumping on Oxnard Plain Basin or further lowering of the Forebay causes landward hydraulic gradient with a resultant landward movement of water in the seward extension of the aquifer. When the landward gradient occurs, a trough is formed. The formation of the trough will depend on elevation of the water table in Oxnard Forebay Basin and the amount of pumping in the Oxnard Plain Basin. De tailed water level contours indicate that the position of the trough axis approximates the shape of two circular segments with Hueneme and Mugu Canyons as center of the circles. The position of the trough axis varies seasonally as pumping rates and water levels in Oxnard Forebay Basin change. The trough position in t³

fal of 1951 is shown on Plate 16-B, along with the area in which water levels were below sea level.

Movement of ground water in the Fox Canyon aquifer is not well known because few water wells are drilled into it and water level data are largely lackiz. From available evidence it appears that water in the Fox Canyon aquifer nves from Oxnard Forebay Basin toward the southern portion of Oxnard Plain Basin. Wen pumping occurs in Pleasant Valley Basin, ground water in the Fox Canyon aufer moves eastward from Oxnard Plain Basin into Pleasant Valley Basin, as sown on Plate 16-B.

In the southeast portion of Oxnard Plain Basin there are a few wells wich are perforated in both the Oxnard and Fox Canyon aquifers, and water levels apear to be nearly the same in the two aquifers.

Replenishment and Depletion of Ground Water. Oxnard Plain Basin is rplenished by subsurface inflow from Oxnard Forebay and from the ocean side of te trough during periods of low water levels. It is possible that a minor sount of water is supplied to the Oxnard aquifer during periods of low water lvel by compaction of overlying clays. Appreciable leakage may occur through te clay from the semi-perched zone into the Oxnard aquifer, but its amount culd not be estimated because of the considerable time and expense required. No hdrologic evidence is available to show that leakage does occur through the clay cp, but well logs consistently indicate that this cap contains irregular intertdded lenses of gravel and sand, and it is therefore conceivable that leakage culd occur.

Extractions from Oxnard Plain include pumping and, during periods of Igh water levels, outflow to the ocean and effluent discharge through uncapped Hils. It is probable that some water is lost by leakage through the clay cap ten the piezometric surface of the Oxnard aquifer is higher than the water table the semi-perched ground water body.

It is also possible that an unknown amount of water is transferred between Oxnard Plain Basin and Mount Basin through the San Pedro formation.

<u>Subsurface Inflow and Outflow</u>. Subsurface inflow and outflow is discussed above.

<u>Ground Water Storage Capacity</u>. Since the aquifers in Oxnard Plain Basi are confined, there is essentially no change in storage except that resulting frc compaction of overlying clays. Such change of storage is probably negligible. The base of the clay which caps the Oxnard aquifer in general slopes oceanward as shown on Geologic Section K-K', Plate 12-B, and the diagrammatic sketch on Plate 13. Therefore, the line of intersection of the water table with the base of the clay cap shifts laterally with varying water levels in Oxnard Forebay Basin resuling in change of storage in the area defined as Oxnard Plain Basin. For convenience, such change of storage has been included with that in Oxnard Forebay Basin,

It is estimated that the total storage capacity of that portion of the Oxnard aquifer within Oxnard Plain Basin, if it could be dewatered, would be abou 500,000 acre-feet, and that the capacity of the Fox Canyon aquifer is probably the same.

<u>Yield of Wells</u>. Water wells in Oxnard Plain Basin yield from 300 to 2,300 gallons per minute and have an estimated average yield of about 900 gallons per minute and an average specific capacity of about 70.

Peasant Valley Basin

Pleasant Valley Basin has been divided in prior reports into pressure ad non-pressure areas. It is considered in its entirety as a pressure area in tis report for reasons discussed below. This basin consists of about 23,850 ares and is second only to Oxnard Plain Basin in irrigated acreage. It ranges i elevation from about 15 to over 240 feet above sea level.

<u>Geology</u>. Many aspects of the geology and ground water hydrology in Feasant Valley Basin are not clearly understood; faulting, folding, rapid thinnng of formations, multiple perforations of individual wells, and lack of adequate lgging and inspection of wells during drilling make an interpretation of the gology of the area difficult.

The water-bearing formations in Pleasant Valley Basin include alluvium c Recent and upper Pleistocene age, and the marine San Pedro and Santa Barbara frmations. These formations are underlain by the Pico and "Santa Margarita" frmations, Modelo shale, and volcanics of Miocene age. The volcanic rocks outcop in the Santa Monica Mountains on the southeast side of Pleasant Valley Basin.

In general, there are two areas in Pleasant Valley Basin where aquifers in alluvium of Recent and upper Pleistocene age are utilized. One area is north if the Camarillo fault and south of the Camarillo Hills, the other is south of the (marillo fault in the east and southeast portion of the basin. The aquifers in rese areas do not appear to be connected with the Oxnard aquifer in Oxnard Plain hsin. Alluvium north of the Camarillo fault reaches a thickness of 400 feet and insists of grey sand, gravel, and yellow and blue clay deposited in alluvial fans in Arroyo Las Posas and by other smaller creeks. The sands and gravels are nickest to the north and appear to pinch out toward the south. The alluvium buth of the Camarillo fault is about 400 feet thick and is mostly clay with irigular interbedded sands and gravels. Sands and gravels are predominant in ne easterly portion of Pleasant Valley Basin and appear to thin out westward

from the area south of the town of Camarillo into the west central portion of this basin.

The San Pedro formation underlies all Pleasant Valley Basin and consists of from 400 to 1,500 feet of gravels, sands and clays. The most important aquife in Pleasant Valley Basin is the basal Fox Canyon member which consists of sand an gravel. Thickness of the Fox Canyon aquifer varies from 100 feet near Santa Rosa Valley to 300 feet in most of the area. The Fox Canyon aquifer can be easily traced by well logs over all but the eastern corner of the basin, where there are few logs of deep wells. It is possible that the Fox Canyon aquifer is connected by interbedded gravels with the shallower sands and gravels of the alluvium in th eastern portion of Pleasant Valley.

The Fox Canyon aquifer is underlain by the Santa Barbara formation, whin consists of sand, clay, and some gravel and varies in thickness from 50 feet near Somis to over 900 feet at the west border of the area. The Santa Barbara formatin is reached by few water wells in Pleasant Valley Basin. It is possible that the equivalent of the Grimes Canyon member in East Las Posas Basin is present in Pleasant Valley Basin near the top of the Santa Barbara formation as shown on Geologic Sections L-L' and M-M', Plate 12-B. The Grimes Canyon aquifer consists of up to 300 feet of loose, coarse gravel and sand.

The volcanic rocks which are adjacent to and underlie the southern portion of Pleasant Valley Basin yield water to wells from fractures and from grave interbedded with the volcanic flows. Most wells drilled into the volcanics also obtain water from overlying gravels of the alluvium or San Pedro formation.

Structural features of Pleasant Valley Basin include at least two east west trending faults and associated folds in the northern portion of the area. The faults and folds appear to die out westward into the Oxnard Plain. These structural features are the Camarillo Hills and Springville anticlines, the Spriville fault zone, the Camarillo fault, and a snycline and anticline between thes faults. B-9h The Springville fault zone (see Plate B-lB) is up to 1,000 feet wide and consists of two major and probably other minor faults which parallel the south edge of the Camarillo Hills. The major faults of this zone are well exposed in portions of the Camarillo Hills. Several exposures along one of these faults show that it i turn consists of a complex zone up to 100 feet wide with highly folded sediments icluded between lesser faults. The principal fracture occurs in a zone of crushed stiments varying from a foot to several feet in width.

The Camarillo fault can be detected in water well logs where displacement o aquifers may be noted. It also has effected older alluvium and can be traced b surficial features.

The folds between the Camarillo fault and the Springville fault zone cnsist of an east-west trending anticline just north of the Camarillo fault, and asyncline farther north. These folds can be traced from well log data and surface ctcrops of older alluvium near Camarillo. The gentle synclinal fold between the (marillo fault and the Santa Monica Mountains can be detected from well logs.

The Fox Canyon aquifer is folded in the Camarillo Hills anticline so hat the top of it is exposed near Somis, as shown on Geologic Section L-L', Late 12-B. The Fox Canyon aquifer is displaced by the Springville fault zone ong the south side of the Camarillo Hills and also by the Camarillo fault. Well sindicate that the Fox Canyon aquifer lies unconformably on the volcanic rocks ong the south side of the Pleasant Valley Basin but does not outcrop there. It hins eastward, north of the Camarillo fault, into the Santa Rosa Basin where it inches out.

Occurrence of Ground Water. Ground water occurs in sands and gravels of Ne alluvium and of the San Pedro formation, as well as in the fractured volcanics. cound water in the basin is essentially confined. However, the Fox Canyon member 3 unconfined in a limited area near Somis. Some of the very shallow sands and

gravels around the north and southeast sides of the area may be unconfined, but available well logs indicate the shallow sands and gravels to be underlain by thick clays which probably prevent appreciable amounts of surface water from reacing the major aquifers.

<u>Movement of Ground Water</u>. Ground water moves toward the center of Pleasant Valley Basin during periods of heavy draft. When water levels are high the ground water generally moves in a southerly direction. Plates 14-B, 15-B, an 16-B show water level elevation contours of the two principal aquifers in this basin.

Replenishment and Depletion of Ground Water. Pleasant Valley Basin is replenished principally by subsurface inflow from East Las Posas Basin near Somis and from Oxnard Plain Basin through the Fox Canyon Aquifer. Replenishment of smaller magnitude also occurs by subsurface inflow from Santa Rosa Basin through: the San Pedro formation, from West Las Posas Basin through the Springville fault zone, and from the volcanics to the south and southwest of the basin. Ground way from Pleasant Valley Basin is depleted by pumped extractions and possibly by subsurface outflow toward the ocean during periods of high water level.

Subsurface Inflow and Outflow. Nearly all ground water used in Pleasa Valley Basin is supplied by subsurface inflow from the following sources: (1) From the ocean and Oxnard Forebay Basin through the Fox Canyon aquifer under Oxnard Plain Basin; (2) From East and West Las Posas Basins through the Fox Canyon aquifer near Somis and across the Springville fault zone; (3) From Sant Rosa Valley through the San Pedro formation; (4) From the fractured volcanics into overlying and adjacent shallow aquifers and the Fox Canyon aquifer.

Subsurface outflow toward the ocean may occur during periods of high water level.

Ground Water Storage Capacity. Negligible change of storage occurs in te little used shallow sands and gravels and in the confined aquifers. It is lkely that some change of storage has occurred in the volcanic rocks, but data as not available to make an estimate of this change. Similarly, data are lacking ir an estimate of total storage capacity of the basin, although it is probably c the order of magnitude of the storage capacity of Oxnard Plain Basin.

<u>Yield of Wells</u>. In general, the wells which are perforated in the Fox (nyon aquifer yield the greatest amounts of water. The maximum is about 2,400 ellons per minute and the average about 1,000 gallons per minute with a drawdown c about 10 to 50 feet. Wells perforated in both volcanic rocks and shallower auifers or in shallower aquifers only generally yield up to 1,000 gallons per n.nute, the average being about 400 gallons per minute and the drawdown 30 to 70 pet.

Ground Water Basins Within the Calleguas-Conejo Hydrologic Unit

The ground water basins of the Calleguas-Conejo Hydrologic Unit includ Simi, East and West Las Posas, Conejo, Tierra Rejada, and Santa Rosa Basins. Si Basin is the only basin in this hydrologic unit which is essentially a simple al luvial filled type. The others are complex and consist of two or more formation which are folded and faulted. Geologic features of this unit are shown on Plate B-lC, and certain physical characteristics are summarized in Table 16 of Chapter II.

<u>Simi Basin</u>

Simi Basin, comprising an area of about 10,760 acres, underlies the alluvial area of Simi Valley in the extreme east central portion of the Callegu-Conejo Hydrologic Unit. The floor of Simi Valley is formed by coalescing alluvi fans emanating from Tapo Creek and other canyons. Surface elevation ranges from 700 feet at the western end of the valley to 1,100 feet near the apex of the Tay Creek cone. A maximum surface elevation of 3,117 feet is attained on the draine divide in the Santa Susana Mountains to the north.

<u>Geology</u>. Geologic formations in the Simi Valley area may be divided into permeable alluvium of Recent and Fleistocene age and older semi-permeable formations. The folded Santa Barbara formation forms a ground water basin in t Tapo Canyon area which is separated from the alluvial filled Simi Valley by sempermeable older formations. Semi-permeable formations include the volcanics, Sespe, upper and lower Llajas, Santa Susana-Martinez, and sandstones of Cretaceco age. Of these semi-permeable formations, the Sespe, lower Llajas, and Cretaceco formations yield some water to wells in the hills on the south and southeast sie of Simi Valley. Ground water in some of these formations appears to be of poor quality, especially at depths of more than 300 or 400 feet.

Alluvium in Simi Basin consists of stream deposited gravel, sand, and cly up to 730 feet thick. The base of the alluvium is bowl-shaped and tapers upard to its edges. It is underlain and flanked by the older formations mentined above. The alluvium has a high clay content in the west end of the valley, whre it locally confines the underlying gravels. Elsewhere in Simi Basin the clys are lenticular and quite irregular.

The older formations in the hills surrounding Simi Valley form a synclne which plunges gently westward. The syncline is cut off on the north side of the valley by the Simi fault.

In the Tapo Canyon area about three miles north of Simi Basin, the Santa Bebara formation of Plio-Pleistocene age is exposed. It consists of marine and cutinental gravels, sands, and clays, all of which have been folded into a tight sucline by southward thrusting of the Santa Susana fault. The Santa Barbara formaion is over 1,000 feet thick in this area. Although some of the deep alluvium i Simi Basin may be equivalent to part of the Santa Barbara formation, the two as not in hydrologic continuity as they are separated by the semi-permeable formations north of Simi Valley.

Occurrence of Ground Water. Ground water occurs in the alluvial fill of Sni Valley and in interstices and fractures of the older formations that flank to valley. The alluvial fill constitutes the principal aquifer and underlies the asa of Simi Basin. Second in importance is the isolated area of the Santa Erbara formation which yields water from permeable sand and gravel members in the vcinity of Tapo Canyon.

In cross-section the alluvium of Simi Basin is shown to be shallow near te perimeter of the basin and to increase in thickness toward the center (Sectons Q-Q' and R-R', Plate 12-C) where it exceeds 700 feet.

Near the westerly extremity of the valley the alluvium at shallow depth (ntains considerable clay and silt. These fine materials serve to locally

confine ground water. In periods of high water level, wells that penetrate beneath these materials have flowed. However, unconfined conditions predominate in Simi Basin.

Movement of Ground Water. Ground water in the alluvium of Simi Valley moves westerly except in dry periods when wells are heavily pumped (see Plates 14-C, 15-C, and 16-C). During such periods a depression forms in the central portion of the basin and the ground water converges on this low area.

Ground water in the older semi-permeable formations moves in general toward the valley fill. In the eastern portion of Simi Valley water levels in wells in alluvium are generally lower than water levels in wells perforated only in the underlying older formations.

Replenishment and Depletion of Ground Water. Ground water in Simi Basi is replenished by percolation of direct precipitation, stream flow, and the unccsumed portion of water applied for irrigation and other uses. Additional source of replenishment are artificial spreading and a minor amount of subsurface inflk from older formations. Ground water in the older semi-permeable formations and in the Santa Barbara formation in the Tapo Canyon area is replenished by rainfal penetration and stream percolation.

The alluvial basin is depleted by pumped extractions, consumptive use f phreatophytes, effluent discharge and subsurface outflow. The semi-permeable femations are depleted by evapo-transpiration, pumping, by outflow through spring: during periods of high water level, and by subsurface outflow into the alluvium The Santa Barbara formation is depleted by spring discharge and by pumping of water for export to Simi Valley.

<u>Subsurface Inflow and Outflow</u>. Subsurface inflow enters the alluvial fill of Simi Basin from adjacent older formations, but no subsurface inflow is known to enter this hydrologic subunit from other subunits. Subsurface outflow

Javes Simi Valley through the Arroyo Simi where the alluvium appears to be only () to 100 feet thick and about 1,000 feet wide, and enters East Las Posas Basin. Absurface flow out of Simi Valley through this alluvium has been estimated by the slope-area method to be about 100 acre-feet per year. During periods of low wher levels, it is possible that subsurface outflow becomes negligible.

Ground Water Storage Capacity and Specific Yield. Estimates of change is storage in the alluvium of Simi Basin are discussed in Chapter II. Estimated wighted mean specific yield of alluvial sediments in Simi Basin is 8.6 per cent. Otal storage capacity of the alluvium below high water level of 1929 was estimaod to be about 180,000 acre-feet. Storage above this level was estimated to be bout 40,000 acre-feet.

<u>Yield of Wells</u>. Wells in the alluvium of Simi Valley yield an average ? about 400 gallons per minute. An exceptional well in Cretaceous sandstone is ported to yield 1,200 gallons per minute, but most of the wells in the older ocks yield about 100 gallons per minute. Wells in the Santa Barbara formation in ne Tapo Canyon area have an average yield of about 100 gallons per minute.

Artificial Spreading of Water as a Means of Basin Replenishment. Stuies of the Soil Conservation Service and the Division of Water Resources indicate ne most suitable locations for major spreading works on alluvium are situated ear the mouth of Tapo Canyon, along Chivo Creek and along Arroyo Simi just west f Santa Susana. The Tapo Creek location provides greater available ground water torage than does the Arroyo Simi location, but infiltration rates at this site are nferior. The Chivo Creek area appears to have least available storage but infilration rates are suitable. Before any particular site is chosen here or in any ther area for large scale spreading, exploratory test wells should be drilled and ilot spreading operations conducted to insure success.

East Las Posas Basin

East and West Las Posas Basins are geologically similar in some respect but differences are great enough that they can be described separately. East Las Posas Basin comprises about 36,370 acres and is located within the East Las Posas subunit. It is bounded by nonwater-bearing formations which are adjacent to the basin on the south slope of Oak Ridge, in the Happy Camp Canyon area, and in the Las Posas Hills. The western boundary is the surface drainage divide between Eas and West Las Posas Basins. Near Somis the boundary was arbitrarily placed across the narrowest part of the southwesterly-trending valley through that town. Eleve tion of the drainage area ranges from about 250 feet near Somis to about 2,800 fit on Oak Ridge.

<u>Geology</u>. East Las Posas Basin is a broad east-west trending valley be tween Cak Ridge and the hills on the south and is presently undergoing stream di section. The principal water-bearing materials of the basin are Recent and uppe Pleistocene alluvium and the San Pedro and Santa Barbara formations. Semi-perme able older formations adjacent to and underlying the water-bearing formations include the Sespe, Vaqueros, Modelo, and Pico formations, as well as limited are; of volcanic rocks. Most of these older formations contain water of poor quality but good water has been obtained from sandstones and conflomerates of the Sespe formation. Late Quaternary alluvium occurs as fill in most of the valleys of th basin. The thickest, most extensive, and most important alluviated area is in the vicinity of Moorpark, where the alluvium consists principally of up to 200 feet f sand and gravel and underlies about 5,100 acres. Near Somis the alluvium is on 40 to 80 feet thick, and consists of silts and clays. In the smaller valleys, alluvium generally varies up to 40 feet in thickness and consists of silt and sid with some clay and gravel.

Frevious workers have called the youngest of the pre-alluvial sediments cerrace deposits". Since most of these deposits are folded and since it is exremely difficult to differentiate them from the underlying San Pedro formation hey are considered in this report as part of the San Pedro formation. The San edro formation is up to 2,000 feet thick in this basin and consists predominantly f yellow, red, and blue silty clay, with lenticular sands and gravels.

The San Pedro formation contains two members notable as aquifers; namely he Epworth gravels and the Fox Canyon aquifer. The Epworth gravels, near the op of the San Fedro formation, are located in a rather limited area lying about wo to three miles north and northwest of Moorpark. They consist of up to 200 eet of gravel, gravelly clay, and silt, grading westward and southward into silt nd clay. The Epworth gravels are probably remnants of an ancient alluvial fan, hich accounts for their limited extent. The gravels have been folded and parially eroded so that they now outcrop in the area shown on the geologic map Plate B-1C) and they underlie a total area of about six square miles.

The basal Fox Canyon member of the San Fedro formation has been named rom its excellent exposure in Fox Canyon, about a mile west of Bradley Road. It onsists of from 100 to 400 feet of sand and gravel containing some clay and silt enses. The outcrop of the Fox Canyon member along the south slope of Oak Ridge s irregularly bedded as a result of facies changes and scour and fill, and varies onsiderably in total thickness. Fossils found in the member indicate deposition nder shallow marine conditions. Sediments of the Fox Canyon member generally re white or gray in color. These sediments can be easily differentiated on the utcrop from the underlying Grimes Canyon sediments because of the distinct brown coloring of the latter. In well logs it is usually difficult to differentiate 'ax and Grimes Canyon sediments. From a study of all available logs it is clear hat the Fox Canyon aquifer extends under most of East Las Fosas Basin. In generl, the Fox Canyon aquifer is thickest in the central portion of the basin where

it consists principally of coarse sand. On Oak Ridge it is variable in thickness and consists of gravel and sand grading into fine sand near Happy Camp Canyon, where it pinches out entirely. On the Las Posas Hills it consist of sand and gravel, grading into sand near Moorpark.

In East Las Posas Basin, most of the San Pedro formation other than the above mentioned aquifers consists of fine silt and clay with scattered lenses of gravel and sand. Since individual gravel lenses are quite local, and since yield of wells in this material is generally quite low, this portion is here called the semi-permeable portion of the San Pedro formation. These materials overlie the Fox Canyon aquifer, and confine the ground water under pressure in that aquifer.

The Santa Barbara formation underlies the San Pedro formation and in this basin consists of up to 2,000 feet of clay, silt, sand, and gravel. At the west end of Oak Ridge it consists of clay and silt, but east of Bradley Road sand and gravel lenses become more common along the outcrop until in Happy Camp Canyon they predominate. The formation also thins to about 1,000 feet near Happy Camp Canyon. A coarse gravel member near the top of the Santa Barbara formation is exposed east and north of Bradley Road and is called the Grimes Canyon aquifer. This aquifer consists of coarse to fine brown gravel, sand, and lenses of clay and silt. The rusty brown color of the Grimes Canyon is usually distinctive, but occsionally is not evident in exposures or well logs. The aquifer varies in thicknes from zero to about 1,000 feet in Happy Camp Canyon where it comprises nearly all the Santa Barbara formation. The Grimes Canyon aquifer underlies most of East Lⁱ Posas Basin.

The Grimes Canyon aquifer is overlain by the Fox Canyon aquifer, and several outcrops reveal them to be in direct contact, although a clay member with the Santa Barbara formation separates the two aquifers in other exposed areas. ¹ thickness of the aquifers indicated by some water and oil well logs suggests tha the Fox Canyon and Grimes Canyon aquifers are in direct contact under much, if n

most, of East Las Posas Basin. It is possible that some of the sediments of the Sata Barbara formation in Tapo Canyon are also the equivalent of the Grimes Canyh member. The detailed field work necessary to make such a discrimination was no undertaken in this investigation. The Santa Barbara formation with exception of the Grimes Canyon aquifer previously described is for the most part composed of materials of low permeability.

The San Pedro formation, Santa Barbara formation, and the underlying Pco, Modelo, Vaqueros, and Sespe formations are all folded and faulted, only alluvum being undisturbed. In general, East Las Posas Basin is a synclinal area, punging gently westward, and includes several minor en echelon synclines and anticines. Oak Ridge and the Las Posas Hills are major anticlinal uplifts. The foldig of the area has resulted in the Fox Canyon and Grimes Canyon aquifers being bried quite deeply in the central portion of the basin and exposed around the finges. Structural features and relationships of various aquifers are shown on Cologic Section N-N' and P-P', Plate 12-C.

Occurrence of Ground Water. Ground water occurs in the sands and gravels the alluvial deposits, in the Epworth gravels, and in the Fox Canyon and Grimes (nyon aquifers. Limited amounts of ground water occur in sands and gravel lenses i the semi-permeable portion of the San Pedro formation which overlies the Fox (nyon member. Limited supplies of water occur in the older semi-permeable formatons and may be of poor quality.

Ground water in the alluvial deposits and in the Epworth gravels is esentially unconfined, although water level behavior in some wells indicate locally onfined conditions. Ground water in the Fox Canyon and Grimes Canyon aquifers is onfined by the overlying silts and clays. Ground water is unconfined in these quifers, however, near their upturned edges which approximate the outcrop areas ; shown on Plate B-1C.

<u>Movement of Ground Water</u>. Ground water in the alluvium near Moorpark generally moves westward toward Somis except during periods of low water level, when a pumping depression forms southwest of Moorpark. Ground water in the Epworth gravels appears to move in a southerly direction, indicating some movemen from the Epworth gravels into the semi-permeable portion of the San Pedro formation.

Ground water in the Fox Canyon aquifer moves in a southwesterly direction from Happy Camp Canyon and the outcrop along the north side of East Las Pose Basin. Subsurface flow in the fall of 1951 as depicted by dashed ground water contours on Plate 16-C converges on the Somis area and moves into Pleasant Valley Basin. Meager historic data suggests that in periods of high water levels ground water in the Fox Canyon aquifer moves westward into West Las Posas Basin as well as into Pleasant Valley Basin. Water levels of fall 1951 indicate a ground water mound in the piezometric surface of the Fox Canyon aquifer near the west boundar of East Las Posas Basin.

Replenishment and Depletion of Ground Water. Ground water in East Las Posas Basin is replenished by percolation of direct precipitation, stream flow, and the unconsumed portion of water applied for irrigation and other uses in out crop areas of aquifers, and possibly to some extent by subsurface inflow from older formations that surround the basin. Alluvium southwest of Moorpark along Arroyo Las Posas overlies the Fox Canyon aquifer where they are probably in hydrlogic continuity. In the vicinity of Somis and Moorpark, studies of water level and the chemical character of ground water have led to the conclusion that groun water moves from the alluvium into the Fox Canyon aquifer.

East Las Posas Basin is depleted by pumped extractions from the Fox Cayon aquifer and by consumptive use of phreatophytes. Additional depletion is effected by subsurface outflow and export of water into West Las Posas Basin.

Spings are reported to have flowed near Somis in Arroyo Las Posas in the early 190's which would indicate that in periods of high water levels the basin was to some extent depleted by effluent discharge.

Subsurface Inflow and Outflow. Subsurface inflow into East Las Posas Bein is limited to that coming from older rocks and about 100 acre-feet per yer which enters from Simi Basin through alluvium. This latter increment of inflow has been described under the paragraph on inflow and outflow to Simi Basin. Subsurface outflow into Pleasant Valley Basin through the Fox Canyon aquifer is indicated by water level contours (Plates 14-C, 15-C, and 16-C). This outflow has been estimated by the slope area method to be on the order of 3,000 acre-feet per yer. As previously mentioned, subsurface outflow to West Las Posas Basin has pubably occurred in the past during periods of high ground water level.

Ground Water Storage Capacity and Specific Yield. Changes in ground wher storage occurring within East Las Posas Basin during selected study periods where estimated following the procedures described in Chapter B-VI and are discussed in Chapter II. Total storage capacity of aquifers in the basin could not be estimued, but is probably very large.

Depth to water in the outcrop area of the Fox Canyon and Grimes Canyon auifers on Oak Ridge was approximately 500 or 600 feet in 1951 and 1952, and terefore considerable available storage exists in these aquifers above the water tole. The average specific yield of the Fox Canyon and Grimes Canyon aquifers is blieved to vary between 10 and 20 per cent. Estimated specific yield of the Evorth gravels is about six per cent; most of the remainder of the San Pedro formtion, three per cent, and the alluvium in the Moorpark area, eight per cent. Yield of Wells. Estimated average yield of wells in East Las Posas

Basin is summarized below:

| Alluvium | 400 | gallons | per | minute |
|--|-----|---------|-----|--------|
| Epworth Gravels | 300 | 11 | Ħ | 12 |
| Fox Canyon and Grimes Canyon Aquifers | 600 | п | # | н |
| Semi-permeable portion of San Pedro formation | 10 | 11 | 11 | Ħ |

Artificial Spreading of Water as a Means of Basin Replenishment. Water could be spread artificially on any portion of the outcrop area of the Fox Canyon or Grimes Canyon aquifers and reach the water table. The most desirable spreadin area for these aquifers is in Happy Camp Canyon about three miles north of Arroyo Simi. This locality has available surface area for construction of spreading grounds, high rates of percolation according to the Soil Conservation Service, an free access to the water table of the Grimes Canyon aquifer which is in hydrologi continuity with the Fox Canyon aquifer. In addition, the water table is about 50 feet below the surface in this area so that adequate ground water storage is avaiable. A seismic survey of the spreading area by this Division indicates an absence of clay lenses within 30 to 60 feet of the surface. Most other areas of outcrop have limited surface area available for construction of spreading works.

Spreading into the Epworth gravels may be possible, but this would bene fit only the wells in these gravels. A large surface area is available for construction of spreading works near the corner of Broadway and Moorpark Roads, abou two miles north of Moorpark, but spreading rates are probably low.

Spreading into alluvium near Moorpark is feasible from percolation rate and surface area aspects. However, available storage of alluvium is probably smal even when the alluvium is dewatered, and it might be filled by natural stream percolation of Arroyo Simi during wet periods.

Wst Las Posas Basin

West Las Posas Basin is located within the corresponding subunit and cmprises about 11,450 acres. Elevation of the subunit ranges from 200 feet to a nximum of 2,258 feet on South Mountain. Boundaries of West Las Posas Basin are the outcrop of the Fox Canyon aquifer on the north, the surface drainage divide a the east and south, and the limit of the Oxnard zone of Oxnard Plain and (nard Forebay Basins on the west.

<u>Geology</u>. Aquifers of significance in West Las Posas Basin include the bx Canyon and Grimes Canyon. The upper semi-permeable portion of the San Pedro brmation overlies the Fox Canyon aquifer and in turn is overlain by alluvium of heent and upper Pleistocene age. The alluvium is not easily differentiated from he silts and clays of the underlying San Pedro formation, but it is probably up > 200 or 300 feet thick. The alluvium consists of fine yellow silt and clay with pattered lenticular sands and gravels and has been deposited in alluvial fans by nall streams draining Oak Ridge. The semi-permeable portion of the San Pedro prmation consists of over 1,000 feet of yellow and blue silty clay and clay, with pattered lenticular sands and gravels.

The Fox Canyon aquifer consists of 200 to 300 feet of sand and gravel at ne base of the San Pedro formation. The Fox Canyon aquifer continues into East is Posas Basin, Oxnard Plain and Forebay Basins, and into the Camarillo Hills nd Pleasant Valley Basin. The Fox Canyon aquifer outcrops on the south slope of ik Ridge and in the east end of the Camarillo Hills.

The Fox Canyon aquifer is underlain by the Santa Barbara formation which ontains the Grimes Canyon aquifer near its top. The Grimes Canyon aquifer does ot outcrop in the West Las Posas Basin but underlies it as shown by electric logs and drillers logs. It consists of up to 300 feet of coarse gravel and sand. Well

logs indicate that a clay bed up to 600 feet thick lies between the Fox Canyon al Grimes Canyon aquifers in the Camarillo Hills (see Section L-L', Plate 12-B). A similar clay bed is found on the outcrop in East Las Posas Basin and it is likely that these two clay beds are of a similar origin. Field inspection of the clay bed in East Las Posas Basin shows that an erosional unconformity at the base of the Fox Canyon aquifer has resulted in direct contact of the Fox Canyon and Grimes Canyon aquifers where the clay has been eroded.

As in East Las Posas Basin, folding of the Fox Canyon has resulted in its being exposed on the edges of the basin and deeply buried in the middle. Th most prominent folds are the Camarillo Hills anticline and the Las Posas synclin

Occurrence of Ground Water. The Fox Canyon and Grimes Canyon aquifers are the principal sources of ground water in West Las Posas Basin. Some water i derived from sand and gravel zones of limited extent contained within the semipermeable portion of the San Pedro formation. Ground water in the Fox Canyon an Grimes Canyon aquifers is confined except where these aquifers outcrop on the southern slopes of Oak Ridge and where they have been folded in the Camarillo Hills (see Section L-L', Plate 12-B).

<u>Movement of Ground Water</u>. Movement of ground water in 1951, as depictd by contours (Plate 16-C), was westerly in the Fox Canyon aquifer toward Oxnard Forebay Basin. Some ground water possibly moves southward across the Camarillo Hills, through the Springville fault zone, and into Pleasant Valley.

Replenishment and Depletion of Ground Water. West Las Posas Basin is replenished by percolation of direct precipitation and stream flow on the outcro area of the Fox Canyon aquifer and possibly to some extent by subsurface inflow from East Las Posas Basin. The silty upper portion of the San Pedro formation a alluvium may in addition be replenished by percolation of the unconsumed portion of water applied for irrigation and other uses. West Las Posas Basin is deplet

b; pumping from the Fox Canyon and other aquifers and by subsurface outflow.

Subsurface Inflow and Outflow. Subsurface flow into West Las Posas Bsin probably occurs from East Las Posas Basin during periods of high water Ivel. Subsurface outflow occurs into Oxnard Plain and Pleasant Valley Basins trough the Fox Canyon aquifer. The outflow has been estimated by the slope area mthod to be on the order of 600 acre-feet per year into the Oxnard Plain Basin. Sbsurface outflow probably occurs across the Springville fault zone into Pleasant Vlley Basin, but no data are available to estimate the amount. Since the ground wter divide in the piezometric surface of the Fox Canyon aquifer is located close t the surface divide, it is likely that subsurface outflow into East Las Posas Esin through that aquifer is negligible.

Ground Water Storage Capacity and Specific Yield. Change of ground water corage in West Las Posas Basin is discussed in Chapter II. Specific yield of the bx Canyon aquifer is estimated by inspection to be about 15 to 20 per cent. Spefic yield of the overlying San Pedro formation and the alluvium is estimated to be about three per cent.

<u>Yield of Wells</u>. Yield of wells in the Fox Canyon and Grimes Canyon aquiers averages about 600 gallons per minute. Wells in the semi-permeable portion the San Pedro formation yield about ten gallons per minute.

Artificial Spreading. Artificial spreading on the outcrop area of the ox Canyon aquifer is physically possible, as in East Las Posas Basin, although ne rugged topography limits areas in which spreading works could be constructed.

onejo Basin

Conejo Basin is located in the southern portion of the Calleguas-Conejo ydrologic Unit as shown on Plate 11. The basin varies in elevation from about

600 feet to 2,300 feet except on the floor of Conejo Creek Canyon, the elevation of which is about 300 feet. Within the hydrologic unit most of the rocks including volcanics and consolidated sediments absorb and transmit water, but wells in these rocks generally yield small amounts of water. Since there are no areas which can be easily defined as ground water basins, the entire drainage area of about 28,930 acres is considered as the basin.

<u>Geology</u>. Most of Conejo Basin is an upland valley area which has drained eastward in the geologic past, possibly into Triunfo Creek. The ancestral drainage was subsequently captured by headward erosion of Conejo Creek, so that the area now drains into Santa Rosa Basin.

Geologic formations in Conejo Basin include alluvium, Modelo sandstone and shale, volcanic rocks, the Topanga formation, and limited exposures of the lower Llajas and Santa Susana-Martinez formations as well as some consolidated sediments of Cretaceous age. Alluvium of Recent and Pleistocene age occurs as valley fill in the Newbury Park and Thousand Oaks areas, on the floor of Conejo Creek Canyon, and as terrace deposits scattered throughout the basin. The alluvium is generally shallow, probably being only a few feet thich except in the valley fill areas where it attains a thickness of about 60 feet. The volcanic rocks, the Topanga formation, and the Modelo sandstones and shales are drilled by many wells in Conejo Basin. The limited outcrops of other formations are not generally drilled within the basin. All the aforementioned formations are described in Chapter B-III of this Appendix. All the formations with the exception of the alluvium are folded and faulted as shown on the geologic map (Plate B-1C).

Occurrence of Ground Water. Ground water occurs in the alluvium, in the fractures and weathered portions of the volcanic rocks and Modelo shales, and in pervious zones of the Modelo sandstone and Topanga formations. The ground water
suface conforms, in general, with the topography as shown on Plate 16-C and is esentially unconfined. Most wells in alluvial areas penetrate the alluvium compitely and obtain water from underlying formations as well as from alluvium. At the time of this investigation no water wells were known to have penetrated very deply into the older rocks. Scattered oil well logs indicate that such previous zues exist in the older rocks, but cuality of water in them is uncertain.

Movement of Ground Water. Ground water from the periphery of the basin enverges toward Conejo Creek as indicated by ground water contours on Plate 16-C. Prennial springs which are supplied by subsurface flow from Conejo Basin exist in the canyon of Conejo Creek.

Replenishment and Depletion. Ground water is replenished by percolation o direct precipitation and stream flow as is evidenced by a close relationship btween water table and topography and fairly rapid recovery of water levels follwing rains. Replenishment also occurs by percolation of the unconsumed portion o water applied for irrigation and other uses. Ground water is depleted by puped extractions, by consumptive use of phreatophytes, by effluent discharge, ad, most likely, by subsurface outflow.

Subsurface Inflow and Outflow. No subsurface inflow occurs into Cnejo Basin. Subsurface outflow probably occurs into Santa Rosa Basin through te alluvial fill in Conejo Creek Canyon and through the volcanics. Subsurface otflow may also occur into Pleasant Valley Basin, through the volcanics. Subsufface outflow through volcanics appears to be possible because: 1. The volcaros dip toward Santa Rosa and Pleasant Valley Basins; 2. Water levels in Cnejo Basin are higher than in the other basins; 3. The volcanics are permeable. Aground water divide may exist in the same general location as the drainage

divide but water level data are lacking to verify this possibility. If this were the case, subsurface flow into Pleasant Valley and Santa Rosa Basins through the volcanics would be negligible.

Water level measurements in the Thousand Oaks area indicate that a ground water divide exists near the drainage divide so that subsurface flow in or out of the Malibu Hydrologic Unit is probably negligible.

<u>Storage Estimates</u>. Change of storage does occur in Conejo ^Basin as evidenced by fluctuations of water levels and unconfined ground water conditions. Well log and historic water level data are lacking, however, and specific yield of the various formations in the basin is uncertain. For these reasons estimates of change in storage in Conejo ^Basin are not considered to be of sufficient accuracy for use in the hydrologic balance.

<u>Yield of Wells</u>. Because of the general low permeability of the formations in Conejo Basin, average yield of wells is low and on the order of 50 gallons per minute. One exceptional well, however, yields 1,000 gallons per minute and several yield about 300 gallons per minute.

Artificial Spreading. Artificial spreading in Conejo Basin does not appear to be feasible because of relatively shallow depths to water and the general low specific yield of the formations.

<u>Tierra Rejada Basin</u>

Tierra Rejada Basin is located between Simi, Conejo, Santa Rosa, and East Las Posas Basins as indicated on Plate 11. Surface elevation ranges from 6) feet in the valley floor to about 1,600 feet on the drainage divide. Nearly all Tierra Rejada Basin is underlain by water-bearing volcanic rocks. For this reasi the drainage divide is taken as the basin boundary. The basin includes an area of about 4,390 acres.

<u>Geology</u>. Although most of Tierra Rejada Basin is underlain by fractured vocanic rocks, a small portion is underlain by the Modelo, Topanga, and Sespe fomations. The volcanics consist of about 2,000 feet of basaltic flows, agglomentes, rhyolitic tuffs, and interbedded conglomerates and clays. These materials ar intruded by basaltic dikes and sills.

All formations present are folded and faulted. In general the structive of the basin is that of a westward plunging syncline. The volcanic rocks in to southern and eastern parts of the basin dip from 10 to 30 degrees toward the fat irrigated portion of the basin. North of the irrigated area the attitude of to volcanic rocks is nearly vertical. These rocks are terminated near the north bundary of the basin by the east-west trending Simi fault. Another fault trendig north-south displaces the volcanic rocks several hundred feet near the western sde of the basin.

Occurrence of Ground Water. The volcanic rocks are generally highly factured but appear to be most intensively fractured beneath the irrigated porton of the basin, as wells in the volcanics have highest yields there. Ground vter occurs chiefly within these fractures, and is essentially unconfined.

<u>Movement of Ground Water</u>. Ground water moves through the highly fracured volcanic rocks converging toward the westerly end of the basin. At the west ad subsurface flow out of the basin is impeded by the above mentioned north-south ault. That this fault serves as a ground water barrier is evidenced by a probunced drop in water level. In 1951 the ground water level east of this fault a Tierra Rejada Basin stood about 100 feet above the level observed in a well ituated near the fault on its westerly side. Movement of ground water is inicated by ground water elevation contours on Plates 15-C and 16-C.

Replenishment and Depletion of Ground Water. Tierra Rejada Basin is relenished by percolation of direct precipitation, stream flow, and the unconsumed B-115 portion of water applied for irrigation and other uses. The basin is depleted by pumped extraction, limited subsurface outflow into Santa Rosa Basin, and possibly effluent discharge and consumptive use of phreatophytes during periods of high water level.

<u>Subsurface Inflow and Outflow</u>. No subsurface flow enters Tierra Rejad Basin from Simi Basin. Water level measurements in the area of the drainage divide separating these basins indicates that a ground water divide exists which would prevent inflow from Simi Basin through the volcanics. If no ground water divide existed there, some inflow might be expected since water level elevations in the west portion of Tierra Rejada Basin are generally lower than in Simi Vall

As previously discussed, the north-south fault at the west end of Tier, Rejada Basin limits subsurface outflow. A producing well situated a short distance west of the fault suggests that this fault is only a partial barrier since the only feasible source of supply is subsurface flow across the fault. It is likely that subsurface flow northward across Simi fault into East Las Posas Basi is negligible.

Ground Water Storage Capacity. As in Conejo Basin, poor geologic and hydrologic data resulted in uncertainties in change of storage estimates; so direct evaluation thereof could not be made.

<u>Yield of Wells</u>. Wells in Tierra Rejada Basin yield from 10 to 700 gal lons per minute with an average yield in the principal pumping area of about 300 gallons per minute.

Santa Rosa Basin

Santa Rosa Basin, comprising about 3,490 acres, is located just east of Pleasant Valley Basin. It is bounded by the volcanics on the south, the limit the San Pedro formation on the north, Tierra Rejada Basin on the east, and the

topgraphic narrows at the west end of the basin. Santa Rosa Basin ranges from 20 to over 400 feet in elevation with a maximum elevation of about 1,200 feet onthe drainage divide.

<u>Geology</u>. Principal water-bearing sediments in Santa Rosa Basin include Reent alluvium and the San Pedro formation . Formations underlying and adjacent to the basin include the Santa Barbara, the Topanga and Sespe formations, and volcaic rocks.

Recent alluvium in Santa Rosa Basin consists of up to 200 feet of gravel, sad and clay. Fossil remains in outcrops of alluvium in the stream cut gullies inicate that some of the clays in the west end of the basin have been deposited ir a fresh water swamp or shallow lake.

The San Pedro formation consists of up to 700 feet of gravel, sand, silt, al clay. In the western end of the basin a sand and gravel member about 100 feet tick can be traced in well logs at the base of the formation and is probably the evivalent of the Fox Canyon aquifer. This aquifer, however, cannot be traced in wll logs into the central and eastern portion of the basin. In general, the sands al gravels of the San Pedro formation are extremely lenticular and with the above intioned exception cannot be correlated between wells. In the west end of the bsin, the Santa Barbara formation is found below the San Pedro, but it contains wter of poor quality. Only one or two wells are drilled into it here, so that its ture is poorly known, but it apparently consists of silt and clay with lenticular favels and sands. The volcanics of Miocene age which underlie the alluvium and In Pedro formation on the south side of the basin are exposed in the hills to the outh where they dip ten to twenty-five degrees northward. The volcanics consist c over 2,000 feet of interbedded basaltic agglomerates and flows with scattered idesitic intrusions, all of which are fractured. The great thickness of volca-Lcs on the south of the basin is represented in the Las Posas Hills by a basaltic

sill about 15 feet thick. Relationships of the formations are shown on Geologic Sections N-N' and P-P' on Plate 12-C.

All these formations except the alluvium have been folded and faulted. The structure of most significance is the east-west trending Santa Rosa syncline, shown on the geologic map (Plate B-1C), in which the San Pedro formation has been folded. Field inspection of outcrops and well logs indicates that the north dipping flank of the syncline lies beneath the alluvium on the south side of San Rosa Basin. The San Pedro formation and alluvium are underlain in part by volcanics and other formations. The north flank of the folded San Pedro formation has been cut off by the Simi-Santa Rosa fault system, exposing the semi-permeable Sespe and Topanga formations in the Las Posas Hills just north of the basin.

Occurrence of Ground Water. Ground water occurs in pervious zones of the Recent alluvium and San Pedro formation and in the fractured volcanics. Wat of poor quality occurs in the Santa Barbara formation and possibly in the Sespe and Topanga formations. Ground water in Santa Rosa Basin is essentially unconfined, although the pervious lenses of the San Pedro formation are confined in some areas.

<u>Movement of Ground Water</u>. Ground water in Santa Rosa Basin moves westerly and within the basin appears to move northerly from the volcanics and southerly in the San Pedro formation. Plate 16-C shows the southerly movement in to fall of 1951, but the northern direction of movement at this time was not appreciable. When Conejo Creek flows into Santa Rosa Basin, percolation occurs and a ground water mound is built up near the mouth of Conejo Creek. Past measurement of a well on the extreme south side of the basin as well as the presence of springs in the volcanics indicate that some water probably moves directly from te volcanics into the alluvium.

Replenishment and Depletion of Ground Water. Ground water in Santa Rsa Basin is replenished by percolation of direct precipitation, stream flow, and te unconsumed portion of water applied for irrigation and other uses as well as t subsurface inflow. The basin is depleted by pumped extractions, subsurface ctflow, and by effluent discharge and consumptive use of phreatophytes during priods of high water level.

<u>Subsurface Inflow and Outflow</u>. Subsurface inflow to Santa Rosa Basin ccurs from Tierra Rejada and Conejo Basins. Inflow from both these sources is officult to estimate by geologic methods because of the lack of wells and other cta. Subsurface outflow into Pleasant Valley through the San Pedro formation has ten estimated by the slope-area method to be about 200 acre-feet per year.

<u>Ground Water Storage Capacity and Specific Yield</u>. Estimates of change c'storage in alluvium and San Pedro formation are discussed in Chapter II. Wighted average specific yield of the alluvium and San Pedro formation is estinted to be five per cent.

<u>Yield of Wells</u>. Water wells in Santa Rosa Basin yield up to 1,200 galons per minute. Their yield averages about 600 gallons per minute, and specific apacities range from 10 to 30. The highest yielding well is in the volcanics, ad wells in the San Pedro formation generally yield slightly less than those in he alluvium. In general, these differences are controlled by permeability, but h some instances are dependent on the method of well construction.

Artificial Spreading. Spreading in Santa Rosa Basin is feasible near ne mouth of Conejo Creek, where most stream percolation has occurred. In the ther areas where surface conditions appear suitable for spreading, well log data re poor, and it is not known whether large quantities of water would percolate irectly to the water table.

Miscellaneous Areas In and Near Ventura County

The areas discussed below are those which contain ground water bodies (unknown extent and usefulness within and adjacent to Ventura County, but which ar outside the principal developed ground water areas.

Malibu Hydrologic Unit

The Malibu Hydrologic Unit is located in the southeastern portion of the county and includes that portion of the Santa Monica Mountains draining south ward to the ocean. Principal geologic features are shown on Plate B-1C.

Formations in this area include alluvium, Modelo sandstone and shale, volcanic rocks, the Topanga formation, and a small area of older sedimentary rock Ground water is obtained from wells drilled into most of these formations. Principal water-bearing formations, however, are the alluvium and the volcanic rocks.

Alluvium of Recent and Pleistocene age occurs as valley fill up to at least 100 feet thick in the upper drainage areas of Triunfo and Medea Creeks. The alluvial area which has most wells is Hidden Valley, located just west of Lake Sherwood. Water wells here penetrate alluvium and the underlying volcanic rocks. Nearly all the wells in Hidden Valley are used for domestic and limited irrigation purposes. Yield of wells is small in this area, probably averaging 50 gallons per minute. The low yield of the wells suggests that the alluvium and volce nics are fairly impervious and have low specific yield and storage capacity. The direction of movement of ground water is eastward toward Lake Sherwood as shown on Plate 16-C.

Downstream from Lake Sherwood a few irrigation wells obtain a good support of ground water from the coarse alluvial gravels in the valley floor. In most of the remaining alluvial areas shown on the geologic map few wells have been drille

bt the alluvium is most likely thin and probably does not contain large quantites of ground water.

Numerous wells have been drilled into the volcanic rocks, the Topanga frmation, and the Modelo formation. Most of these wells yield small amounts of wter, but one well in the volcanics and one in the Topanga formation reportedly yeld 300 gallons per minute. Scattered well measurements and observations of srings in the Malibu Hydrologic Unit indicate that the ground water surface confrms in general with topography, as would be expected with formations of low prmeability.

Encon Subunit and Rincon Creek Drainage Area

The Rincon subunit is located along the ocean between the Ventura River and the Santa Barbara-Ventura County line. Ground water bodies are extremely small i this subunit, being restricted to the alluvial filled valley bottoms, the beach oposits, and the thin terrace deposits. Older formations probably contain water of poor quality, as do the beach deposits during dry periods. Because of the limied ground water bodies in the subunit only a few small wells are found there, nost of the water being imported.

The geology and occurrence of ground water in the Rincon Creek area has then discussed by Upson (1951). Since Rincon Creek recharges a ground water basin located mostly in Santa Barbara County, the area will not be further discussed here.

(yama River Drainage Area

The principal development in the Cuyama River drainage area has occurred I Santa Barbara and San Luis Obispo Counties. This area has been described by Doson and Worts (1951). The following discussion applies principally to the Venura County portion of the drainage area which is utilized by only a few wells.

Formations in the Ventura County portion of the Cuyama River drainage include alluvium, the Morales, Quatal, Simmler, and older sedimentary formations as well as granitic rocks. All of these are described in Chapter B-III of this appendix. The principal water-bearing formation presently utilized is alluvium of Recent and Pleistocene age which appears to be 60 to 100 feet thick in the valey areas. The morales and portions of the Quatal formations, although not presently utilized by wells, appear from surface lithology to be potentially good sources of ground water. Pronounced lowering of water levels may occur in the area if the ground water is utilized, because of low rainfall and probable limit recharge in the area of outcrop of the formations.

Upper Portions of Piru Creek Drainage

Areas in the upper portion of Piru Creek drainage where a few water we are found include Lockwood Valley and Hungry and Peace Valleys. The latter two valleys are located in the alluvial areas shown on Plate 10 near the northeast corner of Ventura County.

In Lockwood Valley alluvium is very thin, and the few water wells in the area appear to be obtaining ground water from the continental sediments of Mioces age. One well in these sediments, however, had such a high boron content that a young apple orchard was destroyed by application of the water. The volume of ground water which is available for use is unknown, but is probably small.

In Hungry and Peace Valleys a few water wells obtain a supply from the relatively thin alluvium and from sand and gravel of the underlying Ridge Basin, group of sediments of Pliocene age. Two wells in the upper part of the Ridge Basin group reportedly yield large amounts of water for irrigation purposes. It is not known whether ground water storage and recharge is sufficient for potent: future irrigation uses.

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STRATIGRAPHIC COLUMNS - VENTURA COUNTY REGION

| | AGE | | SANTA MONICA MOUNTAINS CONEJO ANO TIERRA REJADA BASINS- MALIBU HYOROLOGIC UNIT | OAK RIDGE-SOUTH MOUNTAIN EAST LAS POSAS, WEST LAS POSAS AND SANTA ROSA BASINS | SIMI HILLS-SANTA SUSANA MOUNTAINS SIMI SUBUNIT | COASTAL PLAIN MOUNO, OXNARO FOREBAY, OXNARO PLAIN ANO PLEASANT VALLEY BASINS | VENTURA RIVER DRAINAGE AND SANTA PAULA BASIN UPPER ANO LOWER VENTURA RIVER BASINS OJAL AND UPPER OJAL BASINS | | | | | | | | |
|----------------------------------|--------------------|---|--|--|---|--|--|--|----------------------------|--------|--------|--------|--------|--------|--|
| ARY | RECENT | | ALLUVIUM AND TERRACE DEPOSITS: Clay wond and grovel, O-60' thick. | STREAM AND TERRACE DEPOSITS: Clay and sill with sand and gravel in some areas, 0-200' thick. | SIMI BASIN: Stream, terroce, alluvial fan and swamp deposits, 0–200' thick SANTA SUSANA MOUNTAINS: Clays, sonds and gravels, 0–50' thick — — — — Unconformity — — — — | OXNARO PLAIN: Stream deposits sand and gravel 20' ELSEWHERE: Cloy, silt and sand, 0 - 50' thick. Stream, swamp and logunal deposits of cloy, sond and gravel, 200 - 400' thick. Oxnard aquifer | Stream depasiled clay, sand and gravel, 0-700' thick. | 5 | | | | | | | |
| QUATERN | PLEISTOCENE | VER | Unconformity | Unconformity SAN PEORO FORMATION: Morine cky, sand and gravel, 500 - 2000' thick Epworth gravels, near top af farmatian, 0-300'. Fax Canyon member - sand and gravel, 100'-300' thick at base af farmatian. | SIMI BASIN: Canlinental clays, sands and gravely 0—500' thick. | Unconformity SAN PEDRO FORMATION: Marine and nen-marine clay, sand and gravel, 600-4000' thick. Fox Conyon member-sand and gravel, 100-300' thick in Oxnard Plain and Placeant Valley at base of formation. | SAN PEDRO FORMATION: Marine and non-marine clay, sand and gravel, 4000' thick. | 60 | | | | | | | |
| | | TON | Missing | Unconformity SANTA BARBARA FORMATION: Marine ctay, slit sond and gravel, 1000 - 2000 thick. Grimss Canyan member-sond and gravel, 100-1000 thick near tap of tarmatian. | Unconformity SANTA BARBARA FORMATION: Marine sand and gravel with some clay, 1000 ¹ thick, | SANTA BARBARA FORMATION Marine clay 600 – 4000' thick Grimes Canyan member-sand and gravel, 0–300' thick in Pleasant Valley at tap at formation. | SANTA BARBARA FORMATION: Marine clay and silt, 4000° thick. | | | | | | | | |
| | PLIOCENE | | | Unconformity PICO FORMATION: Brown and gray stillstane, sand- stane and conglamerate, 0 - 900' thick. Unconformity | Unconformity PICO FORMATION: Brown and gray silistons, sand- | Unconformity PICO FORMATION: Shale, sendstane and conglomeral | PICC FORMATION. Shale, sendstone and cargiam- | | | | | | | | |
| | | æ | "SANTA MARGARITA" FORMATION: Distomaceous shole and sandstone, 1300' thick. | Missing | SANTA MARGARITA" FORMATION: Siliceous shale, 2000 thick. | 12000' ihick. Unconformity | SANTA MARGARITA" FORMATION Siliceous shole | F | | | | | | | |
| | | MIDDLE UPPER | MIDDLE UPPEI | MIDDLE UPPE | MIDDLE UPPE | Unconformity Madeta shale and sandstone, 1000' thick. | and sandstone, 1000' thick. Modelo shale, 1500' thick Madelo shale and sand stane valcanic intrusion at base. Unconformity "SANTA MARGARITA" AND MODELO FORMATION Shale and sandstone. Shale and sandstone. | "SANTA MARGARITA" AND MODELD FORMATIONS: Shole and sandstone. | and sandslane, 1800 Thick. | | | | | | |
| | MIOCENE | | | | | MIDDLE | MIDDLE | MIDDLE | MIDDLE | MIDDLE | MIDDLE | MIDDLE | MIDDLE | MIDDLE | TOPANGA FORMATION: morine sandstone, conglomerate and brown to block shale 5000 - 9000' thick. |
| RY | | LOWER | VAQUEROS FORMATION: Not exposed | VAQUEROS FORMATION: Brown shale and sand- stone, 400' thick. | VAQUEROS FORMATION: Brown shale and sand- stane, 100-1800' thick. | | RINCON FORMATION: Groy, brown, nodular marine shale, 2000' thick. VAOUEROS FORMATION Well cemented sandstane, Interbedded shale, 300- 450' thick. | F | | | | | | | |
| TERTIA | OLIGOCE NE | ENE SESPE FORMATION: Not exposed. | | SESPE FORMATION: Continental, massive sand- stans, canglamerate with red, gray and gresn shate and sillstane, 7000' thick. | SESPE FORMATION: Cantinental, massive sand- stane, conglamerate with red, gray and green shate and siltstane, 7300' thick. | | Unconformity SESPE FORMATION: Continental, massive sandstane conglamerate and red and green shole, 4500' thick | | | | | | | | |
| | EOCENE | UPPER | | | | | COLDWATER SANDSTONE: Marine 2200' thick. COZY DELL SHALE: Marine, 3800' thick. Large sandstone member north of Ventura river. MATILIJA SANDSTONE: Morine, 2400' thick | | | | | | | | |
| 4 | | OWER | Marine shale and sondsione, nat exposed. | | UPPER LLAJAS FORMATION: Sandstone with some shale, 2000' thick. LOWER LLAJAS FORMATION: Shale, 2000-3000' thick. | Older racks nat exposed. | JUNCAL SHALE AND SANOSTONE: Marine, 5000' thick. Includes basal Ilmestane. Unconformity | | | | | | | | |
| | PALEOCENE | | NE SANTA SUSANA-MARTINEZ FORMATION: Un- divided, shale sandstone and conglomerate,3500 thick. | | | Missing ar nat recognized. | | | | | | | | | |
| CRETACEOUS | | ACEOUS Marine shale and sondstane, not expased. | | shale ond sondstane, not expased. | | | Marine slitstone, shale and conglamerate, 5000 ¹ thick. Base not exposed. | | | | | | | | |
| | | | Foull | | Bose not exposed | NOTE: Since stratigraphic units do shed boundary, the column h any plates. The various sub | not generally conform with the delineated basin o eadings refer only to generalized areas not indicat units or basins are listed beneath the column her | r d | | | | | | | |
| | PRE- CRETACEOUS | | Basement complex, not exposed. | | | to Indicate the stratigraphi | c relationships therein. | T | | | | | | | |
| CRETACEOUS PRE- CRETACEOUS | | | Marine shale and sondstane, not expased. <i>Fault</i> Basement complex, not exposed. | | Upper Cretaceous, marina, mossiva sandstane with same shala, 5500' thick. Bose not exposed | NOTE: Since stratigraphic units do shed boundary, the column h any plates. The various sub to indicate the stratigraphi | Marine thick not gener eadings ri units or c relatior | slitstone, shote and canglomerote, 5000 ¹ . Base not exposed. "olly conform with the delineated basin o efer only to generalized areas not indicat bosins are listed beneath the column her ships therein. | | | | | | | |

| SESPE FILLMOF TH | -PIRU CREEK AREA RE-PIRU SUBUNITS NORTH OF IE SANTA CLARA RIVER | CUYAMA RIVER DRAINAGE AREA | | EASTERN SANTA CLARA RIVER DRAINAGE AREA EASTERN BASIN | | |
|---|--|---|--|---|--|--|
| ilream deposile 0~250' thi | id sand and gravel with same clay ck. | Stream and terroce depa same clay, 0 - 100' th | sils: sand, gravet and lick. | Stream and lerrace deposits. Sand and gravs1, 0—150' thick. | | |
| | Unconformity | | ormity | | ormity | |
| SAN PECRO FO clay, sand a | IRMATION: Morine and non-morine and gravel, 4000'thick. RA FORMATION: Marine clay and | Missin | 9 | Missing or no | r recognized. | |
| Bilt, 4000' PICO FORMATI erale, BOOO | thick. ION: Shale, sandstans and conglom- ' thick, | MORALES FORMATION C grovel, 4000' Ihick. | ontinental clay, sand ond | RIOGE BASIN GROUP. Continental shals, sand- stane and canglomerate, 18000' thick | SAUGUS FORMATION: Continental clay, sand and gravel, 2500' thick, PICO FORMATION: Shole sandstone, and con- glamerale, 5500' thick. | |
| SANTA MARG | Unconformity ARITA" FORMATION: Shate and sond- | Unconfe | ormify | Unconformity Missing | SANTA MARGARITA | |
| stone, 1000 thick. | | | | MODELO SANOSTONE ANO | SHALE: 2000' thick | |
| | | | | MINT CANYON FORMATION | Continental sandstone, shale | |
| MODELO | Sondstone, 900' thick | | | and congiamerate, 4000 | prmity | |
| 2200'1a 6500'1hick | Shole, 400'- 1600' thick. | OUATAL FORMATION: Con clay, sondstone and co | ntinantol rad gypsiferous inglomerote, 4500' thtck. | | | |
| | Sandstane, 200'~ 2000' thick, | | | | | |
| | Shale, 500' thick. | , | | Miss | ing | |
| AQUEROS FO | RMATION: Shale and sandstane 250 | Unconfe | prmity | | | |
| SESPE FORMA lanticulor ca shale, 3800 | ATION: Continental massive sandstane, nglamerote and red gray and block ' thick. | , SIMMLER FORMATION: Continental shals, red_sand- stone and canglomerate, 4000' thick. Intrusive andesites near Lockwaod valley | | VASOUEZ FORMATION Conlinental shate, sondstane and conglomerate, 9000 thick, plus basalite flaws, 4000 thick near base. | | |
| | | Unconf | ormity | Uncon | formity | |
| COLDWATER S COZY DELL S Over 5000 MATILIJA SAN | SANDSTONE; Marine, 500 thick. HALE: Marine, missing in some areas, o' thick in , others. DSTONE: Marine, 4000' thick. | Undivided marine shole and sandstane South of Nocimienta Lautt | PATTIWAY FORMATION Brackish and cantinental shale and sendstone 2500' thick | Missing or n | at recognized. | |
| | | Uncont | ormity | 1 | | |
| | | Missing or not | : recagnized | MARTINEZ FORMATION: canglamerote, 5000 <i>Foull or L</i> | Merine sandstone and Thick. <i>Inc onformity</i> | |
| vider roc | , noi expased or recognized. | Marine shele, sandistone a Thick. | nd conglamerate, 5000' | Missie | | |
| on ngs | | Four | // | BASEMENT COMPLEX. G | iranite, granodiorite, quartz | |







ESTIMATES OF COST

APPENDIX C

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| | |

ESTIMATED COST OF CASITAS DAM AND RESERVOIR WITH STORAGE CAPACITY OF 92,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

| Elvation of crest of dam: 523 feet, | Capacity of reservoir to crest of |
|---|-------------------------------------|
| I.S.G.S. datum | spillway: 92,000 acre-feet |
| Envation of crest of spillway: 503 feet | Capacity of spillway with 10.6-foot |
| Height of dam to spillway crest, | freeboard: 17,000 second-feet |
| bove stream bed: 178 feet | |

| | : | : | Unit | : | |
|-----------------------------|---------------|---------|----------|-----------|-------------|
| Item | : Quantit | ty : | price | : Cos | st |
| CPTTAL COSTS | | | | | |
| | | | | | |
| Dia | | | | | |
|)iversion of stream and de- | | | | | |
| watering of foundation | | | lump sum | \$ 10,000 | |
| Excavation, stripping | 740.000 | , | * | 010 000 | |
| Stream bed, common | 780,300 | cu.yd. | ₽ 0.41 | 319,900 | |
| rock Abutmenta mandom | 41,000 | cu.ya. | | 45,100 | |
| Right shutment slide area | 290,500 | cu.ya. | 0.00 | 200,700 | |
| Excavation, from horrow | ,000 | cu.yu. | 0.02 . | 41,000 | |
| nits | 4.319.400 | cu.vd. | Q. 44 | 1,900,500 | |
| Embankment, compacted | 4.715.400 | cu.vd. | 0.24 | 1.131.700 | |
| Gravel fill, pervious | ., | | | | |
| drain | 54,400 | cu.yd. | 4.60 | 250,200 | |
| Rock riprap | 55,300 | cu.yd. | 4.00 | 221,200 | |
| Drilling grout holes | 8,750 | lin.ft. | 3.00 | 26,300 | |
| Pressure grouting | 4,400 | cu.ft. | 4.00 | 17,600 | |
| Slope stabilization, | | | | | # |
| planting | 17.2 | acres | 1,000.00 | 17,200 | \$4,241,400 |
| Sillwar | | | | | |
| Excavation | | : | | | |
| Channel | 82,600 | cu.vd. | 2.75 | 227,200 | |
| Cutoff | 690 | cu.yd. | 6.00 | 4.100 | |
| Concrete | | | | | |
| Weir and bucket | 625 | cu.yd. | 35.00 | 21,900 | |
| Walls | 520 | cu.yd. | 40.00 | 20,800 | |
| Floor | 1,400 | cu.yd. | 30.00 | 42.000 | |
| Cutoff Peinfereine star? | 690 | cu.yd. | 35.00 | 24,200 | 200 500 |
| Reinforcing steel | 323,300 | LDS. | 0.15 | 48,500 | 388,700 |
| (tlet Works | | | | | |
| Excavation | | | | | |
| Stripping for tower | 2,000 | cu.yd. | 1.50 | 3,000 | |
| Rock, tower foundation | 160 | cu.yd. | 6.00 | 1,000 | |
| Rock, conduit trench | 1,200 | cu.yd. | 6.00 | 7,200 | |
| Concrete | | | | | |
| Tower | 465 | cu.yd. | 80.00 | 37,200 | |
| Pipe encasement | 640 | cu.yd. | 40.00 | 25,600 | |
| Reinforcing steel | 110,000 | lbs. | 0.15 | 16,500 | |
| ripe, reini. conc. 42- | 3 000 | 7.1.0. | | 000 50 | |
| Inch ala. | L ,000 | TTU.I. | L.00 | 21,000 | |

+

ESTIMATED COST OF CASITAS DAM AND RESERVOIR WITH STORAGE CAPACITY OF 92,000 ACRE-FEET (continued)

| | | | 6 / 1 d | 4 |
|-----------------------------------|-------------|------------|-----------|-----------|
| Ttem : | Quantity | Unit | : | t. |
| 2004 | Quarretoy | p1 ±00 | | |
| CAPITAL COSTS | | | | |
| Outlet Works (continued) | | | | |
| Gate valves. 30-in. dia. | 3 each | \$3,000,00 | \$ 9.000 | |
| Gate valves, 24-in. dia. | 6 each | 2,000,00 | 12,000 | |
| Floor stands and stems | | lump sum | 6,000 | |
| Reducing thimbles, | | | | |
| cast iron | | lump sum | 2,500 | |
| Miscellaneous metal work | 15,000 lbs. | 0.40 | 6,000 | 4110 rào |
| Control house | | Lump sum | 2,500 | \$149,500 |
| Paganyoin | | | c | |
| Land and improvements | | מווצ מתוון | 1,500,000 | |
| Highway relocation | | lump sum | 415,000 | |
| Relocation of utilities | | lump sum | 60,500 | |
| Clearing reservoir land | 800 acres | 150.00 | 120,000 | 2,095,5 |
| | | | | |
| Subtotal | | | 1 · · | \$6,875,1 |
| | o.d. | | | |
| Administration and engineering, 1 | 0% | | | \$ 087,5 |
| Contingencies, 15% | | | | 1,031,0 |
| interest during construction | | | | |
| TOTAL | | | | \$8,937.7 |
| | | | | w~y/J()! |
| | | | | |
| ANNUAL COSTS | | | | |
| | | | | |
| Interest, 4% | | | | \$ 357.5 |
| Amortization, 40-year sinking fun | d at 4% | | | 94,00 |
| Operation and maintenance | | | | 15,00 |
| | | | | - |
| TOTAL | | | | \$ 466,51 |
| | | | | |
| | | | | |

.

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ESTIMATED COST OF CASITAS DAM AND RESERVOIR WITH STORAGE CAPACITY OF 105,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

| Elevation of crest of dam: 533 feet | Capacity of reservoir to crest of |
|-------------------------------------|-----------------------------------|
| U.S.G.S. Datum | spillway: 105,000 acre-feet |
| Elevation of crest of spillway: 513 | Capacity of spillway with 9-foot |
| feet | freeboard: 17,000 second-feet |
| Height of dam to spillway crest, | |

above stream bed: 188 feet

| | : | | : Unit : | | |
|--------------------------|-----------|--------|-----------|-----------|-------------------------------|
| Item | : Quant | ity | : price : | Co | ost |
| CAPITAL COSTS | | | | | |
| Dam | | | | | |
| Diversion of stream and | | | | | |
| dewatering of foundation | | | lump sum | \$ 10.000 | |
| Excavation, stripping | | | 1 | | |
| Stream bed, common | 830,400 | cu.yd. | \$ 0.41 | 340,500 | |
| rock | 43,800 | cu.yd. | 1.10 | 48,200 | |
| Abutments, random | 310,400 | cu.yd. | 0.88 | 273,200 | |
| Right abutment, slide | | | | | |
| area | 50,000 | cu.yd. | 0.82 | 41,000 | |
| Excavation, from borrow | | | | | |
| pits | 5,013,800 | cu.yd. | 0.44 | 2,206,100 | |
| Embankment, compacted | 5,461,800 | cu.yd. | 0.24 | 1,310,800 | |
| Gravel fill, pervious | | | | | |
| drain | 62,700 | cu.yd. | 4.60 | 288,400 | |
| Rock riprap | 68,000 | cu.yd. | 4.00 | 272,000 | |
| Drilling grout holes | 10,330 | lin.ft | . 3.00 | 31,000 | |
| Pressure grouting | 5,170 | cu.ft. | 4.00 | 20,700 | |
| Slope stabilization, | 19.0 | | 1 000 00 | 17 000 | 21 AFR 000 |
| planting | 17.3 | acres | 1,000.00 | 17,300 | \$4 , 859 , 200 |
| Spillvor | | | | | |
| Freezetion | | | | | |
| Channel | 55 300 | ou vd | 2 75 | 152 100 | |
| Cutoff | 64.0 | cu yu. | 6 00 | 3 800 | |
| Concrete | 040 | cu.yu. | 0.00 | 0000 | |
| Weir and bucket | 590 | cu.vd. | 35.00 | 20.700 | |
| Walls | 630 | cu.vd. | 40.00 | 25,200 | |
| Floor | 1,530 | cu.vd. | 30.00 | 45,900 | |
| Cutoff | 540 | cu.vd. | 35.00 | 18,900 | |
| Reinforcing steel | 330,000 | lbs. | 0.15 | 49,500 | 316,100 |
| - | | | | | |
| Outlet Works | | | | | |
| Excavation | | | | | |
| Stripping for tower | 2,000 | cu.yd. | 1.50 | 3,000 | |
| Rock, tower foundation | 390 | cu.yd. | 6.00 | 2,300 | |
| Rock, conduit trench | 1,320 | cu.yd. | 6.00 | 7,900 | |
| Concrete | | | | | |
| Tower | 520 | cu.yd. | 80.00 | 41,600 | |
| Pipe encasement | 700 | cu.yd. | 40.00 | 28,000 | |
| Reinforcing steel | 121,900 | Ibs. | 0.15 | 18,300 | |

ESTIMATED COST OF CASITAS DAM AND RESERVOIR WITH STORAGE CAPACITY OF 105,000 ACRE-FEET (Continued)

| Item | : : Quant | : ty: | Unit : price : | Co | st |
|--|--------------|--------------|--|---|-------------------------------------|
| CAPITAL COSTS | | | | | |
| Outlet Works (Continued) Pipe, reinf. conc. 42-in. | | | | | |
| dia. | 1,100 | lin.ft. | \$ 21.00 | \$ 23,100 |) |
| Gate valves, 30-in. dia. Gate valves, 24-in. dia. Floor stands and stems Reducing thimbles, cast | 45 | each each | 3,000.00 2,000.00 lump sum | 12,000 10,000 6,500 | |
| iron Miscellaneous metal work | 20,000 | lbs. | lump sum 0.40 | 3,000 8,000 |) • • • · • • • • • |
| Control nouse | | | Taub anu | 2.,500 | Φ 100, « |
| Reservoir Land and improvements Highway relocation Relocation of utilities Clearing reservoir land | 850 | acres | lump sum lump sum lump sum 150.00 | 1,500,000 415,000 60,500 127,500 | 2,103,00 |
| Subtotal | | | | | \$7,444,50 |
| Administration and engineerin Contingencies, 15% Interest during construction | g, 10% | | | | \$ 744,40 1,116,70 372,20 |
| TOTAL | | | | | \$9 , 677,80 |
| ANNUAL COSTS | | | | | 1 |
| Interest, 4% Amortization, 40-year sinking Operation and maintenance | fund at | 4% | | | \$ 387,10 101,80 <u>18,00</u> |
| TOTAL | | | | | \$ 506,90 |

ESTIMATED COST OF CASITAS DAM AND RESERVOIR WITH STORAGE CAPACITY OF 130,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

Elevation of crest of dam: 547 feet, U.S.G.S. datum Elevation of crest of spillway: 527 feet Height of dam to spillway crest, above stream bed: 202 feet Capacity of reservoir to crest of spillway: 130,000 acre-feet Capacity of spillway with 9-foot freeboard: 17,000 second feet

| | : | | : Unit : | | |
|--------------------------|-------------|---|-----------|------------|-------------|
| Item | : Quai | ntity | : price : | Cos | st |
| CAPTTAL COSTS | | | | | |
| CALITAL CODID | | | | | |
| Dam | | | | | |
| Diversion of stream and | | | | | |
| dewatering of foundation | on | | lump sum | 1.0,000 | |
| Excavation, stripping | | | • | | |
| Stream bed. common | 986,800 | cu.vd. | \$ 0.41 | 404.600 | |
| rock | 55,800 | cu.vd. | 1.10 | 61,400 | |
| Abutments random | 1.76,300 | cu vd | 0.88 | 119 100 | |
| Right abutmont alide | 50,000 | ou yd. | 0.82 | 41,000 | |
| Execution from borrow | area Jo,000 | cu.yu. | 0.02 | 41,000 | |
| mita | 6 200 600 | ou rd | 0.11 | 2 011 000 | |
| pros | 6,00,000 | cu.yu. | 0.44 | 2,011,900 | |
| Embankment, compacted | 6,934,100 | cu.ya. | 0.24 | 1,004,200 | |
| Gravel IIII, pervious | 70 (10 | | 1 10 | 001 000 | |
| drain | 70,640 | cu.yd. | 4.60 | 324,900 | |
| Rock riprap | 120,400 | cu.yd. | 4.00 | 481,600 | |
| Drilling grout holes | 15,500 | lin.ft. | 3.00 | 46,500 | |
| Pressure grouting | 7,740 | cu.ft. | 4.00 | 31,000 | |
| Slope stabilization, | | | | | |
| planting | 19.7 | acres | 1,000.00 | 19.700 | \$6,315,900 |
| | | | | | |
| Spillway | | | | | |
| Excavation | | | | | |
| Channel | 44,950 | cu.yd. | 2.75 | 123,600 | |
| Cutoff | 610 | cu.yd. | 6.00 | 3,700 | |
| Concrete | | , i i i i i i i i i i i i i i i i i i i | | | |
| Weir and bucket | 590 | cu.vd. | 35.00 | 20,700 | |
| Walls | 610 | cu.vd. | 40.00 | 24.400 | |
| Floor | 1,170 | cu.vd. | 30.00 | 35,100 | |
| Cutoff | 510 | cu.vd. | 35.00 | 17,900 | |
| Reinforcing steel | 287 800 | lhs | 0.15 | 13,200 | 268,600 |
| | ~01,000 | 700. | 0.1) | 4),200 | ~00,000 |
| Outlet Works | | | | | |
| Excevation | | | | | |
| Stripping for tower | 2 000 | ou ard | 1 50 | 3 000 | |
| Book towar foundation | 2,000 | ou yu | 6.00 | 2,200 | |
| Pools conduit thench | 2,000 | cu, yu. | 6.00 | 2,500 | |
| Concerte Conduit Crench | 2,090 | cu.yu. | 0.00 | 12,500 | |
| Terrer | F 190 | | d0_00 | 15 (00 | |
| lower | 570 | cu.yd. | 80.00 | 45,600 | |
| Fipe encasement | 740 | cu.yd. | 40.00 | 29,600 | |
| Reinforcing steel | 129,000 | Lbs. | 0.15 | 19,400 | |
| Pipe, reini. conc. 48- | | | | 0.5. / 0.5 | |
| in. dia. | 1,150 | lin.ft. | 24.00 | 27,600 | |

ESTIMATED COST OF CASITAS DAM AND RESERVOIR WITH STORAGE CAPACITY OF 130,000 ACRE-FEET (Continued)

| | : : Unit : : Quantity : price : Cost | | | net. |
|--|---|---------------|-----------|----------------------|
| 106m | • quantity | | 00 | |
| CAPITAL COSTS | | | | |
| Outlet Works (Continued) | | | | |
| Gate valves, 30-in. dia. | 5 each | \$3,000.00 \$ | 15,000 | 1 |
| Gat valves, 24-in. dia. | 4 each | 2,000.00 | 8,000 | |
| Fioor stands and stems Reducing thimbles cast | | Lump sum | 6,500 | |
| iron | | lump sum | 3.000 | |
| Miscellaneous metal work | 20,000 lbs. | 0.40 | 8,000 | |
| Control house | | lump sum | 2,500 | \$ 183,00 |
| Reservoir | | | | |
| Land and improvements | | lump sum | 1,500,000 | |
| Highway relocation | | lump sum | 415,000 | |
| Relocation of utilities | 000 | lump sum | 60,500 | 0.130 50 |
| creating reservoir rand | 900 acres | 120.00 | | 2,110,50 |
| Subtotal | | | | \$ 8,878,00 |
| Administration and anginaomina | 109 | | | \$ 007 00 |
| Contingencies. 15% | 5, 10% | | | ⊕ 007,00 1.331.70 |
| Interest during construction | | | | 665,90 |
| momt t | | | | #== = = (= 1 =) |
| TOTAL | | | | \$11,763,40 |
| | | | | |
| | | | | |
| ANNUAL COSTS | | | | |
| Interest, 4% | | | | \$ 470,50 |
| Amortization, 40-year sinking | fund at 4% | | | 123,80 |
| Uperation and maintenance | | | | 21,0() |
| TOTAL | | | | \$ 615.30 |
| | | | | |

.
| ESTIMATED WITH STORA | COST OF CASIS GE CAPACITY (| PAS DAM OF 156, | AND RESER | RVOIR FEET | |
|---|--|--------------------|---|--|---|
| (Based on pr | ices prevail: | ing in | spring of | 1953) | |
| Elevation of crest of dam: U.S.G.S. datum Elevation of crest of spillw Height of dam to spillway cr above stream bed: 215 fee | 560 feet, ay: 540.5 fe est, t | C eet C | apacity of of spilly fæt. apacity of foot free second-fe | f reservoir way: 156,00 f spillway w eboard: 17, eet | to crest 00 acre- vith 8.5- ,000 |
| ······································ | : | : | Unit : | | |
| ltem | : Quanti | ty : | price : | Cost | ; |
| CAPITAL COSTS | | | | | |
| Dam | | | | | |
| Diversion of stream and | | | | | |
| dewatering of foundation | | | lump sum | \$ 10,000 | |
| Excavation, stripping | | | | | |
| Stream bed, common | 1,132,700 | cu.yd. | \$ 0.41 | 464,400 | |
| rock | 59,600 | cu.yd. | 1.10 | 65,600 | |
| Abutments, random | 1,404,400 | cu.yd. | 0.88 | 1,235,900 | |
| Right abutment, slide ar | ea 50,000 | cu.yd. | 0.82 | 41,000 | |
| Excavation, from borrow | | | | | |
| pits | 11,731,900 | cu.yd. | 0.44 | 5,162,000 | |
| Embankment, compacted | 12,441,800 | cu.yd. | 0.24 | 2,986,000 | |
| Gravel fill, pervious | | Ū | | | |
| drain | 77,600 | cu.yd. | 4.60 | 357,000 | |
| Rock riprap | 270,800 | cu.yd. | 4.00 | 1,083,200 | |
| Drilling grout holes | 19,850 | lin.ft | . 3.00 | 59,600 | |
| Pressure grouting | 9,950 | cu.ft. | 4.00 | 39,800 | |
| Slope stabilization, | | | | | |
| planting | 31.9 | acres | 1,000.00 | 31,900 | \$1,536,400 |
| | | | | | |
| Spillway | | | | | |
| Excavation | | | | | |
| Channel | 158,400 | cu.yd. | 2.75 | 435,600 | |
| Cutoff | 720 | cu.yd. | 6.00 | 4,300 | |
| Concrete | | | | | |
| Weir and bucket | 395 | cu.yd. | 35.00 | 13,800 | |
| Walls | 840 | cu.yd. | 40.00 | 33,600 | |
| Floor | 2,070 | cu.yd. | 30.00 | 62,100 | |
| Cutoff | 720 | cu.yd. | 35.00 | 25,200 | |
| Reinforcing steel | 403,000 | lbs | . 0.15 | 60,500 | 635,100 |
| | | | | | |
| Outlet Works | | | | | |
| Excavation | | | | | |
| Stripping for tower | 1,370 | cu.yd. | 1.50 | 2,100 | |
| Rock, tower foundation | 550 | cu.yd. | 6.00 | 3,300 | |
| Rock, conduit trench | 3.160 | cu.vd. | 6.00 | T9,000 | |

ESTIMATED COST OF CASITAS DAM AND RESERVOIR WITH STORAGE CAPACITY OF 156,000 ACRE-FEET (Continued)

| | : | | : | Unit : | | | |
|--|--------|----------|----------------|----------|------------|----------|-------------|
| Item | : | Quantit | y : | price : | | Cost | , |
| CAPITAL COSTS | | | | | | | |
| Outlet Works (Continued) Concrete | | | | | | | |
| Tower | | 693 | cu.yd. | \$ 80.00 | \$ | 55,400 | |
| Reinforcing steel | | 182,500 | cu.yd. lbs. | 40.00 | 5 | 27,400 | |
| in. dia. | | 1,740 | lin.ft | . 24.00 |) | 41,800 | |
| Gate valves, 30-in. dia. | | 63 | each | 3,000.00 |) | 18,000 | - |
| Floor stands and stems Reducing thimbles, cast | | , | | lump sur | n | 6,500 | |
| iron | | 05 000 | | lump sur | 1 | 3,000 | |
| Control house | | 25,000 | LDS. | lump sur |) 1 | 2,500 | \$ 248,40 |
| Popowin | | | | | _ | | |
| Land and improvements | | | | lump sur | 1 1 | ,500,000 | |
| Highway relocation | | | | lump sun | 1 | 415,000 | |
| Clearing reservoir land | | .1,000 | acres | 150.00 |) | 150,000 | 2,125,50 |
| Subtotal | | | | | | 4 | \$14.545.40 |
| | | 2.00 | | | | | |
| Administration and engineer: Contingencies. 15% | ing, | 10% | | | | ÷ | 2,181,80 |
| Interest during construction | n | | | | | | 1,454,50 |
| TOTAL | | | | | | ţ | 19,636,20 |
| | | | | | | | |
| | | | • | | | | |
| ANNUAL COSTS | | | | | | | |
| Interest, 4% | og fur | nd at 10 | | | | \$ | 785,50 |
| Operation and maintenance | ig iu | nu at 4% | | | | | 25,00 |
| TOTAL | | | | | | ¢ | 1.017.10 |
| 101112 | | | | | | 51 | |

ESTIMATED COST OF VENTURA RIVER DIVERSION TO CASITAS RESERVOIR WITH CONDUIT OF 100 SECOND-FOOT CAPACITY AND DIVERSION AT THE MIDDLE SITE (Based on prices prevailing in spring of 1953)

Elevation at crest of weir: 910 feet, U.S.G.S. datum Height of weir above stream bed: 10 feet

Total length of pipe line: 17,600 feet Total length of canal and flume: 14,730 feet

| | Tt.em | : • Ouentity | : Unit : | Cost |
|-----|-----------------------------|-----------------|-----------------------|----------------|
| | 2 0000 | · QUATULUY | i filce i | |
| CA | PITAL COSTS | | | |
| Di | version Works | | | |
| | Excavation | 200 cu.yd. | \$ 4.00 \$ | 800 |
| | Stripping | 870 cu.yd. | 3.00 | 2,600 |
| | Concrete | | - | |
| | Weir and cutoff | 750 cu.yd. | 35.00 | 26,300 |
| 2 | Walls | 20 cu.yd. | 50.00 | 1,000 |
| | Reinforcing steel | 2,500 lbs. | 0.15 | 400 |
| | Trash rack steel | 1,000 lbs. | 0.20 | 200 |
| | Outlet gates | | lump sum | 2,200 \$33,500 |
| Di | no Timo | | | |
| L T | Pipe reinforced concrete | | | |
| 1 | 12-inch installed | | | |
| 1 | including earthwork | 17 600 lin ft | 1 2 21 | 110 200 |
| | Air valves, blowoffs, and | | ±ر•ر∠ | 410,000 |
| | structures | | ໄມໝາວ ຮຸມໜ | 26 000 |
| | Sand trap | | lump sum | 9,800 106,100 |
| | | | | |
| Ca | nal and Flume | | | |
| | Excavation | 52,690 cu.yd. | 0.45 | 23,700 |
| | Compacted fill | 6,870 cu.yd. | 0.31 | 2,100 |
| | Shotcrete lining | 21,600 sq.yd. | 3.50 | 75,600 |
| | Flume, semicircular, metal, | | | |
| | 0.4 foot diameter, | | a . b . | |
| | Flume Comisineulan metal | 600 lin.ft. | 21.80 | 13و 13 |
| | 7 0 feet diameter | • | | |
| | including structures | 20 14 - 8+ | 19 00 | roo |
| Į | Special structure | JU 111.1 6. | | 17 000 |
| | Fam road bridges | l. each | | |
| | | 4 each | د ، د 00 ، 0 0 | <u> </u> |
| Ri | ghts of Way | | | |
| | Canal | 20 acres | 1,000.00 | 20,000 |
| | Pipe line | 21 acres | 150.00 | 3,200 23,200 |
| , | | | | |
| | Subtotal | | | \$643,600 |

ESTIMATED COST OF VENTURA RIVER DIVERSION TO CASITAS RESERVOIR WITH CONDUIT OF 100 SECOND-FOOT CAPACITY AND DIVERSION AT THE MIDDLE SITE (Continued)

| - 10 - F |
|------------------------------------|
| 64,400 96,500 32,200 |
| 836,700 |
| |
| |
| 33,500 8,800 4,000 46,300 |
| |

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ESTIMATED COST OF VENTURA RIVER DIVERSION TO CASITAS RESERVOIR WITH CONDUIT OF 150 SECOND-FOOT CAPACITY AND DIVERSION AT THE MIDDLE SITE (Based on prices prevailing in spring of 1953)

levation at crest of weir: 910 feet, U.S.G.S. datum leight of weir above stream bed: 10 feet

Total length of pipe line: 17,600 feet Total length of canal and flume: 14,730 feet

| | Tt om | o | : | Unit : | | | |
|----|-----------------------------|----------|----------|----------|----|--------------|----------|
| - | Trein | Quantity | 7 | price : | | Cost | |
| AI | PITAL COSTS | | | | | | |
| in | version Works | | | | | | |
| _ | Excavation | 200 | cu.vd. | \$ L.00 | \$ | 800 | |
| | Stripping | 870 | cu.vd. | 3.00 | τr | 2.600 | |
| | Concrete | • | | - | | | |
| | Weir and cutoff | 750 | cu.yd. | 35.00 | | 26.300 | |
| | Walls | 20 | cu.yd. | 50.00 | | 1,000 | |
| | Reinforcing steel | 2,500 | lbs. | 0.15 | | 400 | |
| | Trash rack steel | 1,000 | lbs. | 0.20 | | 200 | |
| | Outlet gates | | | lump sum | | 2,200 | \$33,500 |
| | | | | | | | - |
| iŗ | pe Line | | | | | | |
| | Pipe, reinforced concrete, | | | | | | |
| | 48-inch diameter, | | | | | | |
| | installed | 17,600 | lin.ft. | 26.62 | | 468,500 | |
| | Air valves, blowoffs, and | | | | | | |
| | structures | | | lump sum | | 28,100 | 1-1-1 |
| | Sand trap | | | lump sum | | 9,800 | 506,400 |
| | | | | | | | |
| ar | Tal and Flume | 77 5/0 | | | | 20.000 | |
| | Composted fill | 11,500 | cu.ya. | 0.45 | | 32,200 | |
| | Shotanota lining | 11,010 | cu.ya. | | | 3,000 | |
| | Flumo Somicineulon motol | 27, 520 | sq.ya. | 3.50 | | 102,000 | |
| | 7.6 foot diamoter | | | | | | |
| | including structures | 600 | lin.ft. | 29.10 | | 17 600 | |
| | Flume, Semicircular, metal. | 000 | TTUET OF | 27.40 | | 11,000 | |
| | 8.3 foot diameter. | | | | | | |
| | including structures | 30 | lin.ft. | 25.80 | | 800 | |
| | Special structures | | | lumo sum | | 17.000 | |
| | Farm road bridges | Ц | each | 2.200.00 | | 8,800 | 182.600 |
| | Ŭ | - | | | | | |
| ię | ghts of Way | | | | | | |
| | Canal | 34 | acres | 1,000.00 | | 34,000 | |
| | Pipe line | 21 | acres | 150.00 | | 3,200 | 37,200 |
| | | | | | | | |
| | Subtotal | | | | | \$ | 759,700 |

ESTIMATED COST OF VENTURA RIVER DIVERSION TO CASITAS RESERVOIR WITH CONDUIT OF 150 SECOND-FOOT CAPACITY AND DIVERSION AT THE MIDDLE SITE (Continued)

| Item | : | Quantity | : | Unit price | : | Cost |
|--|--------|----------|---|---------------|---------------------------|----------------|
| CAPITAL COSTS | | | | | | |
| Administration and engineering Contingencies, 15% Interest during construction | , 10% | | | \$ | 76,00 113,90 _38,00 | 00 00 00 |
| TOTAL | | | | \$ | 987,6 | 00 |
| ANNUAL COSTS | | | | | | |
| Interest, 4% Amortization, 40-year sinking Operation and maintenance | fund a | t 4% | | \$ | 39,50 10,40 4,00 | 00 00 00 |
| TOTAL | | | | \$ | 53,90 | 00 |

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ESTIMATED COST OF VENTURA RIVER DIVERSION TO CASITAS RESERVOIR WITH CONDUIT OF 200 SECOND-FOOT CAPACITY AND DIVERSION AT THE MIDDLE SITE

(Based on prices prevailing in spring of 1953)

levation of crest of weir: 910 feet, U.S.G.S. datum eight of weir above stream bed: 10 feet

*

Total length of pipe line: 17,600 feet Total length of canal and flume: 14,730 feet

| | • | | Unit | • | |
|--------------------------|---------|---------|------------|---------|------------|
| Item | : Quant | ity : | price | : Cos | st |
| APITAL COSTS | | | | | |
| iversion Works | | | | | |
| Excavation | 200 | cu.yd. | \$ 4.00 | \$ 800 | |
| Stripping | 870 | cu.yd. | 3.00 | 2,600 | |
| Concrete | 750 | | 25 00 | 0(200 | |
| Welr and Cutoli | (50 | cu.yd. | 35.00 | 26,300 | |
| Reinforcing steel | 2.500 | Ths. | 0.15 | 1,000 | |
| Trash rack steel | 1,000 | lbs. | 0.20 | 200 | |
| Outlet gates | -, | | lump sum | 2,200 | \$ 33,500 |
| | | | - | | |
| ipe Line | | | | | |
| Pipe, reinforced concret | е | -· ·· | | | |
| 54-inch dia., installed | 11,600 | lin.it. | 30.97 | 545,100 | |
| and structures | | | חווא ממוול | 30,000 | |
| Sand trap | | | lump sum | 12,300 | 587,100 |
| - | | | | | 2013400 |
| inal and Flume | | | | | |
| Excavation | 76,700 | cu.yd. | 0.45 | 34,500 | |
| Compacted fill | 11,610 | cu.yd. | 0.31 | 3,600 | |
| Shotcrete Lining | 31,500 | sq.yd. | 3.50 | 110,500 | |
| motal 8 3=foot dia | | | | | |
| including structures | 600 | lin.ft. | 3/1-60 | 20,800 | |
| Flume, semicircular, | | 2 | 94.00 | 20,000 | |
| metal, 8.9-foot dia., | | | | | |
| including structures | 30 | lin.ft. | 26.60 | 800 | |
| Special structures | , | | lump sum | 18,100 | |
| Farm road bridges | 4 | each | 2,200.00 | 8,800 | 197,100 |
| abt of Wor | | | | | |
| Canal | 21, | acres | 1 000 00 | 31, 000 | |
| Pipe line | 21 | acres | 150.00 | 3,200 | 37.200 |
| | | | | | |
| Subtotal | | | | | \$ 855,200 |

ESTIMATED COST OF VENTURA RIVER DIVERSION TO CASITAS RESERVOIR WITH CONDUIT OF 200 SECOND-FOOT CAPACITY AND DIVERSION AT THE MIDDLE SITE (Continued)

| Iter | n | : | Quantit | y | : | Unit price | : | Co | ost | |
|---|------------------------------------|-------------|---------|----|---|---------------|---|----|--------------------------|----------------------------|
| CAPITAL COSTS | | | | | | | | | | |
| Administration Contingencies, Interest during | and enginee 15% g constructi | ring; on | , 10% | | | | | | \$ 8 12 <u>1</u> 2 | 35,500 28,300 42,800 |
| TOTAL | | | | | | | | | \$1,11 | 1,800 |
| ANNUAL COSTS | | | | | | | | | | |
| minorial ocorra | | | | | | | | | | |
| Interest, 4% Amortization, 1 Operation and n | 40-year sink maintenance | ing f | fund at | 4% | | | | | \$ L] | 14,500 11,700 14,000 |
| TOTAL | | | | | | | | | \$ 6 | 50,200 |

ESTIMATED COST OF FERNDALE DAM AND RESERVOIR WITH STORAGE CAPACITY OF 12,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

Elevation of crest of dam: 1,100 feet, U.S.G.S. datum Elevation of crest of spillway: 1,075 feet Height of dam to spillway crest, above stream bed: 165 feet Capacity of reservoir to crest of spillway: 12,000 acre-feet Capacity of spillway with 5-foot freeboard: 37,000 second-feet

| Thom | : | : | Unit : | Quet | |
|----------------------------|-----------|------------|--------------|----------|-------------|
| LCem | : Quality | <u>/ :</u> | price : | COST | |
| CAPTTAL COSTS | | | | | |
| ONTINE COOLD | | | | | |
| Dam | | | | | |
| Exploration | | | lump sum # | 30,000 | |
| Diversion of stream and | | | Tomb bom # | ,)0,000 | |
| dewatering of foundation | | | ງມາຫຼາວ ຮຸມຫ | 10.000 | |
| Stripping topsoil | 26 600 | cu, vd. | \$ 0.60 | 16,000 | |
| Foundation excavation | ~0,000 | out ju | Ψ 0.00 | 20,000 | |
| Abutment | 208,500 | cu.vd. | 1.10 | 229.400 | |
| Channel | 20,700 | cu.vd | 0.60 | 12,400 | |
| Embankment | | | | | |
| Impervious | 902,900 | cu.vd. | 0.70 | 632,000 | |
| Pervious | 1.408.500 | cu.vd. | 0.45 | 633,800 | |
| Rock riprap | 38.000 | cu.vd. | 4.00 | 152,000 | |
| Drilling grout holes | 5,880 | lin.ft | 3.00 | 17,600 | |
| Pressure grouting | 3,920 | cu.ft. | 4.00 | 15,700 | \$1,748,900 |
| | | | | | |
| Spillway | | | | | |
| Excavation | 328,800 | cu.yd. | 2.00 | 657,600 | |
| Concrete | | | | | |
| Weir and cutoff | 1,440 | cu.yd. | 35.00 | 50,400 | |
| Floor | 1,340 | cu.yd. | 30.00 | 40,200 | |
| Walls | 840 | cu.yd. | 40.00 | 33,600 | |
| Reinforcing steel | 267,800 | lbs | 0.15 | 40,200 | 822,000 |
| - | - | | | | |
| Outlet Works | | | | | |
| Excavation | | | | | |
| Inlet structure | 300 | cu.yd. | 5.00 | 1,500 | |
| Conduit trench | 7,920 | cu.yd. | 6.00 | 47,500 | |
| Concrete | | | | | |
| Inlet structure | 220 | cu.yd | 60.00 | 13,200 | |
| Conduit encasement | 3,590 | cu.yd. | 40.00 | 143,600 | |
| Reinforcing steel | 183,900 | lbs | 0.15 | 27,600 | |
| Miscellaneous metal work | 32,400 | lbs. | 0.40 | 13,000 | |
| Steel pipe 42-inch dia. | 198,000 | lbs. | 0.28 | 55,500 | |
| High pressure slide gate | | | lump sum | 25,000 | |
| Needle valve, 36-inch dia. | | | lump sum | 12,000 | 010 000 |
| Control house, etc. | | | lump sum | 9,100 | 348,000 |

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ESTIMATED COST OF FERNDALE DAM AND RESERVOIR WITH STORAGE CAPACITY OF 12,000 ACRE-FEET (Continued)

| Item : Qua | : intity : | Unit : price : | Cost | |
|---|---------------|---|--|--|
| CAPITAL COSTS | | | | |
| Reservoir Land acquisition Improvements State road relocation Clearing | 270 acres | ·lump sum \$ lump sum lump sum \$ 150.00 | 258,800 576,700 420,000 <u>40,500</u> | \$ <u>1,296,000</u> |
| Subtotal | | | | \$4,214,900 |
| Administration and engineering, 10% Contingencies, 15% Interest during construction | | | | \$ 421,50(632,20(105,40(|
| TOTAL | | | | \$5,374,00(|
| ANNUAL COSTS | | | | |
| Interest, 4% Amortization, 40-year sinking fund at Operation and maintenance | , 4% | | | <pre>\$ 215,00(56,50(5,00(</pre> |
| TOTAL | | | | \$ 276,500 |
| | | | | |

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ESTIMATED COST OF FERNDALE DAM AND RESERVOIR WITH STORAGE CAPACITY OF 24,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

| Elevation of crest of dam: | Capacity of reservoir to crest of |
|----------------------------------|-----------------------------------|
| 1,150 feet, U.S.G.S. datum | spillway: 24,000 acre-feet |
| Elevation of crest of spillway: | Capacity of spillway with 5-foot |
| 1,120 feet | freeboard: 37,000 second-feet |
| Height of dam to spillway crest, | |
| above stream bed: 210 feet | |

| | : | | : | Unit : | | |
|----------------------------|------------|------------|---|------------|-----------|-------------|
| Item | : Quai | ntity | : | price : | Co | st |
| CADITAL COSTS | | | | | | |
| CAPITAL COSTS | | | | | | |
| Dam | | | | | | |
| Exploration | | | | ໄມຫຼາວ ເມຫ | \$ 40.000 | |
| Diversion tunnel | | | | ramb pan | ۹۶ 40,000 | |
| 15-foot diameter | 1 250 | lin ft | | \$386 00 | 1.82 500 | |
| Portal excavation | 20,200 | cu vd | | 0.80 | 16,200 | |
| Concrete nlug | 260 | cu yd | | 30.00 | 7,800 | |
| Diversion of stream and | 200 | cu, yu. | | J0.00 | 1,000 | |
| dewatering of foundatio | n | | | Jump sum | 10.000 | |
| Stripping topsoil | 37.000 | cu.vd. | | 0.60 | 22,200 | |
| Foundation excavation | 57,9000 | ousjus | | 0,00 | ~~,~~~ | |
| Abutment | 31.9.100 | cu.vd. | | 1.10 | 384.000 | |
| Channel | 25,200 | cu.yd. | | 0.60 | 15,100 | |
| Embankment | ~),~00 | ousjus | | 0.00 | 2/,200 | |
| Impervious | 1.757.400 | cu.vd. | | 0.70 | 1,230,200 | |
| Pervious | 2,3/,3,900 | cu.vd. | | 0.45 | 1.054.800 | |
| Rock riprap | 56,830 | cu.vd. | | 4.00 | 227,300 | |
| Drilling grout holes | 7,140 | lin.ft. | | 3.00 | 22,300 | |
| Pressure grouting | 1,960 | cu.ft. | | 1.00 | 19,800 | \$3.532.200 |
| | 4,700 | 04.10. | | 4.00 | | */ , / / |
| Spillway | | | | | | |
| Excavation | 160,800 | cu.vd. | | 2.00 | 321,600 | |
| Concrete | 200,000 | ou y j u v | | | Jazyejee | |
| Weir and cutoff | 970 | cu.vd. | | 35.00 | 34,000 | |
| Floor | 1.270 | cu.vd. | | 30.00 | 38,100 | |
| Walls | 590 | cu.vd. | | 40.00 | 23,600 | |
| Reinforcing steel | 220,600 | lbs. | | 0.15 | 33,100 | 450.400 |
| | ,, | | | | | |
| Outlet Works | | | | | | |
| Inlet structure concrete | 300 | cu.yd. | | 60.00 | 18,000 | |
| Inlet structure excavation | n 400 | cu.vd. | | 6.00 | 2,400 | |
| Steel pipe 60-inch dia. | 326,500 | lbs. | | 0.28 | 91,400 | |
| Reinforcing steel | 41.000 | lbs. | | 0.15 | 6,200 | |
| High pressure slide gate | , | | | lump sum | 25,000 | |
| Needle valve 48-inch dia. | | | | lump sum | 18,000 | |
| Miscellaneous metal work | 35,000 | lbs. | | 0.40 | 14,000 | |
| Control house, etc. | | | | lump sum | 11,000 | 186,000 |

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ESTIMATED COST OF FERNDALE DAM AND RESERVOIR WITH STORAGE CAPACITY OF 24,000 ACRE-FEET (Continued)

| | • • | · Unit · | | |
|--|---------------|--|---|--------------------------------|
| Item | : Quantity | : price : | Cost | , 1 |
| CAPITAL COSTS | | - | | |
| Reservoir Land acquisition Improvements State road relocation Clearing | · 340 acres | lump sum lump sum lump sum lump sum | 294,200 641,900 420,000 51,000 \$1 | L, <i>1</i> 407,100 |
| Subtotal | | | \$ | 5,575,700 |
| Administration and engineeri Contingencies, 15% Interest during construction | ing, 10% n | | \$ | 557,600 836,400 278,800 |
| TOTAL | 1 | | \$1 | 7,248,50(|
| ANNUAL COSTS | | | | |
| Interest, 4% Amortization, 40-year sinkir Operation and maintenance | ng fund at 4% | | ψ̈́ | 289,90 76,30 <u>6,50</u> |
| TOTAL | | | ÷. | 372,70 |

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ESTIMATED COST OF FERNDALE DAM AND RESERVOIR WITH STORAGE CAPACITY OF 34,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

Elevation of crest of dam: 1,180 feet, U.S.G.S. datum Elevation of crest of spillway: 1,150 feet Height of dam to spillway crest, above stream bed: 240 feet Capacity of reservoir to crest of spillway: 34,000 acre-feet Capacity of spillway with 5-foot freeboard: 37,000 second-feet

| - | : | : | Unit : | 0 | , |
|----------------------------|-----------|---------|-----------|-------------------------|------------------|
| Item | : Quai | ntity : | price : | Cos | st |
| CAPITAL COSTS | | | | | |
| Dam | | | | | |
| Exploration | | | lump sum | \$ 40,000 | |
| Diversion tunnel | | | | | |
| 15-foot diameter | 1,600 | lin.ft. | \$ 386.00 | 617,600 | |
| Portal excavation | 12,760 | cu.yd. | 0.80 | 10,200 | |
| Concrete plug | 260 | cu.yd. | 30.00 | 7,800 | |
| Diversion of stream and | | - | | | |
| dewatering of foundation | n | | lump sum | 10,000 | |
| Stripping topsoil | 68,300 | cu.yd. | 0.60 | 41,000 | |
| Foundation excavation | | | | | |
| Abutment | 423,000 | cu.yd. | 1.10 | 465,300 | |
| Channel | 29,600 | cu.yd. | 0.60 | 17,800 | |
| Embankment | | | | | |
| Impervious | 2,303,000 | cu.yd. | 0.70 | 1,612,100 | |
| Pervious | 4,031,800 | cu.yd. | 0.45 | 1,814,300 | |
| Rock riprap | 82,220 | cu.yd. | 4.00 | 328,900 | |
| Drilling grout holes | 7,920 | lin.ft. | 3.00 | 23,800 | |
| Pressure grouting | 5,280 | cu.ft. | 4.00 | 21,100 | \$5,009,900 |
| 0.433 | | | | | |
| Spillway | 2/2 000 | | 2.00 | R 0 R (00 | |
| Excavation | 363,820 | cu.ya. | 2.00 | 727,600 | |
| Voncrete | 1 260 | | 25.00 | 11 200 | |
| Weir and cutoii | 1,200 | cu.yd. | 35.00 | 44,100 | |
| r 100r | 2,110 | cu.ya. | 30.00 | 65,300 | |
| Walls Deinfensing steel | 1,030 | cu.ya. | 40.00 | 6 5 ,200 | 051 200 |
| Reinfording Steel | 340,500 | LDS. | 0.15 | | 97 1 ,500 |
| Outlet Works | | | | | |
| Inlet structure concrete | 300 | cu.vd. | 60.00 | 18,000 | |
| Inlet structure excavation | 1 L00 | cu.vd. | 6.00 | 2,400 | |
| Steel pipe 60-inch dia. | 381,600 | lbs. | 0.28 | 106,800 | |
| Reinforcing steel | 41,000 | lbs. | 0.15 | 6.200 | |
| High pressure slide gate | 42,000 | | lump sum | 25,000 | |
| Needle valve 48-inch dia. | | | lump sum | 18,000 | |
| Miscellaneous metal work | 40.000 | lbs. | 0.40 | 16,000 | |
| Control house, etc. | | | lump sum | 11,000 | 203,400 |

ESTIMATED COST OF FERNDALE DAM AND RESERVOIR WITH STORAGE CAPACITY OF 34,000 ACRE-FEET (Continued)

| | | Unit a | | |
|--|--------------|--|--|--|
| Item | : Quantity : | price : | Cos | st |
| CAPITAL COSTS | | | | - |
| Reservoir Land acquisition Improvements State road relocation Clearing | 450 acres | lump sum \$ lump sum lump sum \$ 150.00 | 294,200 641,900 420,000 <u>67,500</u> | \$ <u>1,423,60</u> |
| Subtotal | | | | \$7,588,20 |
| Administration and engineering Convingencies, 15% Interest during construction | g, 10% | | | \$ 758,80 1,138,20 <u>379,40</u> |
| TOTAL | | - 1 | | \$9,864,60 |
| ANNUAL COSTS | - | | | |
| Interest, 4% Amortization, 40-year sinking Operation and maintenance | fund at 4% | | | \$ 394,60 103,80 7,00 |
| TOTAL | | | | \$ 505,40 |

ESTIMATED COST OF COLD SPRING DAM AND RESERVOIR WITH STORAGE CAPACITY OF 35,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

| Elevation of crest of dam: | Capacity of reservoir to crest of |
|-----------------------------------|---|
| 3,400 feet, Santa Clara Water | spillway: 35,000 acre-feet |
| Conservation District datum, 1932 | Capacity of spillway with 5-foot |
| Elevation of crest of spillway: | freeboard: 50,000 second-feet |
| 3,378 feet | and the second se |
| Height of dam to spillway crest, | |
| above stream bed: 178 feet | |

| | : | : | Unit : | | |
|--------------------------|-----------|---------|----------|-----------|-------------|
| Item | : Quant: | ity : | price : | Co | st |
| CAPITAL COSTS | | | | | |
| Dam | | | | | |
| Exploration | | | lump sum | \$ 40,000 | |
| Diversion of stream and | | | - | | |
| dewatering of foundation | | | lump sum | 10,000 | |
| Stripping topsoil | 33,150 | cu.yd. | \$ 0.50 | 16,600 | |
| Foundation excavation | | | | | |
| Abutment | 63,630 | cu.yd. | 1.60 | 101,800 | |
| Channel | 45,840 | cu.yd. | 0.60 | 27,500 | |
| Embankment | | | | | |
| Impervious | 655,560 | cu.yd. | 0.74 | 485,100 | |
| Random | 1,264,070 | cu.yd. | 0.58 | 733,200 | |
| Rock riprap | 35,200 | cu.yd. | 4.00 | 140,800 | |
| Gravel fill, pervious | | | | 100 000 | |
| drain | 19,100 | cu.yd. | 5.25 | 100,300 | |
| Drilling grout noies | 4,380 | lin.it. | 3.00 | 13,100 | |
| Pressure grouting | 2,920 | cu.it. | 4.00 | 11,700 | |
| Slope stabilization, | | | 1 000 00 | 7 500 | 12 (00 (00 |
| planting | 7.5 | acres | 1,000.00 | 7,500 | \$1,687,600 |
| Spiller | | | | | |
| Excertion unalessified | 31.3 000 | ou red | 2 00 | 687 800 | |
| Concrete | 545,500 | cu.yu. | 2.00 | 007,000 | |
| Weir and cutoff | 020 | by up | 35 00 | 32 200 | |
| Floor | 1 930 | cu vd. | 30.00 | 57,900 | |
| Walls | 690 | cu.yd. | 10.00 | 27,600 | |
| Reinforcing steel | 3/1.700 | lbs. | 0.15 | 51,300 | 856,800 |
| | 24-31.40 | • | | | |
| Outlet Works | | | | | |
| Excavation | | | | | |
| Inlet structure | 300 | cu.yd. | 5.00 | 1,500 | |
| Conduit trench | 8,890 | cu.yd. | 6.00 | 53,300 | |
| Concrete | | | | | |
| Inlet structure | 220 | cu.yd. | 60.00 | 13,200 | |
| Conduit encasement | 2,960 | cu.yd. | 40.00 | 118,400 | |
| Reinforcing steel | 152,400 | lbs. | 0.15 | 22,900 | |
| Miscellaneous metal work | 32.000 | lbs. | 0.40 | 12,800 | |

ESTIMATED COST OF COLD SPRING DAM AND RESERVOIR WITH STORAGE CAPACITY OF 35,000 ACRE-FEET (Continued)

| Item | : Quantity | : Unit : : price : | Cost |
|---|-----------------|---|---|
| CAPITAL COSTS | | | |
| Outlet Works (Continued) Steel pipe 5h -inch dia. High pressure slide gate 48" Howell-Bunger valve Control house, etc. | 221,000 lb: | 5.\$ 0.28 lump sum lump sum lump sum | 61,900 25,000 12,000 9,000 330,000 |
| Reservoir Land acquisition Clearing Access road | 760 acre: | lump sum 5 50.00 lump sum _ | 25,000 38,000 40,000 103,000 |
| Subtotal | | | \$2,977,400 |
| Administration and engineerin Contingencies, 15% Interest during construction | ng, 10% | | \$ 297,700 446,600 74,400 |
| TOTAL | | | \$3,796,100 |
| ANNUAL COSTS | | | 17 |
| Interest, 4% Amortization, 40-year sinking Operation and maintenance | ; fund at 4% | | \$ 151,800 39,900 <u>7,500</u> |
| TOTAL | | | \$ 199,200 |

ESTIMATED COST OF COLD SPRING DAM AND RESERVOIR WITH STORAGE CAPACITY OF 43,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

| | a the start of the start processing the start proce |
|-----------------------------------|--|
| Elevation of crest of dam: | Capacity of reservoir to crest of |
| 3,410 feet, Santa Clara Water | spillway: 43,000 acre-feet |
| Conservation District datum, 1932 | Capacity of spillway with 5-foot |
| Elevation of crest of spillway: | freeboard: 50,000 second-feet |
| 3,390 feet | |
| Height of dam to spillway crest, | |
| above stream bed: 190 feet | |

| | : | | Unit : | - 1 | |
|--------------------------|---|---------|------------|---------|------------|
| Item | : Quant: | ity : | price : | Co | ost . |
| CAPTTAL COSTS | | | | | |
| CRITIAL COOLD | | | | | 1 |
| Dam | | | | La Le | |
| Exploration | | | lump sum 🗘 | 40,000 | |
| Diversion of stream and | | | | | |
| dewatering of foundation | _ | | lump sum | 10,000 | |
| Stripping topsoil | 35,000 | cu.yd. | \$ 0.50 | 17,500 | |
| Foundation excavation | | | - 10 | | |
| Abutment | 85,800 | cu.yd. | 1.60 | 137,300 | |
| Channel | 67,600 | cu.yd. | 0.60 | 40,600 | |
| Embankment | 952 000 | | 0.71 | 620 500 | |
| Pandom | 1 201 500 | cu.ya. | 0.74 | 808 800 | |
| Rade minner | 1, 394, 500 | cu.ya. | 0.50 | 158,000 | |
| Gravel fill pervious | 59,500 | cu.ya. | 4.00 | 150,000 | |
| drain | 10 000 | ou ud | 5 25 | 101 500 | |
| Drilling grout holes | 1,620 | lin ft. | 3.00 | 13,900 | |
| Pressure grouting | 3,080 | cu.ft. | L.00 | 12,300 | |
| Slope stabilization. | | | | | |
| planting | 8.0 | acres | 1,000.00 | 8,000 | Q1,981,400 |
| | | | | | |
| Spillway | | | | | |
| Excavation, unclassified | 370,000 | cu.yd. | 2.00 | 740,000 | |
| Concrete | | | | | |
| Weir and cutoff | 1,100 | cu.yd. | 35.00 | 38,500 | |
| Floor | 1,900 | cu.yd. | 30.00 | 57,000 | 4 |
| Walls | 610 | cu.yd. | 40.00 | 24,400 | 075 (00 |
| Reinforcing steel | 371,200 | Lbs. | 0.15 | 55,700 | 915,000 |
| Outlet Venka | | | | | |
| Exervation | | | | | |
| Inlet structure | 300 | cu .vd | 5.00 | 1 500 | |
| Conduit trench | 9,030 | cu.vd. | 6.00 | 51,200 | |
| Concrete | ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, | Jusjus | 0.00 | 24,000 | |
| Inlet structure | 220 | cu.vd. | 60.00 | 13,200 | |
| Conduit encasement | 3,010 | cu.yd. | 40.00 | 120,400 | |
| Reinforcing steel | 154,300 | lbs. | 0.15 | 23,100 | |

ESTIMATED COST OF COLD SPRING DAM AND RESERVOIR WITH STORAGE CAPACITY OF 43,000 ACRE-FEET (Continued)

.

| | | 17 | |
|---|-----------------------------|---|---|
| Item | Quantity | : Unit : | Cost |
| CAPITAL COSTS | | | |
| Outlet Works (Continued) Miscellaneous metal work Steel pipe 60-inch dia. High pressure slide gate 48" Howell-Bunger valve Control house, etc. | 32,000 lbs. 273,500 lbs. | 0.40 \$ 0.28 1ump sum 1ump sum 1ump sum | 12,800 76,600 25,000 12,000 9,000 9 347,800 |
| Reservoir Land acquisition Clearing State road relocation Access road | 850 acres | lump sum 50.00 lump sum 1, lump sum | 25,000 42,500 050,000 40,000 1,157,500 |
| Subtotal | | , | ;24,402,300 |
| Administration and engineering Contingencies, 15% Interest during construction | 3, 10% | | \$ 440,200 660,300 110,000 |
| TOTAL | | | \$5,612,800 |
| ANNUAL COSTS | • | | |
| Interest, 4% Amortization, 40-year sinking Operation and maintenance | fund at 4% | | \$ 224,500 59,000 8,000 |
| TOTAL | | | \$ 291,500 |

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ESTIMATED COST OF COLD SPRING DAM AND RESERVOIR WITH STORAGE CAPACITY OF 77,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

Elevation of crest of dam: 3,450 feet, Santa Clara Water Conservation District datum, 1932 Elevation of crest of spillway: 3,430 Height of dam to spillway crest, above stream bed: 230 feet

| | : | | | : | Unit : | ····· | |
|----------------------------|-------------|--------------|---------|----|----------|------------|-------------|
| Item | : | Quan | tity | : | price : | Cos | st |
| CAPITAL COSTS | | | | | | | |
| Dam | | | | | | | |
| Exploration | | | | | lump sum | \$ 45,000 | |
| Diversion tunnel | | 7 500 | 2 2 01 | ш | 1/0.00 | (00.000 | |
| 10-100t diameter | | 1,520 | lin.it. | \$ | 460.00 | 699,200 | |
| Portal excavation | | 3,550 | cu.ya. | | 1.50 | 5,300 | |
| Concrete plug | | 370 | cu,yd. | | 30.00 | 11,100 | |
| Diversion of stream and | | | | | - | 00.000 | |
| dewatering of foundation | n | 07 100 | | | Lump sum | 20,000 | |
| Stripping topsoil | | 37,400 | cu,yd. | | 0.50 | 18,700 | |
| Foundation excavation | | 04 000 | | | - 10 | 3 4 8 0.00 | |
| Abutment | | 98,100 | cu.yd. | | 1.60 | 157,000 | |
| Channel | | 80,100 | cu.yd. | | 0.60 | 48,100 | |
| Embankment | - | 0 da . 0 0 0 | | | 0.40 | | |
| Impervious | نو <u>ل</u> | 281,200 | cu.yd. | | 0,80 | 1,025,000 | |
| Random | 2,. | 121,800 | cu.yd. | | 0.58 | 1,230,600 | |
| Rock riprap | | 50,640 | cu.yd. | | 4.00 | 202,600 | |
| Gravel fill, pervious | | | | | | | |
| drain | | 29,400 | cu.yd. | | 5.25 | 154,400 | |
| Drilling grout holes | | 5,160 | lin.ft. | | 3.00 | 15,500 | |
| Pressure grouting | | 3,440 | cu.ft. | | 4.00 | 13,800 | |
| Slope stabilization, | | | | | | | |
| planting | | 10.7 | acres | | 1,000.00 | 10,700 | \$3,657,000 |
| | | | | | | | |
| Spillway | | | | | | | |
| Excavation, unclassified | | 213,100 | cu.yd. | | 2.00 | 426,200 | |
| Concrete | | | | | | | |
| Weir and cutoff | | 1,210 | cu.yd. | | 35,00 | 42,400 | |
| Floor | | 1,850 | cu.yd. | | 30.00 | 55,500 | |
| Walls | | 550 | cu.yd. | | 1+0.00 | 22,000 | |
| Reinforcing steel | | 281,900 | lbs. | | 0.15 | 42,300 | 588,400 |
| Outlet Works | | | | | | | |
| Inlet structure concrete | | 300 | cu.vd. | | 60.00 | 18.000 | |
| Inlet structure excavation | 1 | 400 | cu.vd. | | 5.00 | 2,000 | |
| Steel pipe 60-inch dia. | 1 | 322.200 | lbs. | | 0.28 | 90,200 | |
| Reinforcing steel | - | 41.000 | lbs. | | 0.15 | 6,200 | |
| High pressure slide gate | | ,_, | | | lump sum | 25,000 | |

ESTIMATED COST OF COLD SPRING DAM AND RESERVOIR WITH STORAGE CAPACITY OF 77,COO ACRE-FEET (Continued)

| Item | Quanti | : ty : | Unit : price : | C | ost |
|--|------------|-----------|---|--|---|
| CAPITAL COSTS | | | | | |
| Outlet Works (Continued) 54" Howell-Bunger valve Miscellaneous metal work Control house, etc. | 35,000 | lbs. (| lump sum 0.40 lump sum | <pre>\$ 18,000 14,000</pre> | \$ 184,700 |
| Reservoir Land acquisition Clearing State road relocation Access road | 1,140 | acres | lump sum 50.00 lump sum lump sum | 25,000 57,000 1,050,000 <u>40,000</u> | <u>1,172,000</u> |
| Subtotal | | | | | \$5,602,100 |
| Administration and engineering Contingencies, 15% Interest during construction | z, 10% | | | | <pre>\$ 560,200 840,300 280,100</pre> |
| TOTAL | | | | | \$7,282,700 |
| | | | | | |
| ANNUAL COSTS | | | | | |
| Interest, 4% Amortization, 40-year sinking Operation and maintenance | fund at 43 | 6 | | | <pre>\$ 291,300 76,600 10,000</pre> |
| TOTAL | | | | | \$ 377,900 |

ESTIMATED COST OF COLD SPRING DAM AND RESERVOIR WITH STORAGE CAPACITY OF 100,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

| Elevation of crest of dam: | Capacity of reservoir to crest of |
|-----------------------------------|-----------------------------------|
| 3,472 feet, Santa Clara Water | spillway: 100,000 acre-feet |
| Conservation District datum, 1932 | Capacity of spillway with 5-foot |
| Elevation of crest of spillway: | freeboard: 50,000 second-feet |
| 3,452 feet | |
| Height of dam to spillway crest, | |
| above stream bed: 252 feet | |

| | : | | : Unit : | | |
|---------------------------|-----------|---------|-----------|-----------|-------------|
| Item | : Quan | tity | : price : | C | ost |
| CAPTTAL COSTS | | | | | |
| CALLIAL CODID | | | | | |
| Dom | | | | | |
| Exploration | | | Jump sum | \$ 50,000 | |
| Diversion tunnel | | | Tranh Pan | Ψ)0,000 | |
| 16-foot diameter | 1,520 | lin.ft. | \$ 160.00 | 699,200 | |
| Portal excavation | 3,600 | cu.vd. | 1,50 | 5,400 | |
| Concrete plug | 370 | cu.vd. | 30.00 | 11,100 | |
| Diversion of stream and | 510 | •••• | 50000 | | |
| dewatering of foundation | on | | lumo sum | 22,000 | |
| Stripping topsoil | 48,860 | cu.vd. | 0.50 | 24.400 | |
| Foundation excavation | | | | , | |
| Abutment | 132,600 | cu.vd. | 1.60 | 212,200 | |
| Channel | 119,200 | cu.yd. | 0.60 | 71,500 | |
| Embankment | | Ŭ | | | |
| Impervious | 1,534,700 | cu.yd. | 0.83 | 1,273,800 | |
| Random | 3,034,400 | cu.yd. | 0.60 | 1,820,600 | |
| Rock riprap | 64,140 | cu.yd. | 4.00 | 256,600 | |
| Gravel fill, pervious | | | | | |
| drain | 38,800 | cu.yd. | 5.25 | 203,700 | |
| Drilling grout holes | 5,520 | lin.ft. | 3.00 | 16,600 | |
| Pressure grouting | 3,680 | cu.ft. | 4.00 | 14,700 | |
| Slope stabilization, | | | | | |
| planting | 13.3 | acres | 1,000.00 | 13,300 | \$4,695,100 |
| 0.177 | | | | | |
| Spillway | | | | | |
| Excavation, unclassified | 185,800 | cu.yd. | 2.00 | 371,600 | |
| Concrete | 1 000 | | 05.00 | | |
| Weir and cutoli | 1,230 | cu.yd. | 35.00 | 43,100 | |
| FLOOP | 1,800 | cu.ya. | 30.00 | 54,000 | |
| Reinforginà steel | 201 100 | cu.ya. | 40.00 | 22,800 | 522 800 |
| tterniorcring bleer. | 201,400 | LDS. | 0.15 | 42,200 | 533,700 |
| Outlet Works | | | | | |
| Inlet structure concrete | 300 | cu, vd | 60.00 | 18 000 | |
| Inlet structure excavatio | on 1.00 | cu.yd. | 5.00 | 2,000 | |
| Steel pipe 60-inch dia. | 322,200 | lbs. | 0.28 | 90,200 | |

ESTIMATED COST OF COLD SPRING DAM AND RESERVOIR WITH STORAGE CAPACITY OF 100,000 ACRE-FEET (Continued

| | • | • | linit • | | |
|---|--|--------------------------------|--|---|----------------------------------|
| Item | : Quant | ity ; | price : | Co | st |
| CAPITAL COSTS | | | | | |
| Outlet Works (Continued Reinforcing steel High pressure slide g 54" Howell-Bunger valv Miscellaneous metal w Control house, etc. |) 41,000 ate ze ork 35,000 | lbs. \$ 1 1 1bs. 1 | 0.15 \$ ump sum ump sum 0.40 ump sum | 6,200 25,000 18,000 14,000 11,300 | \$ 184,700 |
| Reservoir Land acquisition Clearing State road relocation Access road | 1,290 | l acres l l | ump sum 50.00 ump sum ump sum | 25,000 64,500 1,050,000 40,000 | 1,179,500 |
| Subtotal | | | | | \$6,593,000 |
| Administration and engine Contingencies, 15% Interest during constru | neering, 10% ction | | | | \$ 659,300 989,000 329,700 |
| TOTAL | | | | | \$8 ,571,00 (|
| ANNUAL COSTS | | | | | |
| Interest, 4% Amortization, 40-year s Operation and maintenan | inking fund at ce | 4% | | | \$ 342,800 90,200 12,50 |
| TOTAL | | | | | \$ 445,501 |

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ESTIMATED COST OF TOPATOPA DAM AND RESERVOIR WITH STORAGE CAPACITY OF 50,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

| Elevation of crest of dam: | Height of dam to top of gates, above |
|------------------------------------|--------------------------------------|
| 2,395 feet, U.S.G.S. datum | stream bed: 280 feet |
| Elevation of crest of chute spill- | Capacity of reservoir to top of |
| way: 2,360 feet | gates: 50,000 acre-feet |
| Elevation of crest of overpour | Capacity of spillways with 5-foot |
| spillway: 2,380 feet | freeboard: 82,000 second-feet |
| Elevation of top of gates, chute | |
| spillway: 2.380 feet | |

| Ttem | : : Quanti | itv | Unit | : C | ost. |
|--|--------------------------------------|---------------------------------------|--|--|---|
| CAPITAL COSTS | - Quarres | | | | |
| Dam Concrete Excavation Drilling grout holes Pressure grouting Diversion of stream Exploration | 287,000 100,000 9,000 7,000 | cu.yd. cu.yd. lin.ft. cu.ft. | <pre>\$ 18.00 3.00 3.00 4.00 Lump sum lump sum</pre> | \$5,166,000 300,000 27,000 28,000 50,000 35,000 | \$ 5,606,000 |
| Chute sp illway Concrete Excavation Reinforcing steel Gates and hoists | 4,500 155,000 380,000 3 | cu.yd. cu.yd. lbs. each | 40.00 4.00 0.15 2 5,00 0.00 | 180,000 620,000 57,000 75,000 | 932,000 |
| Outlet Works | | | lump sum | | 60,000 |
| Reservoir Land acquisition Roads to dam Clearing | | | lump sum lump sum lump sum | 25,000 400,000 20,000 | 445,000 |
| Subtotal | | | | | \$7,043,000 |
| Administration and engineerin Contingencies, 15% Interest during construction | ng, 10% | | | | \$ 704,000 1,056,000 <u>352,000</u> |
| TOTAL | | | | | \$9,155,000 |

ANNUAL COSTS

| Inter Amort Opera | rest, 4% tization, 40-year sinking ation and maintenance | fund at 4% | \$ 366,200 96,300 20,000 |
|-------------------------|--|------------|-----------------------------------|
| | TOTAL | | \$ 482,500 |

ESTIMATED COST OF TOPATOPA DAM AND RESERVOIR WITH STORAGE CAPACITY OF 75,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

| Elevation of | crest of dam: |
|--------------|---------------------|
| 2,437 feet | , U.S.G.S. datum |
| Elevation of | crest of chute |
| spillway: | 2,400 feet |
| Elevation of | crest of overpour |
| spillway: | 2,420 feet |
| Elevation of | top of gates, chute |
| spillway: | 2,420 feet |

Height of dam to top of gates, above stream bed: 322 feet Capacity of reservoir to top of gates: 75,000 acre-feet Capacity of spillways with 5-foot freeboard: 82,000 second-feet

| Item | Quantii | : | Unit : | Co | |
|--|---|-------------------------------------|---|--|------------------------------------|
| CAPITAL COSTS | QUALIUT | | | 001 | 50 |
| | | | | | |
| Dam Concrete Excavation Drilling grout holes Pressure grouting Diversion of stream Exploration | 412,000 ct 190,000 ct 11,000 15 9,000 ct | u.yd. 4 u.yd. in.ft. u.ft. | 18.00 3.00 3.00 4.00 lump sum lump sum | \$7,416,000 570,000 33,000 36,000 50,000 35,000 | \$8 ,140,0 0(|
| Chute spillway Concrete Excavation Reinforcing steel Gates and hoists | 4,300 cr 146,000 cr 360,000 3 e | u.yd. u.yd. lbs. each | 40.00 4.00 0.15 25,000.00 | 172,000 584,000 54,000 75,000 | 885,00(|
| Outlet Works | | | lump sum | | 60,000 |
| Reservoir Land acquisition Roads to dam Clearing | | | lump sum lump sum lump sum | 25,000 400,000 _30,000 | <u></u> |
| Subtotal | | | | | \$9,540,000 |
| Administration and engineering Contingencies, 15% Interest during construction | , 10% | | | | \$ 954,000 1,431,000 596,000 |
| TOTAL | | | | \$ | 12 ,521, 000 |
| | | | | | |

| Interest, 4% Amortization, 40-year sinking fund at 4% Operating and maintenance | \$ 500,800 131,700 20,000 |
|---|------------------------------------|
| TOTAL | \$ 652,500 |

ESTIMATED COST OF TOPATOPA DAM AND RESERVOIR WITH STORAGE CAPACITY OF 100,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

| Elevation of crest of dam: 2,470 | Height of dam to top of gates, above |
|------------------------------------|---|
| feet, U.S.G.S. datum | stream bed: 355 feet |
| Elevation of crest of chute spill- | Capacity of reservoir to top of |
| way: 2,435 feet | gates: 100,000 acre-feet |
| Elevation of crest of overpour | Capacity of spillways with 5-foot |
| spillway: 2,455 feet | freeboard: 82,000 second-feet |
| Elevation of top of gates, chute | and the second se |
| spillway: 2,455 feet | |

| | : | : | Unit | : | |
|--|--|---------------------------------------|--|---|--|
| Item | : Quantit | ty : | price | : 0 | ost |
| CAPITAL COSTS | | | | | |
| Dam Concrete Excavation Drilling grout holes Pressure grouting Diversion of stream Exploration | 522,000 c 275,000 c 13,000 l 11,000 c | cu.yd. cu.yd. lin ft. cu.ft. | \$ 18.00 3.00 3.00 4.00 lump sum lump sum | \$9,396,000 825,000 39,000 44,000 50,000 35,000 | \$10 ,389,000 |
| Chute spillway Concrete Excavation Reinforcing steel Gates and hoists | 4,100 c 122,000 c 350,000 3 | cu.yd. cu.yd. lbs. each 2 | 40.00 4.00 0.15 25,000.00 | 164,000 488,000 52,500 75,000 | 779,500 |
| Outlet Works | | | lump sum | 1 | 60,000 |
| Reservoir Land acquisition Roads to dam Clearing Subtotal | | | lump sum lump sum lump sum | 62,500 400,000 40,000 | <u>502,500</u> \$11,731,000 |
| Administration and engineering Contingencies, 15% Interest during construction TOTAL | g, 10% | | | | \$ 1,173,000 1,760,000 <u>880,000</u> \$15,544,000 |
| ANNUAL COSTS Interest, 4% Amortization, 40-year sinking Operation and maintenance | fund at 4% | 2 | | | <pre>\$ 621,800 163,500 _20,000</pre> |

TOTAL

805,300

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| ESTIMATED COST OF HAMMEL DAM AND RESERVOIR WITH STORAGE CAPACITY OF 25,000 ACRE-FEET | | | | | | |
|---|---------------------------------------|---|---|--|---|--|
| (Based on pi | rices prev | vailing : | in spring of | : 1953) | | |
| Elevation of crest of dam: I feet, U.S.G.S. datum Elevation of top of gates: I feet Height of dam to top of gates stream bed: 330 feet | L,125 L,120 S, above | Capac gat Capac fre | ity of resen es: 25,000 ity of spill eboard: 90, | rvoir to top acre-feet Way with 5- ,000 second- | o of -foot -feet | |
| | : Quan | tity | : Unit : price : | Co | os t | |
| CAPITAL COSTS | | | | | | |
| Dam Exploration Diversion tunnel 7-foot diameter Portal excavation | 490 2,000 | lin.ft. cu.yd. | lump sum \$ 140.00 2.00 | \$ 90,000 68,600 4,000 | | |
| Concrete plug Diversion of stream and dewatering of foundation | 40 | cu.yd. | 30.00 lump sum | 1,200 40,000 | | |
| Stripping Mass concrete Cooling concrete Parapet wall concrete Drilling grout holes | 179,400 530,700 530,700 180 | cu.yd. cu.yd. cu.yd. cu.yd. lin.ft. | 3.00 15.00 0.50 50.00 | 538,200 7,960,500 265,300 9,000 | | |
| Pressure grouting Spillway Reinforced concrete Walls | 2,200 | cu.ft. | 3.00 | <u> 6,600</u> <u> 59,600</u> | \$8,996,600 | |
| Piers Gates and hoists Reinforcing steel Bridge | 740 928,000 549,000 | cu.yd. lbs. lbs. | 50.00 0.32 0.15 lump sum | 37,000 297,000 82,300 20,000 | 495,900 | |
| Outlet Works Ring seal gates Needle valve 48-inch dia. Steel pipe 54-inch dia. Trashrack steel | 120,000 32,000 75,000 90,000 | lbs. lbs. lbs. lbs. | 0.45 0.55 0.28 0.40 | 54,000 17,600 21,000 36,000 | | |
| Miscellaneous metal work Reservoir | 371,000 | lbs. | 0.40 | 148,400 | 277,000 | |
| Access road Clearing Land and improvements | 2 210 | miles acres | 50,000.00 150.00 lump sum | 100,000 31,500 12,500 | 144,000 | |
| Subtotal | | | | | \$9 ,913,5 00 | |
| Administration and engineerin Contingencies, 15% Interest during construction | ng, 10% | | | | \$ 991,300 1,487,000 <u>495,700</u> | |
| TOTAL | | | | \$ | 12,887,500 | |

ESTIMATED COST OF HAMMEL DAM AND RESERVOIR WITH STORAGE CAPACITY OF 25,000 ACRE-FEET (Continued)

| Item | : Quantity | : Unit : price | : | Cost |
|--|---------------|-------------------|---|---------------------------------|
| ANNUAL COSTS | | | | |
| Interest, 4% Amortization, 40-year sinking Operation and maintenance | fund at 4% | | | \$ 515,500 135,600 15,000 |
| TOTAL | | | | \$ 666,100 |

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ESTIMATED COST OF HAMMEL DAM AND RESERVOIR WITH STORAGE CAPACITY OF 50,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

Elevation of crest of dam: 1,223 feet, U.S.G.S. datum Elevation of top of gates: 1,218 feet Height to top of gates, above stream bed: 428 feet Capacity of reservoir to top of gates: 50,000 acre-feet Capacity of spillway with 5-foot freeboard: 90,000 second-feet

| | : | | : Unit | : | |
|---------------------------|-----------|---------|---------------|-----------------------------|--------------|
| Item | : Quan | tity | : price | : 00 | ost |
| CAPITAL COSTS | | | | | |
| Dam | | | | | |
| Exploration | | | lump sum | \$ 110,000 | |
| Diversion tunnel | | | A | -0.1 | |
| 7-foot diameter | 560 | lin.ft. | \$ 140.00 | 78,400 | 1 |
| Portal excavation | 2,000 | cu.yd. | 2.00 | 4,000 | 1 |
| Concrete plug | 40 | cu.yd. | 30.00 | 1,200 | |
| Diversion of stream and d | e- | | 2 | 10,000 | |
| Watering of foundation | 286 200 | | Lump sum | 40,000 | |
| Surpping | 200,100 | cu.ya. | 3.00 | 050,300 | 1. |
| Cooling compate | 1,007,900 | cu.ya. | 15.00 | 10,010,500 | |
| Perspet well concrete | 1,007,900 | cu.ya. | U.50 | 333,900 | |
| Drilling grout bolog | 1. 020 | lin ft | 50.00 | 10,000 | |
| Pressure grouting | 2 280 | | 2.00 | 19,700 | ¢17 680 80 |
| riessure grouting | 200 و ز | CU+1 U+ | 3.00 | 9,000 | ф1100,000,00 |
| Spillway | | | | | |
| Reinforced concrete | | | | | |
| Walls | 2.120 | cu.vd. | 10.00 | 8/1,800 | |
| Piers | - J10 | cu.vd. | 50.00 | 37,000 | |
| Gates and hoist | 928,000 | lbs. | 0.32 | 297,000 | |
| Reinforcing steel | 725,000 | lbs. | 0.15 | 108,700 | , |
| Bridge | | | lump sum | 20,000 | 547,50 |
| - | | | - | talaanna constant farmerika | |
| Outlet Works | | | | | |
| Ring seal gates | 120,000 | lbs. | 0.45 | 54,000 | |
| Needle valve 48-inch dia. | 32,000 | lbs. | 0.55 | 17,600 | |
| Steel pipe 54-inch dia. | 124,000 | lbs. | 0.28 | 34,700 | |
| Trashrack steel | 90,000 | lbs. | 0.40 | 36,000 | |
| Miscellaneous metal work | 747,500 | lbs. | 0.40 | 299,000 | 441,30 |
| Reservoir | | | | | |
| Access road | 2 | miles | 50,000,00 | 100.000 | |
| Clearing | 320 | acres | 150.00 | 1.8.000 | |
| Land and improvements | 520 | 40100 | | 12,500 | 160.50 |
| | | | a dunio o dun | | |
| Subtotal | | | | | \$18,839,10 |

ESTIMATED COST OF HAMMEL DAM AND RESERVOIR WITH STORAGE CAPACITY OF 50,000 ACRE-FEET (Continued)

| Item | : Quantity | : | Unit price | : | Cost |
|--|-------------------|---|---------------|---|--------------------------------------|
| (PITAL COSTS | | | | | |
| Aministration and engine Ontingencies, 15% Interest during construct | ering, 10% ion | | | | \$ 1,883,900 2,825,900 942,000 |
| TOTAL | | | | | \$24,490,900 |
| | | | | | |
| INUAL COSTS | | | | | |
| nterest, 4% mortization, 40-year sind peration and maintenance | king fund at 4% | | | | \$ 979,600 257,600 15,000 |
| TOTAL | | | | | \$ 1,252,200 |

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ESTIMATED COST OF UPPER BLUE POINT DAM AND RESERVOIR WITH STORAGE CAPACITY OF 50,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

Elevation of crest of dam: 1,320 feet, U.S.G.S. datum Elevation of crest of spillway: 1,295 feet Height of dam to spillway crest, above stream bed: 205 feet

| T+ | : Ouont | | : Unit : | Cr | st. |
|--------------------------|------------|----------------|------------|-----------|-------------|
| Item | : Quan | <u>, т с у</u> | • price • | | |
| CAPITAL COSTS | | | | | |
| Dam | | | 2 | * 50.000 | |
| Exploration | | | Lump sum | ₽ 50,000 | |
| Diversion tunnel | 1 050 | 7:00 64 | # 500 00 | 625 000 | |
| 20-foot diameter | 1,250 | LIN.IU. | ₽ 500.00 | 2 800 | |
| Portal excavation | 4,000 | cu.yu. | 30.00 | 21,900 | |
| Concrete plug | 750 | cu.yu. | 50.00 | 21,700 | |
| Diversion of stream and | - | | ີງການ ອາເພ | 20,000 | |
| dewatering of foundation | E2 000 | an red | | 21,200 | |
| Stripping topsoil | 679,500 | ou yu. | 0.40 | 611 600 | |
| Foundation excavation | 079,900 | cu.yu. | 0.70 | 011,000 | |
| Imponutione | 1 867 500 | ou vd. | 0.70 | 1,307,300 | |
| Pomious | 3 118 900 | cu.yd. | 0.50 | 1,559,500 | |
| Rock ninnen | 77 000 | cu.vd. | 4.00 | 308,000 | |
| Drilling grout holes | 6 660 | lin.ft. | 3.00 | 20,000 | |
| Pressure grouting | 1,440 | cu.ft. | 4.00 | 17,800 | \$4.565.100 |
| Tressure grouting | 49440 | 041101 | 4.00 | | n • • 3 = 3 |
| Spillway | | | | | |
| Excavation | 599,900 | cu.yd. | 2.25 | 1,349,800 | |
| Concrete | | | | | |
| Weir and cutoff | 1,860 | cu.yd. | 30.00 | 55,800 | |
| Floor | 4,080 | cu.yd. | 25.00 | 102,000 | |
| Walls | 1,840 | cu.yd. | 40.00 | 73,600 | |
| Reinforcing steel | 624,100 | lbs. | 0.15 | 93,600 | 1,674,80 |
| | | | | | |
| Outlet Works | | | | | |
| Tower concrete | 660 | cu.yd. | 80.00 | 52,800 | |
| Tower excavation | 400 | cu.yd. | 5.00 | 2,000 | |
| Steel pipe 72-in. dia. | 336,600 | lbs. | 0.28 | 94,200 | |
| Tower inlet valve | | | | | |
| 36-in. dia. | 4 | each | 3,600.00 | 14,400 | |
| Howell-Bunger valve | | | | 10,000 | |
| 48-in. dia. | 1 | each | 12,000.00 | 12,000 | |
| Needle valve 48-in. dia. | 1 | each | T8,000.00 | 18,000 | |
| Sluice gate | | | Lump sum | 25,000 | |
| Miscellaneous metal work | 32,000 | lbs. | 0.40 | 12,800 | 013 000 |
| Control house, etc. | | | lump sum | 10,000 | 241,201 |

ESTIMATED COST OF UPPER BLUE POINT DAM AND RESERVOIR WITH STORAGE CAPACITY OF 50,000 ACRE-FEET (Continued)

| | • | | Unit : | | |
|---|--|-----------|-------------------------------------|-----------------------------------|--|
| | Item | Quantity | price : | Cos | t |
| CAPITAL COSTS | | | | | |
| Reservoir Land acquisi Clearing Access Road | tion | 640 acres | lump sum \$ \$ 50.00 lump sum | 33,300 32,000 <u>15,000</u> | \$ <u>80,300</u> |
| Subtotal | L · | | | : | \$6,561,400 |
| Administratior Contingencies, Interest durir TOTAL | and engineering, 15% ag construction | 10% | | | \$ 656,200 984,200 <u>328,000</u> \$8,529,800 |
| ANNUAL COSTS | | | | | |
| Interest, 4% Amortization, Operation and | 40-year sinking fu maintenance | nd at 4% | | | \$ 341,200 89,700 <u>7,500</u> |
| TOTAL | | | | : | \$ 438,400 |

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ESTIMATED COST OF BLUE POINT DAM AND RESERVOIR WITH STORAGE CAPACITY OF 50,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

Elevation of crest of dam: 1,305 feet, U.S.G.S. Datum Elevation of crest of spillway: 1,280 feet Height of dam to spillway crest, above stream bed: 215 feet

Capacity of reservoir to crest of spillway: 50,000 acre-feet Capacity of spillway with 5-foot freeboard: 100,000 second-feet

| | : | | : Unit : | | |
|--------------------------------|-----------|--------|-----------|-----------|--------------|
| Item | : Quant | ity | : price : | Co | ost |
| CAPITAL COSTS | | | | | |
| Dam | | | | | |
| Exploration | | | lump sum | \$ 50,000 | |
| Diversion of stream and | | | | | |
| dewatering of foundation | | | lump sum | 40,000 | |
| Stripping topsoil | 33,200 | cu.yd. | \$ 0.40 | 13,300 | |
| Foundation excavation | 733,100 | cu.yd. | 0.90 | 659,800 | |
| Embankment | | | | | |
| Impervious | 1,435,800 | cu.yd. | 0.70 | 1,005,100 | |
| Pervious | 2,061,900 | cu.yd. | 0.50 | 1,031,000 | |
| ROCK riprap | 55,400 | cu.ya. | 4.00 | 221,000 | |
| Processing grout notes | 4,030 | LIN.IU | . 5.00 | 12,000 | \$2 01.8 200 |
| Tressure grouting |),220 | CU.10. | 4.00 | 12,700 | \$ |
| Spillway | | | | | |
| Portal excavation | 300,000 | cu.vd. | 2.25 | 675,000 | |
| Tunnel excavation | 60,000 | cu.vd. | 15.00 | 900,000 | |
| Concrete | , | | -, | , | |
| Weir | 4,500 | cu.yd. | 30.00 | 135,000 | |
| Walls and paving | 2,500 | cu.yd. | 40.00 | 100,000 | |
| Tunnel lining | 12,000 | cu.yd. | 50.00 | 600,000 | |
| Reinforcing steel | 2,000,000 | lbs. | 0.15 | 300,000 | 2,710,000 |
| | | | | | |
| Outlet Works | | | | | |
| Excavation Towar foundation | 200 | | F 00 | 1 000 | |
| Lower Loundation | 580 | cu.ya. | 5.00 | 1,900 | |
| Concrete | 9,000 | cu.yu. | 0.00 | 50,500 | |
| Tower | 660 | cu.vd. | 80.00 | 52,800 | |
| Pipe encasement | 3,340 | cu.vd. | 40.00 | 133,600 | |
| Reinforcing steel | 283,400 | lbs. | 0.15 | 42.500 | |
| Steel pipe, 72-inch dia. | 359,600 | lbs. | 0.28 | 100,700 | |
| Gate valve 36-inch dia. | 4 | each | 3,600.00 | 14,400 | |
| Howell-Bunger valve | | | -, | | |
| 48-inch dia. | 1 | each | 12,000.00 | 12,000 | |
| Needle valve 48-inch dia. | 1 | each | 18,000.00 | 18,000 | |
| Sluice gate | 1 | each | 20,000.00 | 20,000 | |
| Miscellaneous metal work | 33,000 | lbs. | 0.40 | 13,200 | |
| Control house, etc. | | | lump sum | 10,000 | 449,600 |

ESTIMATED COST OF BLUE POINT DAM AND RESERVOIR WITH STORAGE CAPACITY OF 50,000 ACRE-FEET (Continued)

| item : | : Quantity | Unit : price : | Cost |
|---|---------------|-------------------------------------|--------------------------------------|
| CAPITAL COSTS | | | |
| Reservoir Land acquisition Clearing Access Road | 640 acres | lump sum \$ \$ 50.00 lump sum | 33,300 32,000 12,200 \$ 77,500 |
| Subtotal | | | \$6,285,300 |
| Administration and engineering, Contingencies, 15% Interest during construction | , 10% | | \$ 628,500 942,800 314,300 |
| TOTAL | - 30 | | \$8,170,900 |
| ANNUAL COSTS | | | |
| Interest, 4% Amortization, 40-year sinking a Operation and maintenance | fund at 4% | | \$ 326,800 86,000 7,500 |
| TOTAL | | | \$ 420,300 |

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ESTIMATED COST OF DEVIL CANYON DAM AND RESERVOIR WITH STORAGE CAPACITY OF 100,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

Elevation of crest of dam: 1,245 feet, U.S.G.S. datum Elevation of crest of spillway: 1,220 feet Height of dam to spillway crest, above stream bed: 240 feet Capacity of reservoir to crest of spillway: 100,000 acre-feet Capacity of spillway with 5-foot freeboard: 102,00 second-feet

| | : | | : | Unit | : | | |
|---------------------------|-----------------|---------------|----|----------|----------|-----------|-------------|
| Item | : Quar | n tity | : | price | : | Cos | st |
| CLETTELT COSES | | | | | | | |
| CAPITAL COSTS | | | | | | | |
| Dam | | | | | | | |
| Exploration | | | •• | lump sur | 1 \$ | 40.000 | |
| Diversion tunnel | | | | • | | | |
| 21-foot diameter | 1,750 | lin.ft. | \$ | 510.00 |) | 892,500 | |
| Portal excavation | 13,300 | cu.yd. | | 1.50 |) | 19,900 | |
| Concrete plug | 730 | cu.yd. | | 30.00 |) | 21,900 | |
| Diversion of stream and | | | | | | | |
| dewatering of foundation | | | | lump sur | n | 50,000 | |
| Stripping topsoil | 57,200 | cu.yd. | | 0.40 |) | 22,900 | |
| Foundation excavation | | | | | | | |
| Abutment | 105,300 | cu.yd. | | 1.40 |) | 147,400 | |
| Channel | 905,000 | cu.yd. | | 0.60 |) | 543,000 | |
| Embankment | | | | | | | |
| Impervious | 2,599,400 | cu.yd. | | 0.6 | 5 1 | ,689,600 | |
| Pervious | 3,764,100 | cu.yd. | | 0.4 | 5 1 | ,693,800 | |
| Rock riprap | 80,000 | cu.yd. | | 4.00 |) | 320,000 | |
| Drilling grout holes | 6,360 | lin.ft. | | 3.00 |) | 19,100 | |
| Pressure grouting | 4,240 | cu.ft. | | 4.00 |) | 17,000 | \$5,477,1α |
| | | | | | | | |
| Spillway | 000 000 | | | • • | | | |
| Excavation | 1,288,300 | cu.yd. | | 2.00 |) 2 | 2,576,600 | |
| Concrete | T 000 | | | | | | |
| Weir and cutoff | 5,220 | cu.yd. | | 35.00 |) | 182,700 | |
| Floor | 8,460 | cu.yd. | | 30.00 |) | 253,800 | |
| Walls | 5,180 | cu.yd. | | 40.00 |) | 207,200 | 2 1 5 (10) |
| Reinforcing steel | 1,573,700 | Lbs. | | 0.1 | · _ | 236,100 | 3,450,401 |
| Outlot Nerka | | | | | | | |
| Inlet structure concrete | 200 | ou | | 60 00 | ` | 18 000 | |
| Intet structure concrete | 500 | cu.yu. | | E 00 | , ` | 2,000 | |
| Steel nine 72-in dia | 100 | lhe | | 0.28 | , | 126,000 | |
| Beinforcing steel | 30,000 | lhe | | 0.20 | | 120,000 | |
| High pressure slide gate | 000,000 | TD2. | | ່ມຫຼາວເມ | , 1 | 25,000 | |
| Needle valve 60-in. dia | | leach | 2 | |) | 27,500 | |
| Miscell aneous metal work | 32.000 | lhs | 2 | 0.10 | | 12,800 | |
| Control house, etc. | J= j 000 | 1001 | | lump sun | 1 | 10.000 | 225,80 |

ESTIMATED COST OF DEVIL CANYON DAM AND RESERVOIR WITH STORAGE CAPACITY OF 100,000 ACRE-FEET (Continued)

| | | ** •• | |
|---|-------------|----------------------------|--------------------------------------|
| Item : | Quantity | onit : price : | Cost |
| CAPITAL COSTS | • | | |
| Reservoir Land acquisition Clearing | 1,050 acres | `lump sum \$ \$ 50.00 _ | 110,300 52,500 \$ |
| Subtotal | | | \$9,322,100 |
| Administration and engineering, Contingencies, 15% Interest during construction | , 10% | | \$ 932,200 1,398,300 466,100 |
| TOTAL | | | \$12,118,700 |
| | | | |
| ANNUAL COSTS | | | |
| Interest, 4% Amortization, 40-year sinking f Operation and maintenance | fund at 4% | | <pre>\$ 484,700 127,500 12,500</pre> |
| TOTAL | | | \$ 624,700 |

ESTIMATED COST OF DEVIL CANYON DAM AND RESERVOIR WITH STORAGE CAPACITY OF 150,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

--- Elevation of crest of dam: 1,290 feet, U.S.G.S. datum Elevation of crest of spillway: 1,265 feet Height of dam to spillway crest,

above stream bed: 285 feet

| | : | | : Unit | : | |
|---------------------------------|-------------|---------|-------------|---|--------------|
| Item | : Quan | tity | : price | : C | ost |
| | | | | | |
| CAPITAL COSTS | | | | | |
| Dom | | | | | |
| Dam | | | 7 | # 10.000 | |
| Exploration Dimonsion tunnel | | | Lump sum | \$ 40,000 | |
| 2] fact diameter | 0 100 | 14 £t | # 510 00 | 1 00/ 200 | |
| 21-1000 diameter | 2,130 | lin.it. | . \$ 510.00 | 1,086,300 | |
| Concrete plug | 12,000 | cu.ya. | 1.50 | 18,000 | |
| Diversion of stream and | 750 | cu.ya. | 30.00 | ~1,900 | |
| devotoring of foundatio | ~ | | | 50 000 | |
| Stripping topooil | 07 940 | au | Truth anu | 26,000 | |
| Foundation exception | 91,000 | cu.yu. | 0.40 | 30,700 | |
| Abutment | 101 100 | 011 TTd | 1 10 | 116 200 | |
| Channel | 1 026 200 | cu.ya. | 1.40 | 40,200 | |
| Embankment | 1,000,000 | cu.yu. | 0.00 | 021,000 | |
| Imperirious | 3 1.21 500 | ou red | 0.65 | 2 221 000 | |
| Pervious | 6 1.67 1.00 | ou yu. | 0.05 | 2 010 200 | |
| Rock ripran | 112 81.0 | cu yu. | 1.00 | 2,710,500 | |
| Drilling grout holes | 7 080 | lin ft | 3 00 | 21 200 | |
| Pressure grouting | 1,720 | cu ft. | | 18 000 | \$7 61.6 700 |
| | 4,1~0 | Cu.10. | 4.00 | | \$1,040,100 |
| Spillway | | | | | |
| Excavation | 1.280.100 | cu.vd. | 2.00 | 2,560,200 | |
| Concrete | | | ~~~~ | ~,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, | |
| Weir and cutoff | 4.250 | cu.vd. | 35.00 | 148,800 | |
| Floor | 10,520 | cu.vd. | 30.00 | 315,600 | |
| Walls | 5,490 | cu.vd. | 40.00 | 219,600 | |
| Reinforcing steel | 1,727,800 | lbs. | 0.15 | 259,200 | 3.503.400 |
| - | | | | | 292-2910 |
| Outlet Works | | | | | |
| | | | | | |
| Tower concrete | 850 | cu.yd. | 80,00 | 68,000 | |
| Tower excavation | 400 | cu.yd. | 5.00 | 2,000 | |
| Steel pipe 72-inch dia. | 673,200 | lbs. | 0.28 | 188,500 | |
| Tower inlet valves 36- | | | | | |
| in. dia. | 5 | each | 3,600.00 | 18,000 | |
| Howell-Bunger valve, | | | | | |
| 48-in. dia. | 1 | each | 12,000.00 | 12,000 | |
| Needle valve, 48-in. dia. | 1 | each | 18,000.00 | 18,000 | |
| Sluice gate | | | lump sum | 20,000 | |
| Miscellaneous metal work | 40,000 | lbs. | 0.40 | 16,000 | |
| control house, etc. | | | lump sum | 10,000 | 352,500 |

Capacity of reservoir to crest of spillway: 150,000 acre-feet Capacity of spillway with 5-foot freeboard: 102,000 second-feet
ESTIMATED COST OF DEVIL CANYON DAM AND RESERVOIR WITH STORAGE CAPACITY OF 150,000 ACRF-FEET (Continued)

| | | e | |
|---|-------------|--|---|
| : | | : Unit : | |
| Item : | Quantity | : price ; | Cost |
| CAPITAL COSTS | the gets | | |
| Reservoir Land acquisition Clearing | 1,500 acres | lump sum \$ \$ 50.00 | 110,300 |
| Subtotal | | | \$11,687,900 |
| Administration and engineering, Contingencies, 15% Interest during construction | 10% | | \$ 1,168,800 1,753,200 <u>876,600</u> |
| TOTAL | | | \$15,486,500 |
| ANNUAL COSTS | | ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, | |
| Interest, 4% Amortization, 40-year sinking f Operation and maintenance | und at 4% | | \$ 619,500 162,900 15,700 |
| TOTAL | | | \$ 798,1 00 |

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ESTIMATED COST OF SANTA FELICIA DAM AND RESERVOIR WITH STORAGE CAPACITY OF 50,000 ACRE-FEET (Based on prices prevailing in spring 1953)

| Elevation of crest of dam: 1,030 feet | Capacity of reservoir to crest of |
|--|-----------------------------------|
| Elevation of crest of spillway: 1,010 feet | spillway: 50,000 acre-feet |
| Height of dam to spillway crest, | Capacity of spillway with 5-foot |
| above stream bed: 140 feet | freeboard: 103,000 second-feet |

| Item Cost CAPITAL COSTS Dam Lump sum \$ 50,000 Diversion of stream and dewatering of foundation Stripping topsoil 30,700 cu.yd. 0.40 12,300 Foundation excavation Abutment 29,400 cu.yd. 1.40 41,200 12,300 Foundation excavation Abutment 29,400 cu.yd. 1.40 41,200 1,2,000 Channel 886,400 cu.yd. 0.55 487,500 1,600 1,600 Excavation for embankment From borrow pits 1,686,600 cu.yd. 0.47 792,700 1,000 Impervious 1,466,600 cu.yd. 0.18 264,000 264,000 Pervious 1,571,300 cu.yd. 0.18 264,000 264,400 Drilling grout holes 6,240 lin.ft. 3.00 164,400 164,400 Drilling grout holes 6,240 lin.ft. 3.00 18,700 18,700 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 15,200 Floor 8,590 cu.yd. 35.00 115,200 15,200 16,000 <th colspan="2">:</th> <th>· Uhit</th> <th>:</th> <th></th> | : | | · Uhit | : | | |
|--|---------------------------|-----------|---------|-----------------|-----------|------------|
| CAPITAL COSTS Dam Exploration lump sum \$ 50,000 Diversion of stream and dewatering of foundation lump sum \$ 50,000 Stripping topsoil 30,700 cu.yd. \$ 0.40 12,300 Foundation excavation 29,400 cu.yd. 1.40 41,200 Abutment 29,400 cu.yd. 0.47 792,700 Channel & 886,400 cu.yd. 0.47 792,700 From borrow pits 1,686,600 cu.yd. 0.47 792,700 From stream bed & 338,700 cu.yd. 0.36 301,900 Embankment Impervious 1,466,600 cu.yd. 0.18 264,000 Pervious 1,571,300 cu.yd. 0.12 188,600 Rock riprap M1,100 cu.yd. 4,000 164,400 16,600 \$2,387,900 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 Floor & 5,590 cu.yd. 30.00 257,700 2,111,600 Walls 3,010 cu.yd. 4,0.00 120,400 <t< th=""><th>Item</th><th>: Quan</th><th>tity</th><th>price</th><th>: Cost</th><th></th></t<> | Item | : Quan | tity | price | : Cost | |
| Dam Iump sum \$ 50,000 Diversion of stream and dewatering of foundation Iump sum \$ 50,000 Stripping topsoil 30,700 cu.yd. \$ 0.40 12,300 Foundation excavation Abutment 29,400 cu.yd. 1.40 11,200 Channel 886,400 cu.yd. 0.40 12,300 Foundation excavation 886,400 cu.yd. 1.40 11,200 Channel 886,400 cu.yd. 0.55 487,500 Excavation for embankment From borrow pits 1,686,600 cu.yd. 0.47 792,700 From stream bed 638,700 cu.yd. 0.36 301,900 Embankment Impervious 1,166,600 cu.yd. 0.12 188,600 Rock riprap 41,100 cu.yd. 0.12 188,600 Pervious 1,571,300 cu.yd. 0.12 18,700 Drilling grout holes 6,240 lin.ft. 3.00 18,700 Pressure grouting 4,160 cu. ft. 4.00 16,600 \$2,387,900 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 | CAPTTAL COSTS | | | | | |
| Dam lump sum \$ 50,000 Diversion of stream and dewatering of foundation Stripping topsoil 30,700 cu.yd. \$ 0.40 12,300 Foundation excavation Foundation excavation lump sum \$0,000 12,300 Foundation excavation 29,400 cu.yd. 1.40 41,200 Channel & & & & & & & & & & & & & & & & & & & | CALLIND CODID | | | | | |
| Exploration lump sum \$ 50,000 Diversion of stream and lump sum \$ 50,000 Stripping topsoil 30,700 cu.yd. \$ 0.40 12,300 Foundation excavation 29,400 cu.yd. \$ 0.40 12,300 Abutment 29,400 cu.yd. 1.40 \$ 1,200 Channel & 886,400 cu.yd. 0.47 792,700 Excavation for embankment From borrow pits 1,686,600 cu.yd. 0.47 792,700 From stream bed & 538,700 cu.yd. 0.36 301,900 Embankment Impervious 1,466,600 cu.yd. 0.18 264,000 Pervious 1,571,300 cu.yd. 0.18 264,000 164,400 Drilling grout holes 6,240 lin.ft. 3.00 18,700 16,600 \$2,387,900 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 15,200 \$2,387,900 Spillway Excavation 1,598,100 cu.yd. 30.00 257,700 \$3,010 cu.yd. 40.00 120,400 Weir and cutoff 3,290 cu.yd. 35.00 115,200 \$2,411,600 Walls | Dam | | | | | |
| Diversion of stream and dewatering of foundation lump sum 50,000 Stripping topsoil 30,700 cu.yd. \$ 0.40 12,300 Foundation excevation 29,400 cu.yd. 1.40 11,200 Abutment 29,400 cu.yd. 0.55 487,500 Excavation for embankment 886,400 cu.yd. 0.47 792,700 From borrow pits 1,666,600 cu.yd. 0.47 792,700 From stream bed 838,700 cu.yd. 0.36 301,900 Embankment Impervious 1,466,600 cu.yd. 0.18 264,000 Pervious 1,571,300 cu.yd. 0.12 188,600 Rock riprap 41,100 cu.yd. 4.00 164,400 Drilling grout holes 6,240 lin.ft. 3.00 18,700 Pressure grouting 4,160 cu. ft. 4.00 16,600 \$2,387,900 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 Floor 8,590 cu.yd. 30.00 257,700 180,000 257,700 Walls 3,010 cu.yd. 40.00 120,400 120,400 120,400 | Exploration | | | lump sum | \$ 50,000 | |
| dewatering of foundation lump sum 50,000 Stripping topsoil 30,700 cu.yd. \$ 0.40 12,300 Foundation excavation Abutment 29,400 cu.yd. 1.40 41,200 Abutment 29,400 cu.yd. 1.40 41,200 Channel 886,400 cu.yd. 0.55 487,500 Excavation for embankment From borrow pits 1,686,600 cu.yd. 0.47 792,700 From stream bed 638,700 cu.yd. 0.36 301,900 Embankment Impervious 1,466,600 cu.yd. 0.18 264,000 Pervious 1,571,300 cu.yd. 0.12 188,600 Rock riprap 41,100 cu.yd. 4.00 164,400 Drilling grout holes 6,240 lin.ft. 3.00 16,600 \$2,387,900 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 Floor 8,590 cu.yd. 30.00 257,700 180,000 2,111,600 Walls 3,010 cu.yd. 40.00 120,400 120,400 | Diversion of stream and | | | - | | |
| Stripping topsoil 30,700 cu.yd. \$ 0.40 12,300 Foundation excavation Abutment 29,400 cu.yd. 1.40 41,200 Abutment 29,400 cu.yd. 0.55 487,500 Excavation for embankment From borrow pits 1,686,600 cu.yd. 0.47 792,700 From stream bed 838,700 cu.yd. 0.47 792,700 Stripping topsoil 1,666,600 cu.yd. 0.47 792,700 From stream bed 838,700 cu.yd. 0.36 301,900 Embankment Impervious 1,466,600 cu.yd. 0.18 264,000 Pervious 1,571,300 cu.yd. 0.12 188,600 18,700 Rock riprap 41,100 cu.yd. 4.00 164,400 16,600 \$2,387,900 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 \$7,700 Walls 3,010 cu.yd. 40.00 120,400 20,400 Reinforcing steel 1,199,800 1bs. 0.15 180,000 2,111,600 | dewatering of foundati | on | | lump sum | 50,000 | |
| Foundation excavation Abutment 29,400 cu.yd. 1.40 41,200 Channel 886,400 cu.yd. 0.55 487,500 Excavation for embankment From borrow pits 1,686,600 cu.yd. 0.47 792,700 From stream bed 838,700 cu.yd. 0.36 301,900 Embankment Impervious 1,466,600 cu.yd. 0.18 264,000 Pervious 1,571,300 cu.yd. 0.12 188,600 Rock riprap 41,100 cu.yd. 4.00 164,400 Drilling grout holes 6,240 lin.ft. 3.00 18,700 Pressure grouting 4,160 cu. ft. 4.00 16,600 \$2,387,900 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 Floor 8,590 cu.yd. 30.00 257,700 120,400 Walls 3,010 cu.yd. 40.00 120,400 120,400 Reinforcing steel 1,199,800 lbs. 0.15 180,000 2,111,600 | Stripping topsoil | 30,700 | cu.yd. | \$ Q. 40 | 12,300 | |
| Abutment 29,400 cu.yd. 1.40 41,200 Channel 886,400 cu.yd. 0.55 487,500 Excavation for embankment From borrow pits 1,686,600 cu.yd. 0.47 792,700 From stream bed 838,700 cu.yd. 0.36 301,900 Embankment Impervious 1,466,600 cu.yd. 0.18 264,000 Pervious 1,571,300 cu.yd. 0.12 188,600 Rock riprap 41,100 cu.yd. 4.00 164,400 Drilling grout holes 6,240 lin.ft. 3.00 18,700 Pressure grouting 4,160 cu. ft. 4.00 16,600 \$2,387,900 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 Walls 3,010 cu.yd. 40.00 120,400 120,400 Reinforcing steel 1,199,800 lbs. 0.15 180,000 2,111,600 | Foundation excavation | | | | | |
| Channel 886,400 cu.yd. 0.55 487,500 Excavation for embankment From borrow pits 1,686,600 cu.yd. 0.47 792,700 From stream bed 638,700 cu.yd. 0.36 301,900 Embankment Impervious 1,466,600 cu.yd. 0.18 264,000 Pervious 1,571,300 cu.yd. 0.12 188,600 Rock riprap 41,100 cu.yd. 4.00 164,400 Drilling grout holes 6,240 lin.ft. 3.00 18,700 Pressure grouting 4,160 cu. ft. 4.00 16,600 \$2,387,900 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 Floor 8,590 cu.yd. 30.00 257,700 180,000 Walls 3,010 cu.yd. 40.00 120,400 180,000 2,111,600 | Abutment | 29,400 | cu.yd. | 1.40 | 41,200 | |
| Excavation for embankment 1,686,600 cu.yd. 0.47 792,700 From stream bed 638,700 cu.yd. 0.36 301,900 Embankment Impervious 1,466,600 cu.yd. 0.18 264,000 Pervious 1,571,300 cu.yd. 0.12 188,600 Rock riprap 41,100 cu.yd. 4.00 164,400 Drilling grout holes 6,240 lin.ft. 3.00 18,700 Pressure grouting 4,160 cu. ft. 4.00 16,600 \$2,387,900 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 Floor 8,590 cu.yd. 30.00 257,700 120,400 Reinforcing steel 1,199,800 1bs. 0.15 180,000 2,111,600 | Channel | 886,400 | cu.yd. | 0.55 | 487,500 | |
| From borrow pits 1,686,600 cu.yd. 0.47 792,700 From stream bed 638,700 cu.yd. 0.36 301,900 Embankment Impervious 1,466,600 cu.yd. 0.18 264,000 Pervious 1,571,300 cu.yd. 0.12 188,600 Rock riprap 41,100 cu.yd. 4.00 164,400 Drilling grout holes 6,240 lin.ft. 3.00 18,700 Pressure grouting 4,160 cu. ft. 4.00 16,600 \$2,387,900 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 Floor 8,590 cu.yd. 30.00 257,700 Walls 3,010 cu.yd. 40.00 120,400 Reinforcing steel 1,199,800 1bs. 0.15 180,000 2,111,600 | Excavation for embankment | 101 1 | | | | |
| From stream bed 838,700 cu.yd. 0.36 301,900 Embankment Impervious 1,466,600 cu.yd. 0.18 264,000 Pervious 1,571,300 cu.yd. 0.12 188,600 Rock riprap 41,100 cu.yd. 4.00 164,400 Drilling grout holes 6,240 lin.ft. 3.00 18,700 Pressure grouting 4,160 cu. ft. 4.00 16,600 \$2,387,900 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 Floor 8,590 cu.yd. 30.00 257,700 Walls 3,010 cu.yd. 40.00 120,400 Reinforcing steel 1,199,800 lbs. 0.15 180,000 2,111,600 | From borrow pits | 1,686,600 | cu.yd. | 0.47 | 792,700 | |
| Embankment Impervious 1,466,600 cu.yd. 0.18 264,000 Pervious 1,571,300 cu.yd. 0.12 188,600 Rock riprap 41,100 cu.yd. 4.00 164,400 Drilling grout holes 6,240 lin.ft. 3.00 18,700 Pressure grouting 4,160 cu. ft. 4.00 16,600 \$2,387,900 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 Floor 8,590 cu.yd. 30.00 257,700 Walls 3,010 cu.yd. 40.00 120,400 Reinforcing steel 1,199,800 1bs. 0.15 180,000 2,111,600 | From stream bed | 838,700 | cu.yd. | 0.36 | 301,900 | |
| Impervious 1,485,600 cu.yd. 0.18 264,000 Pervious 1,571,300 cu.yd. 0.12 188,600 Rock riprap 41,100 cu.yd. 4.00 164,400 Drilling grout holes 6,240 lin.ft. 3.00 18,700 Pressure grouting 4,160 cu. ft. 4.00 16,600 \$2,387,900 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 Floor 8,590 cu.yd. 30.00 257,700 Walls 3,010 cu.yd. 40.00 120,400 Reinforcing steel 1,199,800 1bs. 0.15 180,000 2,111,600 | Embankment | - 1// /00 | | 0.70 | 0(1,000 | |
| Pervious 1,571,300 cu.yd. 0.12 166,600 Rock riprap 41,100 cu.yd. 4.00 164,400 Drilling grout holes 6,240 lin.ft. 3.00 18,700 Pressure grouting 4,160 cu. ft. 4.00 16,600 \$2,387,900 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Veir and cutoff 3,290 cu.yd. 35.00 115,200 Floor 8,590 cu.yd. 30.00 257,700 Walls 3,010 cu.yd. 40.00 120,400 Reinforcing steel 1,199,800 1bs. 0.15 180,000 2,111,600 | Impervious | 1,466,600 | cu.yd. | 0.18 | 264,000 | 1 |
| Rock riprap 41,100 cu.yd. 4.00 164,400 Drilling grout holes 6,240 lin.ft. 3.00 18,700 Pressure grouting 4,160 cu. ft. 4.00 16,600 \$2,387,900 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 Floor 8,590 cu.yd. 30.00 257,700 Walls 3,010 cu.yd. 40.00 120,400 Reinforcing steel 1,199,800 1bs. 0.15 180,000 2,111,600 | Pervious | 1,5/1,300 | cu.yd. | 0.12 | 188,600 | 1 |
| Drilling grout holes 0,240 lin.it. 3.00 10,700 Pressure grouting 4,160 cu. ft. 4.00 16,600 \$2,387,900 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 Floor 8,590 cu.yd. 30.00 257,700 Walls 3,010 cu.yd. 40.00 120,400 Reinforcing steel 1,199,800 1bs. 0.15 180,000 2,111,600 | ROCK riprap | 41,100 | cu.ya. | 4.00 | 104,400 | |
| Pressure grouting 4,100 cu. it. 4.00 10,000 \$2,307,900 Spillway Excavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 Floor 8,590 cu.yd. 30.00 257,700 Walls 3,010 cu.yd. 40.00 120,400 Reinforcing steel 1,199,800 1bs. 0.15 180,000 2,111,600 | Processing grout notes | 0,240 | lin.it. | 3.00 | 16,600 | ¢2 287 000 |
| Spillway Lxcavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 Floor 8,590 cu.yd. 30.00 257,700 Walls 3,010 cu.yd. 40.00 120,400 Reinforcing steel 1,199,800 1bs. 0.15 180,000 2,111,600 | riessure grouting | 4,100 | Cu. 16. | 4.00 | 10,000 | ₩2,007,900 |
| Excavation 1,598,100 cu.yd. 0.90 1,438,300 Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 Floor 8,590 cu.yd. 30.00 257,700 Walls 3,010 cu.yd. 40.00 120,400 Reinforcing steel 1,199,800 1bs. 0.15 180,000 2,111,600 | Spillway | | | | | |
| Concrete Weir and cutoff 3,290 cu.yd. 35.00 115,200 Floor 8,590 cu.yd. 30.00 257,700 Walls 3,010 cu.yd. 40.00 120,400 Reinforcing steel 1,199,800 1bs. 0.15 180,000 2,111,600 | Excavation | 1,598,100 | cu.yd. | 0.90 | 1,438,300 | |
| Weir and cutoff 3,290 cu.yd. 35.00 115,200 Floor 8,590 cu.yd. 30.00 257,700 Walls 3,010 cu.yd. 40.00 120,400 Reinforcing steel 1,199,800 1bs. 0.15 180,000 2,111,600 | Concrete | | U | r - | | 1 |
| Floor8,590 cu.yd.30.00257,700Walls3,010 cu.yd.40.00120,400Reinforcing steel1,199,8001bs.0.15180,0002,111,600 | Weir and cutoff | 3,290 | cu.yd. | 35.00 | 115,200 | |
| Walls 3,010 cu.yd. 40.00 120,400 Reinforcing steel 1,199,800 1bs. 0.15 180,000 2,111,600 | Floor | 8,590 | cu.yd. | 30.00 | 257,700 | 1 |
| Reinforcing steel 1,199,800 lbs. 0.15 <u>180,000</u> 2,111,600 | Walls | 3,010 | cu.yd. | 40.00 | 120,400 | |
| | Reinforcing steel | 1,199,800 | lbs. | 0.15 | 180,000 | 2,111,600 |
| Out 1 at Header | Outlat Harles | | | | | |
| Transfer works | Execution | | | | | |
| $\frac{1}{100}$ | Inlet Structure | 300 | cu vd | 5 00 | 1 500 | |
| $\begin{array}{c} \text{Conduit. Trench} \\ \text{Solution} \\ 5,250 \\ \text{cu.vd.} \\ 6,00 \\ 31 \\ 500 \\ \end{array}$ | Conduit Trench | 5,250 | cu.yd. | 6.00 | 31,500 | |
| Concrete | Concrete | 2,220 | cu.yu. | 0.00 | 000 | |
| $\frac{13,200}{13,200}$ | Inlet structure | 220 | cu.vd. | 60,00 | 13,200 | |
| Conduit encasement 1.750 cu.vd. 10.00 70.000 | Conduit encasement | 1.750 | cu.vd. | 10.00 | 70,000 | |
| Reinforcing steel 91.200 lbs. 0.15 13.700 | Reinforcing steel | 91.200 | lbs. | 0.15 | 13,700 | |
| Steel pipe 60-inch dia. 159,000 lbs. 0.28 44,500 | Steel pipe 60-inch dia. | 159.000 | lbs. | 0.28 | 44.500 | |
| Miscellaneous metal work 32,000 lbs. 0.40 12,800 | Miscellaneous metal work | 32,000 | lbs. | 0.40 | 12,800 | |
| High pressure slide gate lump sum 25,000 | High pressure slide gate | | | lump sum | 25,000 | |
| Needle valve 54-inch dia. lump sum 20,000 | Needle valve 54-inch dia. | | | lump sum | 20,000 | |
| Control house, etc. lump sum 10,000 242,200 | Control house, etc. | | | lump sum | 10,000 | 242,200 |

ESTIMATED COST OF SANTA FELICIA DAM AND RESERVOIR WITH STORAGE CAPACITY OF 50,000 ACRE-FEET (Continued)

| Item | : | Quantity | : Unit : : price : | Cost | |
|---|---------------------------------------|-------------|-----------------------|---------|-------------------------------------|
| APITAL COSTS | | | | | |
| Reservoir Land acquisit Clearing | tion | 1,030 acres | lump sum \$ 50.00 | \$ | |
| well damag | ge | | lump sum | 350,000 | \$848,500 |
| Subtotal | | | | \$5 | ,590,200 |
| Administration a Contingencies, 1 Interest during | and engineering 5% construction | , 10% | | \$ | 559,000 838,500 139,800 |
| TOTAL | | - | | \$7 | ,127,500 |
| ANNUAL COSTS | • | | | | |
| Interest, 4% Amortization, 40 Operation and ma |)-year sinking : hintenance | fund at 4% | | | \$285,100 75,000 <u>9,000</u> |
| TOTAL | | | | | \$3 6 9,100 |

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ESTIMATED COST OF SANTA FELICIA DAM AND RESERVOIR WITH STORAGE CAPACITY OF 75,000 ACRE-FEET (Based on prices prevailing in spring of 1953)

Elevation of crest of dam: 1,055 feetCapacity of reservoir to crest ofElevation of crest of spillway: 1,035 feetspillway: 75,000 acre-feetHeight of dam to spillway crest,
above stream bed: 165 feetCapacity of spillway: 75,000 acre-feet

| item : | Quantity | : | Unit : price : | Cost | 5 |
|--------------------------|------------|---------|-------------------|-----------|-----------|
| CAPITAL COSTS | | | 1.00 | | |
| Dam | | | | | |
| Exploration | | | lump sum | \$ 50,000 | |
| Diversion tunnel | | | • | | |
| 22-foot diameter | 1,080 | lin.ft. | \$ 530.00 | 572,400 | |
| Portal excavation | 18,000 | cu.yd. | 1.50 | 27,000 | |
| Concrete plug | 800 | cu.yd. | 30.00 | 24,000 | |
| Diversion of stream and | | | | | |
| dewatering of foundat | ion | | lump sum | 60,000 | |
| Stripping topsoil | 42,600 | cu.yd. | 0.40 | 17,000 | |
| Foundation excavation | | | | | |
| Abutment | 37,100 | cu.yd. | 1.40 | 51,900 | |
| Channel | 989,300 | cu.yd. | 0.55 | 544,100 | |
| Excavation for embankmen | t | | | | 1 |
| From borrow pits | 2,198,200 | cu.yd. | 0.47 | 1,033,200 | 1 |
| From streambed | 1,794,400 | cu.yd. | 0.36 | 646,000 | |
| Embankment | | | | | |
| Impervious | 1,911,500 | cu.yd. | 0.18 | 344,100 | |
| Pervious | 2,615,500 | cu.yd. | 0.12 | 313,900 | |
| Rock riprap | 59,700 | cu.yd. | 4.00 | 238,800 | |
| Drilling grout holes | 6,960 | lin.ft. | 3.00 | 20,900 | No. 0/0 0 |
| Pressure grouting | 4,640 | cu.ft. | 4.00 | | \$3,961,9 |
| Spillway | | | | | |
| Excavation | 1,034,100 | cu.yd. | 0.80 | 827,300 | |
| Concrete | | • | | | |
| Weir and cutoff | 3,290 | cu.yd. | 35.00 | 115,200 | |
| Floor | 8,230 | cu.yd. | 30.00 | 246,900 | |
| Walls | 2,820 | cu.yd. | 40.00 | 112,800 | |
| Reinforcing steel | 1,153,100 | lbs. | 0.15 | 173,000 | 1,475,20 |
| Outlat Namka | | | | | |
| Injet structure concrete | 200 | an we | 60.00 | 17 100 | |
| Infet structure concrete | ~ 390 | ou yu. | Б 00 | 2 000 | |
| Steel nine 72-inch die | 280 500 | lhs. | 0.28 | 78 500 | |
| Reinforcing steel | 27 300 | lbs | 0.20 | | |
| High pressure slide gate | 000 612 | TDO | lump sum | 25,000 | |
| Needle valve 60-inch dia | . 1 | each | 27.500.00 | 27,500 | |
| Miscellaneous metal work | 32.000 | lbs. | 0,10 | 12,800 | |
| Control house, etc. | J=,000 | | lump sum | 10,000 | 177.30 |

ESTIMATED COST OF SANTA FELICIA DAM AND RESERVOIR WITH STORAGE CAPACITY OF 75,000 ACRE-FEET (Continued)

Unit : : : Item Quantity price Cost : : : APITAL COSTS eservoir Land acquisition lump sum \$ 447,000 \$ 50.00 1,270 acres 63,500 Clearing Road relocation and oil 350,000 \$860,500 well damage lump sum \$6,474,900 Subtotal iministration and engineering, 10% \$ 647,500 971,200 ontingencies, 15% iterest during construction 323,700 \$ 8,417,300 TOTAL JNUAL COSTS iterest, 4% \$336,700 88,500 nortization, 40-year sinking fund at 4% peration and maintenance 10,000 \$435,200 TOTAL

ESTIMATED COST OF SANTA FELICIA DAM AND RESERVOIR WITH STORAGE CAPACITY OF 100,000 ACRE-FEET

(Based on prices prevailing in spring of 1953)

| Elevation of crest of dam: | 1,077 |
|------------------------------|-------|
| feet, U.S.G.S. datum | · |
| Elevation of crest of spilly | vay: |
| 1,057 feet | |
| Height of dam to spillway cr | rest, |
| above stream bed: 187 fee | et |

Capacity of reservoir to crest of spillway: 100,000 acre-feet Capacity of spillway with 5-foot freeboard: 103,000 second-feet

| | : | | : Unit : | | |
|----------------------------|-----------|------------|------------|-----------|-------------|
| Item | : Quant | tity | : price : | C | ost |
| CADTMAL COOMS | | | | | |
| CAPITAL COSTS | | | | | |
| Tam | | | | | |
| Exploration | | | מווא ממוון | \$ 50,000 | |
| Diversion tunnel | | | Lunp Dum | #)0,000 | |
| 22-foot diameter | 1.270 | lin.ft. | \$ 530.00 | 673,100 | |
| Portal excavation | 19,800 | cu.vd. | 1.50 | 29,700 | |
| Concrete plug | 890 | cu.vd. | 30.00 | 26,700 | |
| Diversion of stream and | 0,0 | ou o j u o | , | ~~,,~~ | |
| dewatering of foundation | n | | lumo sum | 70,000 | |
| Stripping topsoil | 92.300 | cu.vd. | 0.40 | 36,900 | |
| Foundation excavation | /~;;; | | | 2-37 | |
| Abutment | 57,700 | cu.vd. | 1.40 | 80,800 | |
| Channel | 1.018.600 | cu.vd. | 0.55 | 560,200 | |
| Excavation for embankment | | | | | |
| From borrow pits | 2,458,200 | cu.yd. | 0.47 | 1,155,400 | |
| From stream bed | 2,492,500 | cu.yd. | 0.36 | 897,300 | |
| Embankment | | · · | | | |
| Impervious | 2,137,600 | cu.yd. | 0.18 | 384,800 | |
| Pervious | 3,290,500 | cu.yd. | 0.12 | 394,900 | |
| Rock riprap | 75,740 | cu.yd. | 4.00 | 303,000 | |
| Drilling grout holes | 7,260 | lin.ft. | 3.00 | 21,800 | |
| Pressure grouting | 4,840 | cu.ft. | 4.00 | 19,400 | \$4,704,00(|
| 0.133. | | | | | |
| Spiliway | (00.300 | | 0.00 | 100 100 | |
| Excavation | 699,100 | cu.ya. | 0.70 | 489,400 | |
| Voir and out off | 2 200 | | 25 00 | 115 200 | |
| Floor | 3,290 | cu.yu. | 30.00 | 260 500 | |
| Walle | 0,970 | cu.yu. | 50.00 | 107 600 | |
| Reinforming stool | 1 215 700 | lbc | 40.00 | 186,000 | 1 197 600 |
| mermoreing steer | 1,24),100 | TD2* | | 100,900 | 1,010,000 |
| Outlet Works | | | | | |
| Inlet structure concrete | 290 | cu.yd. | 60.00 | 17,400 | |
| Inlet structure excavation | n 390 | cu.yd. | 5.00 | 2,000 | |
| Steel pipe 72-inch dia. | 295,800 | lbs. | 0.28 | 82,800 | |
| Reinforcing steel | 30,000 | lbs. | 0.15 | 4,500 | |
| High pressure slide gate | | | lump sum | 25,000 | |
| Needle valve 60-inch dia. | 1 | each | 27,500.00 | 27,500 | |
| Miscellaneous metal work | 32,000 | lbs. | 0.40 | 12,800 | 242 001 |
| Control house, etc. | | | lump sum | 10,000 | 182,000 |

ESTIMATED COST OF SANTA FELICIA DAM AND RESERVOIR WITH STORAGE CAPACITY OF 100,000 ACRE-FEET (Continued)

| | | · Ilmit | |
|--|-------------|-------------------------|------------------------------------|
| Item | Quantity | : price : | Cost |
| CAPITAL COSTS , | | | |
| Reservoir Land acquisition Clearing Road relocation and oil | 1,490 acres | lump sum \$ \$ 50.00 | 447,000 74,500 |
| well damage | | lump sum _ | 350,000 \$ 871,500 |
| Subtotal | | | \$6,945,100 |
| Administration and engineering Contingencies, 15% Interest during construction | , 10% | | \$ 694,500 1,041,800 347,300 |
| TOTAL | | | \$9,028,700 |
| | | | |
| ANNUAL COSTS | · | | |
| Interest, 4% Amortization, 40-year sinking : Operation and maintenance | fund at 4% | | \$ 361,100 95,000 12,500 |
| TOTAL | | | \$ 468,600 |

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(Based on prices prevailing in spring of 1953)

| T+ om i | Chient | :+ | Unit | : | Cost | |
|------------------------------|-----------|-----------|----------|----|---------|------------|
| Trêm | Quant. | i cy | price | • | 0050 | |
| CAPITAL COSTS | | | | | | |
| | | | | | | |
| Main Feeder - Casitas Reserv | oir to Fe | oster Par | rk | | | |
| Excavation and backfill | 12,900 | lin.ft. | \$ 2.80 | \$ | 36,100 | |
| Furnish and install 36- | | | | | | |
| inch diameter reinforced | | | | | | |
| concrete pipe | 12,900 | lin.ft. | 16.25 | | 209,600 | |
| Ventura River crossing | 1,100 | lin.ft. | 26.75 | | 29,400 | |
| Gate valves | - | | lump sum | | 4,000 | |
| Line meters | 2 | each | 4,000.00 | | 8,000 | |
| Blowoff valves | 2 | each | 500.00 | | 1,000 | |
| Air valves | 2 | each | 400.00 | | 800 | |
| Chlorinator structure | | | lump sum | | 10,000 | |
| Chlorinator equipment | | | lump sum | | 10,500 | |
| Road surfacing | 12,900 | lin.ft. | 1.00 | | 12,900 | |
| Concrete in structures | 220 | cu.yd. | 65.00 | | 14,300 | |
| Miscellaneous metal | 2,500 | lbs. | 0.65 | | 1,600 | |
| Fire hydrants | 4 | each | 150.00 | | 600 | |
| Right of way | | | lump sum | | 5,000 | \$ 343,800 |
| | | | - | | | |
| Eastside Conduit - Foster Pa | rk to La | crosse | | | | |
| Excavation and backfill | 8,700 | lin.ft. | 2.15 | | 18,700 | |
| Furnish and install 27- | | | | | | |
| inch diameter concrete | | | | | | |
| cylinder pipe | 8,700 | lin.ft. | 11.85 | | 103,100 | |
| Creek crossing | 300 | lin.ft. | 20.75 | | 6,200 | |
| Gate valve - 18-inch dia. | l | each | 1,000.00 | | 1,000 | |
| Blowoff valves | | | lump sum | | 400 | |
| Air valves | 2 | each | 300.00 | | 600 | |
| Service outlets | 5 | each | 150.00 | | 800 | |
| Concrete structures | 50 | cu.yd. | 65.00 | | 3,300 | |
| Miscellaneous metal | 1,000 | lbs. | 0.65 | | 700 | |
| Road surfacing | ,700 8 | lin.ft. | 0.85 | | 7,400 | 1 |
| Fire hydrants | 2 | each | 150.00 | | 300 | |
| Right of way | | | lump sum | | 1,000 | 143,500 |
| | | | | | | |
| Eastside Conduit - Lacrosse | to Baldw: | in Road | | | | |
| Excavation and backfill | 18,000 | lin.ft. | 1.86 | | 33,500 | |
| Furnish and install 24- | | | | | | ! |
| inch diameter concrete | | | | | | 1 |
| cylinder pipe | 18,000 | lin.ft. | 10.25 | | 184,500 | i |
| Gate valves - 18-inch dia. | 3 | each | 1,000.00 | | 3,000 | |
| Line meter | | | lump sum | | 2,900 | 1 |
| Blowoff valves | 2 | each | 400.00 | | 800 | ! |
| Air valves | 2 | each | 300.00 | | 600 | (|
| Service outlets | 5 | each | 150.00 | | 800 | |
| Concrete structures | 80 | cu.yd. | 65.00 | | 5,200 | |
| Miscellaneous metal | 2,000 | lbs. | 0.65 | | 1,300 | 1 |

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| : | | : Unit | : | |
|-----------------------------|----------------|----------------------|-----------|------------|
| Item : | Quantity | : price | : Cost | t |
| | | | | |
| PITAL COSTS | | | | |
| | t. Delluin De | a (Cantinga) | ١ | |
| Reside Conduit - Lacrosse | to Balawin no | ad (Continued | / & 6.000 | |
| Fine budgents | 7,500 III. | $10. \varphi = 0.00$ | ψ 0,000 | |
| Reservoir | , U ca | | 35,000 | |
| Pumping plant | | lump sum | 32,200 | |
| Right of way | | lumo sum | 15,000 | \$ 322.000 |
| TELEVICE NOW | | | | " <u>J</u> |
| hstside Conduit - Baldwin H | load to Fairvi | ew Wye | | |
| Excavation and backfill | 11,400 lin. | ft. 1.00 | 11,400 | |
| Furnish and install 16- | | | | |
| inch diameter concrete | | a. (a | | |
| cylinder pipe | 11,400 lin. | It. 6.25 | 71,200 | |
| Gate valves - 12-inch dia. | 2 ea | ch 450.00 | 900 | |
| BLOWOII valves | j ea | cn = 200.00 | 600 | |
| Air valves | z ea ۲ aa | ch 300.00 | 800 | |
| Concercie structures | 80 on m | | 5 200 | |
| Miccellanoous metal | | | 600 | |
| Road surfacing | 9,000 1in | $f_{t} = 0.60$ | 5.1.00 | |
| Fire hydrants | 7 63 | h 150.00 | 1,000 | |
| Pumping plant | 1 04 | lumo sum | 27,000 | |
| Right of way | | lump sum | 4.000 | 128,700 |
| | | | | |
| reek Road Conduit - Lacross | se to Terminal | Reservoir | | |
| Excavation and backfill | 42,800 lin. | ft. 1.00 | 42,800 | |
| Furnish and install 16- | | | | |
| inch diameter concrete | | | | |
| cylinder pipe | 42,800 lin. | ft. 6.25 | 267,500 | |
| Gate valves - 12-inch dia. | , ð ea | ch 450.00 | 3,600 | |
| Line meter | 1 | Lump sum | 2,000 | |
| Ain valves | 4 ea | cn = 200.00 | 2 1.00 | |
| Service outlets | ılı ca | ch 150.00 | 2,400 | |
| Concrete structures | 150 on w | d. 65.00 | 9 800 | |
| Miscellaneous metal | 3,300 Th | s. 0.65 | 2,100 | |
| Road surfacing | 24.400 lin. | ft. 0.60 | 1/1.600 | |
| Fire hydrants | 10 ea | ch 150.00 | 1.500 | |
| Highway crossing | | lump sum | 2,500 | |
| Reservoir | | lump sum | 35,000 | |
| Pumping plant | | lump sum | 34,200 | |
| Right of way | | lump sum | 7,000 | 427,900 |
| | | | | |
| per Ojai Conduit | 10.000.11 | 01 0.05 | 0.000 | |
| Excavation and backfill | 10,200 lin. | IT. 0.80 | 8,200 | |
| inch diamotor concrete | | | | |
| cylinder nipe | 10 200 11- | ft), 85 | LO KOO | |
| Carrier hthe | TTIL. | 100 4002 | 47,000 | |

| : | | : | Unit | : | | |
|-------------------------------|--------|---------------|-----------|----|----------|----------------------|
| item : | Quant: | ity : | price | : | Cost | |
| CAPITAL COSTS | | | | | | |
| Honor Oisi Conduit (Continued |) | | | | | |
| Gate valves = 10-inch dia | 2 | each | \$ 350.00 | dk | 700 | |
| Line meter | 2 | each | | Ψr | 1 800 | |
| Blowoff valves | 2 | each | 200.00 | | 1,000 | |
| Air valves | 2 | each | 300.00 | | 600 | |
| Service outlets | 5 | each | 150.00 | | 800 | |
| Concrete structures | 75 | cu.vd. | 65.00 | | 4.900 | i |
| Miscellaneous metal | 1.000 | lbs. | 0.65 | | 600 | |
| Road surfacing | 4,200 | lin.ft. | 0.55 | | 2,300 | |
| Fire hydrants | 3 | each | 150.00 | | 400 | |
| Pumping plant | | | lump sum | | 15,700 | |
| Right of way | | | lump sum | | 2,500 \$ | 88,400 |
| | | | | - | 1 | |
| Cross Tie to Grand Avenue | | | | | | |
| Excavation and backfill | 4,500 | lin.ft. | 0.80 | | 3,600 | |
| Furnish and install 12- | | | | | | |
| inch diameter concrete | 1 500 | | 1.05 | | 0.00 | |
| cylinder pipe | 4,500 | lin.ft. | 4.85 | | 21,800 | |
| Gate valves - 10-inch dia. | 2 | each | 350.00 | | 700 | |
| Line meter | | | Lump sum | | 1,800 | |
| BLOWOII Valves | 0 | an ah | Lump sum | | 200 | |
| Air valves | 2 | each | 300.00 | | 1.00 | |
| Concrete structures | 20 | each cu vd | 65 00 | | 1 300 | |
| Concrete pipe anchors | 20 | cu.yd. | 30.00 | | 600 | |
| Miscellaneous metal | 1.000 | lhs. | 0.65 | | 600 | |
| Road surfacing | 4.500 | lin.ft. | 0.55 | | 2,500 | |
| Fire hydrants | 3 | each | 150.00 | | 400 | |
| Right of way | - | 00.011 | lump sum | | 500 | 35.000 |
| | | | 1 | - | | <i>JJJJJJJJJJJJJ</i> |
| Baldwin Road - Santa Ana Cond | uit | | | | | |
| Excavation and backfill | 11,800 | lin.ft. | 1.00 | | 11,800 | |
| Furnish and install 14- | | | | | | |
| inch diameter concrete | | | | | | |
| cylinder pipe | 11,800 | lin.ft. | 5.45 | | 64,300 | _ |
| Ventura River crossing | 1,600 | lin.ft. | 10.70 | | 17,100 | |
| Gate valves - 12-inch dia. | 3 | each | 450.00 | | 1,400 | |
| Line meter | - | | Lump sum | | 1,800 | |
| BLOWOIT VALVES | 2 | each | 200.00 | | 400 | |
| Air Valves | و | each | 300.00 | | 900 | |
| Service connections | 4 | each | 150.00 | | 2 000 | 1 |
| Missellaneous motal | 1 500 | lho | 05.00 | | 3,900 | 2 |
| Road surfacing | 8,800 | lin ft | 0.69 | | 5 300 | 1 |
| Fire Hydrants | 2,000 | each | 150.00 | | 001 | |
| |) | Guon | 100000 | | 400 | |

| | : | Unit : | | |
|-----------------------------|-------------------|----------|----------|---|
| Item : | Quantity : | price : | Cost | |
| | Y | | | |
| PITAL COSTS | · · · · | * | | |
| | | | | |
| Fldwin Road - Santa Ana Con | nduit (Continued) | | | |
| Highway crossing | | lump sum | \$ 2,500 | |
| Reservoir | | lump sum | 35,000 | |
| Right of way | | lump sum | 5.000 | \$ 151.400 |
| | | | | >->-> |
| nor Canvon Extension | | | | |
| Excavation and backfill | 9.000 lin.ft. | \$ 0.80 | 7,200 | |
| Furnish and install 10- | | | | |
| inch diameter welded | , | | | |
| steel pipe | 9.000 lin.ft. | 2.65 | 23,900 | |
| Gate valves - 6-inch dia. | 2 each | 300.00 | 600 | |
| Line meter | | lump sum | 1.000 | |
| Blowoff valves | 2 each | 200.00 | 100 | |
| Air valves | 2 each | 300.00 | 600 | |
| Service outlets | 3 each | 150.00 | 100 | |
| Concrete structures | 40 cu.vd. | 65.00 | 2,600 | |
| Miscellaneous metal | 1.000 lbs. | 0.65 | 600 | |
| Road surfacing | 2.000 lin.ft. | 0.60 | 1.200 | |
| Fire hydrants | 3 each | 150.00 | 100 | |
| Pumping plant | , | lumo sum | 13,700 | |
| Right of way | | lump sum | 2,500 | 55,100 |
| | | | | ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, |
| Inada Larga Conduit | | | | |
| Excavation and backfill | 29.200 lin.ft. | 0.80 | 23.400 | |
| Furnish and install 6- | | | | |
| inch diameter welded | | | | |
| steel pipe | ·29,200 lin.ft. | 1.80 | 52,600 | |
| Gate valves - 4-inch dia. | 4 each | 170.00 | 700 | |
| Line meter | | lump sum | 100 | |
| Blowoff valves | 3 each | 100.00 | 300 | |
| Air valves | L each | 200.00 | 800 | |
| Service outlets | 5 each | 150.00 | 800 | |
| Concrete structures | 50 cu.vd. | 65.00 | 3,300 | |
| Miscellaneous metal | 1.000 lbs. | 0.65 | 600 | |
| Road surfacing | 10.000 lin.ft. | 0.55 | 5,500 | |
| Fire hydrants | 6 each | 150.00 | 900 | |
| Highway crossing | | lump sum | 1,000 | |
| Pumping plant | | lump sum | 8,500 | |
| Right of way | | lump sum | 2,500 | 101,300 |
| | | | | |
| ncon Conduit | | | | |
| Excavation and backfill | 99,000 lin.ft. | 0.75 | 74,200 | |
| Furnish and install | | | | |
| welded steel pipe | | | | |
| 10-inch diameter | 79,000 lin.ft. | 2.75 | 217,300 | |
| 8-inch diameter | 20,000 lin.ft. | 2.25 | 45,000 | |
| Gate valves - 6-inch dia. | 6 each | 300.00 | 1,800 | |

| Item Quantity price Cost CAPTTAL COSTS Rincon Conduit (Continued) Iump sum \$ 1,000 Line meter lump sum \$ 1,000 2,000 Air valves 10 each \$ 200.00 2,000 Service outlets 10 each \$ 150.00 1,500 Service outlets 10 each \$ 150.00 1,500 Road surfacing 20,000 lin.ft. 0.65 3,200 Reservoir 10 each \$ 150.00 1,500 Reservoir 10 each \$ 150.00 1,500 Pumping plant 1ump sum 5,000 \$ 438,600 Conduit from Matilija Line to Proposed Reservoir 5,000 \$ 438,600 Furnish and install 14- 1.00 \$ 5,000 \$ 438,600 Furnish and install 14- 1.00 \$ 5,000 \$ 438,600 Blowoff valves 2 each \$ 150.00 900 \$ 438,600 Conduit from Matilija Line to Proposed Reservoir 1.00 \$ 5,000 \$ 1,000 Furnish and install 14- 1.00 \$ 5,000 \$ 100 \$ 100 \$ 100 \$ 100 | | | | These | | | | |
|---|----------------------------|-------------|----------|-----------|----|--------|----------------|-----------|
| CAPITAL COSTS Rincon Conduit (Continued) Line meter lump sum \$ 1,000 Blowoff valves 10 each 300.00 3,000 Service outlets 10 each 150.00 1,500 Concrete structures 60 cu.yd. 65.00 5,200 Miscellaneous metal 5,000 lins.ft. 0.665 3,200 Road surfacing 20,000 lin.ft. 0.666 1,500 Pire hydrants 10 each 150.00 1,500 Reservoir lump sum 35,000 Pumping plant lump sum 35,000 Right of way lump sum 5,000 Furnish and install ll- inch diameter concrete cylinder pipe cylinder pipe 5,000 lin.ft. 5.15 27,200 Gate valves - 12-inch dia. 2 each 200.00 100 Air valves 3 each 300.00 900 Service outlets 3 each 300.00 900 Rice laneous metal 1,000 lbs. 0.65 600 Miscellaneous metal 1,000 lbs. 0.65 600 | Item | : Quant: | ity : | price | : | Cos | t | |
| Rincon Conduit (Continued) Line meter lump sum \$ 1,000 Blowoff valves 10 each \$ 200.00 2,000 Air valves 10 each \$ 200.00 2,000 Air valves 10 each \$ 200.00 2,000 Air valves 10 each \$ 200.00 3,000 Service outlets 10 each \$ 200.00 3,000 Miscellaneous metal 5,000 lbs. 0.65 3,200 Road surfacing 20,000 lin.ft. 0.66 12,000 Fire hydrants 10 each 150.00 1,500 Reservoir lump sum 35,000 1,500 Pumping plant lump sum 5,000 \$ 438,600 Conduit from Matilija Line to Proposed Reservoir 5,000 \$ 5,000 Excevation and backfill 5,000 lin.ft. 5,15 27,200 Gate valves - 12-inch dia. 2 each 200.00 400 Air valves 3 each 300.00 900 Service outlets 3 each 300.00 900 Goncrete structures 40 cu.yd. 65.00 2,600 | CADIMAI COOME | | | | | | | - |
| Hincon Conduit (Continued) Lump sum \$ 1,000 Blowoff valves 10 each 300.00 2,000 Air valves 10 each 150.00 1,500 Concrete structures 80 cu.yd. 65.00 5,200 Miscellaneous metal 5,000 1500 Concrete structures 80 cu.yd. 65:00 12,000 Fire hydrants 10 each 150.00 1,500 Reservoir 10 mp sum 35,000 1,500 Pumping plant lump sum 35,000 Furnish and install 14- inch diameter concrete cylinder pipe 5,000 lin.ft. 5,15 Conduit from Matilija Line to Proposed Reservoir 5,000 \$ 438,600 Conduit from Matilija Line to Proposed Reservoir 5,000 \$ 6,000 Furnish and install 14- 1.00 \$ 5,000 \$ 6,000 Inch diameter concrete cylinder pipe \$ 5,000 lin.ft. \$ 1,000 Gate valves - 12-inch dia. 2 each 150.00 \$ 900 Blowoff valves 3 each 150.00 \$ 900 Service outlets 3 each 150.00 \$ 900 Goncrete structures \$ 10 | CAPITAL COSTS | | | | | | | |
| Line meter lump sum \$ 1,000 Blowoff valves 10 each \$ 200.00 2,000 Air valves 10 each \$ 200.00 3,000 Service outlets 10 each \$ 200.00 1,500 Concrete structures 80 cu.yd. 65.00 5,200 Miscellaneous metal 5,000 1bs. 0.65 3,200 Reservoir 10 each \$ 150.00 1,500 1500 Reservoir 10 each \$ 150.00 1,500 Pumping plant 10 ump sum 35,000 Furnish and install \$ 5,000 lin.ft. 1.00 5,000 Furnish and install \$ 5,000 lin.ft. 5,15 27,200 Gate valves - 12-inch dia. 2 each \$ 200.00 400 Air valves 3 each \$ 300.00 900 Blowoff valves 2 each \$ 200.00 400 Air valves 3 each \$ 300.00 900 Service outlets 3 each \$ 300.00 900 Blowoff valves 4 each \$ 150.00 400 Concrete structures 40 cu.yd. 65.00 2,400 Niscellaneous metal 1,000 1bs. 600 600 </td <td>Rincon Conduit (Continued)</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> | Rincon Conduit (Continued) | | | | | | | |
| Air valves 10 each \$ 200.00 2,000 Air valves 10 each \$ 150.00 1,500 Service outlets 10 each \$ 150.00 1,500 Miscellaneous metal 5,000 lbs. 0.65 3,200 Niscellaneous metal 5,000 lbs. 0.65 3,200 Reservoir 10 each \$ 150.00 1,500 Pumping plant 10 bump sum \$ 35,000 1,500 Reservoir 10 bump sum \$ 35,000 1,500 Pumping plant 10 lump sum \$ 35,000 \$,000 Right of way 10 pump sum \$ 5,000 \$,000 Furnish and install ll; 5,000 lin.ft. 5,15 27,200 Gate valves - 12-inch dia. 2 each \$ 200.00 100 Air valves 3 each \$ 150.00 100 Service outlets 3 each \$ 150.00 100 Air valves 3 each \$ 150.00 600 Riscellaneous metal 1,000 10s. 0.65 600 Niscellaneous metal 1,000 10s. 6.65 | Line meter | | | lump sum | \$ | 1,000 | | |
| Air values10each150.005,000Service outlets10each150.001,500Concrete structures80cu.yd.65.005,200Miscellaneous metal5,000lbs.0.653,200Read surfacing20,000lin.ft.0.6012,000Fire hydrants10each150.001,500Reservoirlump sum35,000Pumping plantlump sum35,000Right of waylump sum5,000Conduit from Matilija Line to Proposed Reservoir5,000Excavation and backfill5,000 lin.ft.1.00Furnish and install l4-inch diameter concretecylinder pipe5,000 lin.ft.5.45Concrete structures2eachd10cu.yd.65.002,600Air valves3eachService outlets3eachlawes1,000lbs.0.65Good surfacing4,000lin.ft.0.60Node surfacing4,000lin.ft.0.60Reservoirlump sun35,000Subtotal\$2,316,700Administration and engineering, 10%\$2,31,700Contingencies, 15%\$2,953,800Interest during construction57,900TOTAL\$2,953,800 | Blowolf valves | 10 | each | \$ 200.00 | | 2,000 | | |
| Concrete structures 80 cu,yd. 65:00 5,200 Miscellaneous metal 5,000 lbs. 0.65 3,200 Road surfacing 20,000 lin.ft. 0.60 12,000 Fire hydrants 10 each 150:00 1,500 Reservoir lump sum 35,000 Pumping plant lump sum 35,000 Right of way lump sum 5,000 Conduit from Matilija Line to Proposed Reservoir 5,000 Excavation and backfill 5,000 lin.ft. 1.00 Furnish and install ll- inch diameter concrete cylinder pipe 5,000 lin.ft. cylinder pipe 5,000 lin.ft. 5.15 27,200 Gate valves - 12-inch dia. 2 each 150:00 900 Blowoff valves 2 each 200:00 400 Air valves 3 each 300:00 900 Service outlets 3 each 150:00 400 Koad surfacing 4,000 lin.ft. 66:00 Road surfacing 1,000 lbs. 0.65 600 Road surfacing 4,000 lin.ft. 0.60 2,100 Reservoir lump sum 5,000 | Service outlets | 10 | each | 150.00 | | 3,000 | | |
| Wiscellaneous metal 5,000 lbs. 0.65 3,200 Road surfacing 20,000 lin.ft. 0.60 12,000 Fire hydrants 10 each 150.00 1,500 Reservoir lump sum 35,000 Pumping plant lump sum 30,900 Right of way lump sum 5,000 \$ 438,600 Conduit from Matilija Line to Proposed Reservoir 5,000 \$ 438,600 Excavation and backfill 5,000 lin.ft. 1.00 5,000 Furnish and install ll- inch diameter concrete 2 each 450.00 900 cylinder pipe 5,000 lin.ft. 5.45 27,200 6de valves - 12-inch dia. 2 each 450.00 900 Blowoff valves 3 each 150.00 400 400 400 Air valves 3 each 150.00 400 400 Service outlets 3 each 150.00 2,600 Miscellaneous metal 1,000 1bs. 0.65 600 Road surfacing 4,000 1bs. 0.60 600 Right of way | Concrete structures | 80 | cu.vd. | 65.00 | | 5,200 | | |
| Road surfacing 20,000 lin.ft. 0.60 12,000 Fire hydrants 10 each 150.00 1,500 Reservoir lump sum 30,900 Fight of way lump sum 30,900 Right of way lump sum 5,000 Conduit from Matilija Line to Proposed Reservoir 5,000 \$438,600 Conduit from Matilija Line to Proposed Reservoir 5,000 \$5,000 Excavation and backfill 5,000 lin.ft. 1.00 5,000 Furnish and install l4- inch diameter concrete cylinder pipe 5,000 lin.ft. 5.15 27,200 Gate valves - 12-inch dia. 2 each 200.00 \$000 \$000 Blowoff valves 3 each 300.00 900 Service outlets 3 each 300.00 \$000 Concrete structures \$10 cu.yd. 65.00 2,600 \$000 Miscellaneous metal 1,000 1bs. 0.65 600 Reservoir \$1000 \$1000 \$1000 \$1000 \$1000 Subtotal \$2,316,700 \$1,7500 \$1,7500 \$1,7500 Interest | Miscellaneous metal | 5,000 | lbs. | 0.65 | | 3,200 | | |
| Fire hydrants10each150.001,500Reservoirlump sum35,000Pumping plantlump sum30,900Right of waylump sum5,000Conduit from Matilija Line to Proposed ReservoirExcavation and backfill5,000 lin.ft.Furnish and install l4-inch diameter concretecylinder pipe5,000 lin.ft.cylinder pipe5,000 lin.ft.Service outlets2each150.00Air valves3each300.00Service outlets3each150.00Miscellaneous metal1,000 lbs.0.65600Reservoirlump sumSubtotal\$2,316,700Administration and engineering, 10%\$2,316,700TOTAL\$2,953,800 | Road surfacing | 20,000 | lin.ft. | 0.60 | | 12,000 | | |
| Reservoirlump sum35,000Pumping plantlump sum30,900Right of waylump sum5,000Conduit from Matilija Line to Proposed ReservoirExcavation and backfill5,000 lin.ft.Furnish and install lip-inch diameter concretecylinder pipe5,000 lin.ft.cylinder pipe5,000 lin.ft.Service outlets3 each1000150.00Air valves3 each2 each150.00Service outlets3 each3 each150.00Miscellaneous metal1,0001,000lbs.0.65600Reservoirlump sumSubtotal\$2,316,700Administration and engineering, 10%\$2,953,800TOTAL\$2,953,800ANNUAL COSTSAnnual Annual | Fire hydrants | 10 | each | 150.00 | | 1,500 | | |
| Right of waylump sum30,900Right of waylump sum5,000Conduit from Matilija Line to Proposed ReservoirExcavation and backfill5,000 lin.ft.Furnish and install l4-inch diameter concretecylinder pipe5,000 lin.ft.Subtore outlets2 eachBlowoff valves2 each2 each200.00Air valves3 each3 each150.00Service outlets3 each1,000lbs.0.655600Miscellaneous metal1,0001,000lbs.0.655600Reservoirlump sumSubtotal\$2,316,700Administration and engineering, 10%\$2,31,700Contingencies, 15%15%Interest during construction57.900ADMINUAL COSTS4 | Reservoir Durning alert | | | Lump sum | | 35,000 | | |
| Conduit from Matilija Line to Proposed Reservoir Excavation and backfill 5,000 lin.ft. 1.00 5,000Furnish and install l4- inch diameter concrete cylinder pipe 5,000 lin.ft. 5.45 27,200 | Bight of way | | | Lump sum | | 30,900 | \$ | 1.38 600 |
| Conduit from Matilija Line to Proposed ReservoirExcavation and backfill5,000 lin.ft.1.005,000Furnish and install l4- inch diameter concrete cylinder pipe5,000 lin.ft.5.4527,200Gate valves - 12-inch dia.2 each 450.00900Blowoff valves2 each 200.00400Air valves3 each 300.00900Service outlets3 each 150.00400Concrete structures40 cu.yd.65.002,600Miscellaneous metal1,000lbs.0.65600Road surfacing4,000lin.ft.0.602,400Fire hydrants4 each 150.0060081,000Right of waylump sum5,00081,000Subtotal\$231,700347,500TOTAL\$2,953,800ANNUAL COSTSANNUAL COSTS | terbito or way | | | rung sun | | | 44 | 4,000,000 |
| Excavation and backfill5,000 lin.ft.1.005,000Furnish and install l4- inch diameter concreteinch diameter concrete5,000 lin.ft.5.4527,200Gate valves - 12-inch dia.2 each 450.00900900Blowoff valves2 each 200.00400Air valves3 each 300.00900Service outlets3 each 150.00400Concrete structures40 cu.yd.65.002,600Miscellaneous metal1,000lbs.0.65600Road surfacing4,000lin.ft.0.602,400Fire hydrants4 each 150.0060081,000Reservoirlump sum35,00081,000Subtotal\$2,316,700347,500Administration and engineering, 10%\$2,953,800TOTAL\$2,953,800 | Conduit from Matilija Line | to Propose | ed Reser | voir | | | | |
| Furnish and install 14- inch diameter concrete cylinder pipe5,000 lin.ft.5.4527,200Gate valves - 12-inch dia.2each450.00900Blowoff valves2each200.00400Air valves3each300.00900Service outlets3each150.00400Concrete structures40cu.yd.65.002,600Miscellaneous metal1,000lbs.0.65600Road surfacing4,000lin.ft.0.602,400Fire hydrants4each150.00600Reservoirlump sum35,00081,000Subtotal\$2,316,700Administration and engineering, 10%\$2,31,700Contingencies, 15% | Excavation and backfill | 5,000 | lin.ft. | 1.00 | | 5,000 | | |
| Include the concrete 5,000 lin.ft. 5.45 27,200 Gate valves - 12-inch dia. 2 each 450.00 900 Blowoff valves 2 each 200.00 400 Air valves 3 each 300.00 900 Service outlets 3 each 150.00 400 Concrete structures 40 cu.yd. 65.00 2,600 Miscellaneous metal 1,000 lbs. 0.65 600 Road surfacing 4,000 lin.ft. 0.60 2,400 Fire hydrants 4 each 150.00 600 600 Reservoir lump sum 35,000 81,000 Subtotal \$2,316,700 \$2,316,700 Administration and engineering, 10% \$2,953,800 \$2,953,800 TOTAL \$2,953,800 \$2,953,800 | Furnish and install 14- | | | | | | | |
| Gate valves - 12-inch dia. 2 each 150.00 900 Blowoff valves 2 each 200.00 400 Air valves 3 each 300.00 900 Service outlets 3 each 150.00 400 Concrete structures 40 cu.yd. 65.00 2,600 Miscellaneous metal 1,000 lbs. 0.65 600 Road surfacing 4,000 lin.ft. 0.60 2,400 Fire hydrants 4 each 150.00 600 Reservoir lump sum 35,000 81,000 Subtotal \$2,316,700 Administration and engineering, 10% \$2,953,800 TOTAL \$2,953,800 | cylinder pipe | 5,000 | lin.ft. | 5.15 | | 27,200 | | |
| Blowoff valves 2 each 200.00 400 Air valves 3 each 300.00 900 Service outlets 3 each 150.00 400 Concrete structures 40 cu.yd. 65.00 2,600 Miscellaneous metal 1,000 1bs. 0.65 600 Road surfacing 4,000 1in.ft. 0.60 2,400 Fire hydrants 4 each 150.00 600 Reservoir 1ump sum 35,000 81,000 Subtotal \$2,316,700 \$2,316,700 Administration and engineering, 10% \$2,31,700 347,500 Contingencies, 15% 347,500 347,500 Interest during construction 57,900 57,900 TOTAL \$2,953,800 | Gate valves - 12-inch di | a. 2 | each | 450.00 | | 900 | | |
| Air valves 3 each 300.00 900 Service outlets 3 each 150.00 400 Concrete structures 40 cu.yd. 65.00 2,600 Miscellaneous metal 1,000 1bs. 0.65 600 Road surfacing 4,000 lin.ft. 0.60 2,400 Fire hydrants 4 each 150.00 600 Reservoir 1ump sum 35,000 81,000 Right of way 1ump sum 5,000 81,000 Subtotal \$2,316,700 \$2,316,700 Administration and engineering, 10% \$2,31,700 347,500 Contingencies, 15% 57,900 347,500 Interest during construction 57,900 347,500 TOTAL \$2,953,800 \$2,953,800 | Blowoff valves | 2 | each | 200.00 | | 400 | | |
| Service outlets 3 each 150.00 400 Concrete structures 40 cu.yd. 65.00 2,600 Miscellaneous metal 1,000 lbs. 0.65 600 Road surfacing 4,000 lin.ft. 0.60 2,400 Fire hydrants 4 each 150.00 600 Reservoir 1ump sun 35,000 81,000 Right of way 1ump sun 5,000 81,000 Subtotal \$2,316,700 Administration and engineering, 10% \$231,700 Contingencies, 15% 347,500 Interest during construction 57,900 ANNUAL COSTS \$2,953,800 | Air valves | 3 | each | 300.00 | | 900 | | |
| Concrete structures40 cu.yd.65.002,800Miscellaneous metal1,000lbs.0.65600Road surfacing4,000lin.ft.0.602,400Fire hydrants4each150.00600Reservoir1ump sun35,00081,000Right of way1ump sun5,00081,000Subtotal\$2,316,700Administration and engineering, 10%\$2,316,700Contingencies, 15%347,500Interest during construction57,900TOTAL\$2,953,800 | Service outlets | 3 | each | 150.00 | | 400 | | |
| Miscerianeous metal 1,000 lbs. 0.05 000 Road surfacing 4,000 lin.ft. 0.60 2,400 Fire hydrants 4 each 150.00 600 Reservoir lump sun 35,000 81,000 Right of way lump sun 5,000 81,000 Subtotal \$2,316,700 Administration and engineering, 10% \$231,700 Contingencies, 15% 347,500 Interest during construction 57,900 TOTAL \$2,953,800 | Concrete structures | 1 000 | cu.yd. | 05.00 | | 2,600 | | |
| Note burtering4,000 finition150.00150,00Fire hydrants4 each150.00600Reservoirlump sum35,00081,000Right of waylump sum5,00081,000Subtotal\$2,316,700Administration and engineering, 10%\$2,316,700Contingencies, 15%347,500Interest during construction57,900TOTAL\$2,953,800 | Road surfacing | 1,000 | lin.ft. | 0.60 | | 2.1.00 | | |
| Reservoir Right of waylump sum35,000 5,00081,000Subtotal\$2,316,700Administration and engineering, 10% Contingencies, 15%\$231,700 347,500 57,900TOTAL\$2,953,800ANNUAL COSTS\$2,953,800 | Fire hydrants | 4,000 | each | 150.00 | | 600 | | |
| Right of waylump sum5,00081,000Subtotal\$2,316,700Administration and engineering, 10%\$231,700Contingencies, 15%\$231,700Interest during construction\$7,900TOTAL\$2,953,800ANNUAL COSTS | Reservoir | | | lump sum | | 35,000 | | |
| Subtotal\$2,316,700Administration and engineering, 10%\$ 231,700Contingencies, 15%\$ 231,700Interest during construction\$ 27,900TOTAL\$ 2,953,800ANNUAL COSTS\$ 2,953,800 | Right of way | | | lump sum | | 5,000 | _ | 81,000 |
| Administration and engineering, 10% Contingencies, 15% Interest during construction TOTAL ANNUAL COSTS | Cont. 4 - 4 - 7 | | | | | | de o | 27 6 700 |
| Administration and engineering, 10% Contingencies, 15% Interest during construction TOTAL ANNUAL COSTS Administration and engineering, 10% 347,500 57,900 \$2,953,800 | Subtotal | | | | | | Φ2 | ,310,000 |
| Contingencies, 15% 347,500 Interest during construction 57,900 TOTAL \$2,953,800 ANNUAL COSTS | Administration and enginee | ring, 10% | | | | | \$ | 231.700 |
| Interest during construction 57,900 TOTAL \$2,953,800 ANNUAL COSTS | Contingencies, 15% | | | | | | " | 347,500 |
| TOTAL \$2,953,800 | Interest during constructi | on | | | | | | 57,900 |
| ANNUAL COSTS | TOT A T | | | | | | ^o o | 052 800 |
| ANNUAL COSTS | TOTAL | | | | | | \$2 | ,953,000 |
| ANNUAL COSTS | | | | | | | | |
| | ANNUAL COSTS | | | | | | | |
| Internet 19 | Interest 19 | | | | | | 25 | 220.000 |
| $\frac{1100,200}{400}$ | Amontization bo-year sink | ing fund of | - 1.7 | | | | ê₽ | 110,200 |
| Replacement, 30-year sinking fund at 3.5% 9.700 | Replacement, 30-year sinki | ng fund at | 3.5% | | | | | 9,700 |
| Operation and maintenance 14,500 | Operation and maintenance | 5 | | | | | | 14,500 |
| Electrical energy 78,400 | Electrical energy | | | | | | | 78,400 |
| | mom a t | | | | | | 24 | |
| \$ 252,000 | TUTAL | | | | | | \$ | 252,000 |

ESTIMATED COST CF CASITAS - OXNARD PLAIN DIVERSION

(Based on prices prevailing in spring of 1953)

Capacity of conduit: 25 second-feet Length of conduit: 96,300 lineal feet

| | : | : Unit | : |
|--|----------------------------------|--------------------------------|---------------------------------|
| Item | : Quantity | : price | : Cost |
| CAPITAL COSTS | | | |
| Excavation | 115,000 cu.yd | \$ 1.29 | \$ 148,400 |
| Backfill | 94,800 cu.yd. | 0.53 | 50,200 |
| Pipe, reinforced concrete cylinder, furnish and install. | | | |
| 30-inch diameter 27-inch diameter | 49,000 lin.ft. 47,300 lin.ft. | 10.10 8.93 | 494,900 422,400 |
| Fittings | | lump sum | 41,800 |
| Valves Air release, 3-inch dia. Blowoff, 5-inch dia. Gate, 24-inch dia. | 6 each 6 each 3 each | 300.00 1,000.00 1,300.00 | 1,800 6,000 3,900 |
| Venturi meter | l each | 5,000.00 | 5,000 |
| River crossings | | lump sum | 85,000 |
| Road resurfacing and crossings | | lump sum | 21,000 |
| Right of way | | lump sum | 29,800 |
| Subtotal | | | \$1,310,200 |
| Administration and engineeri Contingencies, 15% Interest during construction | ng, 10% | | \$ 131,000 196,500 32,800 |
| TOTAL | | | \$1,670,500 |
| | - | | - |
| ANNUAL COSTS | | | |
| | | | |

| Interest, 4% Amortization, 20-ye Operation and Maint | ear sinking fund at 4% cenance | \$66,800 56,100 4,200 |
|--|-----------------------------------|-----------------------------|
| TOTAL | | \$127,100 |

ESTIMATED COST OF SANTA CLARA RIVER CONDUIT DEVIL CANYON DAM TO SESPE CREEK

(Based on prices prevailing in spring of 1953)

Capacity of conduit: 65 second-feet Length of conduit: 90,200 lineal feet

| | | | | Unit | • | |
|--|-----|----------------------------|-------------|--------------------------------|---|-----------------------------------|
| Item | • | Quantity | : | price | : | Cost |
| CAPITAL COSTS | 1 | | | | | |
| Excavation | | 142,000 cu.y | rd. | \$ 0.90 | | \$ 127,800 |
| Backfill | | 114,800 cu.y | rd. | 0.45 | | 51,700 |
| Pipe, reinforced concrete, furnish and install 42-inch diameter 36-inch diameter | | 26,600 lin. 63,600 lin. | ft. ft. | 16.19 12.13 | | 430,700 771,500 |
| Fittings | | | | lump sum | | 55,200 |
| Valves Air release, 4-inch diameter Blowoff, 6-inch diameter Gate, 36-inch diameter | | 7 each 8 each 2 each | 1 1 1 | 400.00 1,300.00 3,600.00 | | 2,800 10,400 7,200 |
| Venturi meter | | l each | 1 | 5,000.00 | | 5,000 |
| River crossings | | | | lump sum | | 65,000 |
| Road resurfacing | | | | lump sum | | 35,000 |
| Right of way | | | | lump sum | | 16,000 |
| Subtotal | | | | | | \$1,578,300 |
| Administration and engineering, Contingencies, 15% Interest during construction | 10% | 2 | | | | \$ 157,800 236,700 39,500 |
| TOTAL | | ······ | | | | \$2,012,300 |
| ANNUAL COSTS | | | | | | |
| Interest, 4% Amortization, 40-year sinking fu Operation and maintenance | ind | at 4% | | | | <pre>\$ 80,500 21,200 5,000</pre> |
| TCTAL | | | | | | \$ 106,700 |

ESTIMATED COST OF SANTA CLARA RIVER CONDUIT SANTA FELICIA DAM TO SESPE CREEK

(Based on prices prevailing in spring of 1953)

apacity of conduit: 65 second-feet Length of conduit: 77,100 lineal feet

| : | : | Unit | : | |
|--|----------------------------------|--------------------------------|---------------------------------|--|
| Item : | Quantity : | price | : Cost | |
| APITAL COSTS | | | | |
| xcavation | 129,200 cu.yd. | \$ 0.90 | \$ 116,3 00 | |
| ackfill | 103,700 cu.yd. | 0.45 | 46,700 | |
| ipe, reinforced concrete, | | | | |
| 42-inch diameter 36-inch diameter | 54,200 lin.ft. 22,900 lin.ft. | 16.02 11.65 | 868,300 266,800 | |
| ittings | | lump sum | 56,800 | |
| alves Air release, 4-inch dia. Blowoff, 6-inch dia. Gate, 36-inch dia. | 7 each 8 each 2 each | 400.00 1,300.00 3,600.00 | 2,800 10,400 7,200 | |
| enturi meter | l each | 5,000.00 | 5,000 | |
| iver crossings | | lump sum | 65,000 | |
| bad resurfacing | | lump sum | 35,000 | |
| ight of way | | lump sum | 16,000 | |
| Subtotal | | | \$1,496,300 | |
| Mministration and engineer: Ontingencies, 15% Interest during construction | ing, 10% n | | \$ 149,600 224,400 37,400 | |
| TOTAL | | | \$1,907,700 | |
| INUAL COSTS | | | | |
| terest, 4% ortization, 40-year sinkin eration and maintenance | ng fund at 4% | | \$ 76,300 20,100 4,800 | |
| TOTAL | | | \$ 101,200 | |

ESTIMATED COST OF SANTA CLARA RIVER CONDUIT SESPE CREEK TO OXNARD RESERVOIR

(Based on prices prevailing in spring of 1953)

Capacity of conduit: 120 second-feet Length of conduit: 92,500 lineal feet

| Item | Quantity | price : | Cost |
|---|--|---|------------------------------------|
| CAPITAL COSTS | | | |
| Excavation | 211,000 cu.yd. | \$ 0.90 | \$ 189 , 900 |
| Backfill | 159,700 cu.yd. | 0.45 | 71,900 |
| Pipe, reinforced concrete, furnish and install 54-inch diameter 48-inch diameter 42-inch diameter | 67,500 lin.ft. 15,000 lin.ft. 10,000 lin.ft. | 20.10 16.95 14.25 | 1,356,800 254,300 142,500 |
| Fittings | | lump sum | 81,700 |
| Valves Air release, 4-inch dia. Blowoff, 8-inch dia. Gate, 48-inch dia. Venturi meter | 4 each 3 each 3 each 1 each | 400.00 1,600.00 8,700.00 10,000.00 | 1,600 4,800 26,100 10,000 |
| Pood nocumfacing | | lump cum | 10,000 |
| Right of way Subtotal | | lump sum | <u>10,600</u> \$2,280,200 |
| Administration and engineering, 10 Contingencies, 15% Interest during construction | % | | \$ 228,000 342,000 57,000 |
| TOTAL | | | \$2,907,200 |

| ANNUAL COSTS | |
|---|-----------------------------------|
| Interest, 4% Amortization, 40-year sinking fund at 4% Operation and maintenance | <pre>\$ 116,300 30,6007,300</pre> |
| TOTAL | \$ 154,200 |

ESTIMATED COST OF SANTA CLARA RIVER CONDUIT SESPE FEEDER

(Based on prices prevailing in spring of 1953)

apacity of conduit: 55 second-feet Length of conduit: 28,800 lineal feet

| : | | | Un | i.t | : | | |
|------------------------------|---------|---------|-----|--------------|---|-----------|-------------|
| Item : | Quant | ity : | pr | ice | ; | Cost | |
| APITAL COSTS | | | | | | | |
| ivension works | | | | | | | |
| Excavation | 9,770 | cu.yd. | \$ | 4.00 | | \$ 39,100 | |
| Concrete | | | | | | | |
| Weir and cutoff | 3,620 | cu.yd. | • | 35.00 | | 126,700 | |
| Reinforcing steel | 109,900 | lbs. | | 0.15 | | 16,500 | |
| Trashrack steel | 16,700 | lbs. | | 0.20 | | 3,300 | |
| Outlet gates | | | lum | p sum | | 4,200 | (* 00). 100 |
| Sand trap | | | Tun | p sum | | 9,000 | φ 224,100 |
| ipe line | | | | | | | |
| Excavation | 43,200 | cu.yd. | | 0.95 | | 41,000 | |
| Pipe, reinforced con- | 35,140 | cu.ya. | | 0.45 | | 15,000 | |
| crete, furnish and | | | | | | | |
| install, 36-inch dia. | 28,800 | lin.ft. | | 11.65 | | 335,500 | |
| Valves, furnish and | | | Lum | p sum | | 16,800 | |
| install | | | | | | | |
| Air release, 4-inch dia. | 2 | each | 4 | 00.00 | | 800 | |
| Blowoff, 8-inch dia. | 2 | each | 1,5 | 00.00 | | 3,000 | |
| Meter and junction | 2 | each | lum | o sum | | 7,500 | |
| Road and railroad crossing | | | lum | p sum | | 3,000 | |
| Right of way | | | lum | p sum | | 8,500 | 437,900 |
| Subtotal | | | | | | | \$662,000 |
| | | | | | | | |
| iministration and engineerin | ıg, 10% | | | | | | \$ 66,200 |
| iterest during construction | | | | | | | 16,500 |
| | | | | | | | |
| TOTAL | | | | | | | \$844,000 |
| | | | | | | | |
| | | | | | | | |
| INUAL COSTS | | | | | | | |
| sterest, 4% | | | | | | | \$ 33,800 |
| portization, 40-year sinking | fund at | : 4% | | | | | 8,900 |
| peration and maintenance | | | | | | | 4,200 |
| TOTAL | | | | | | | \$ 46,900 |

ESTIMATED COST OF OXNARD PLAIN-PLEASANT VALLEY DISTRIBUTION SYSTEM

(Based on prices prevailing in spring of 1953)

Capacity of conduit: 120 second-feet Length of conduit: 174,500 lineal feet

| Item | : | Quantity | : | Unit price | : | Cost |
|---|-----|---------------|-----|------------------|---|--|
| CAPITAL COSTS | | | | | | |
| Excavation | | 219,000 cu.yo | 1. | \$ 0 .9 0 | | \$ 197,100 |
| Backfill | | 178,400 cu.yo | d. | 0.45 | | 80,300 |
| Pipe, reinforced concrete, furnish and install | | 174,500 lin. | ŝt. | 9.49 | | 1,656,000 |
| Fittings | | | | lump sum | | 82,800 |
| Valves | | | | lump sum | | 35,700 |
| Line meters | | 3 each | | 4,000.00 | | 12,000 |
| Service outlets | | 191 each | | 150.00 | | 28,700 |
| Road and stream crossings | | | | lump sum | | 5,000 |
| Road resurfacing | | | | lump sum | | 10,000 |
| Regulating reservoirs | | | | lump sum | | 250,000 |
| Right of way | | | | lump sum | | 25,000 |
| Subtotal | | | | | | \$2,382,600 |
| Administration and engineering, Contingencies, 15% Interest during construction | 10 | z | | | | \$ 238,300 357,400 <u>59,400</u> |
| TOTAL | | | | | | \$3,037,700 |
| ANNUAL COSTS | | | | | | |
| Interest, 4% Amortization, 40-year sinking for Operation and maintenance | und | at 4% | | | | \$ 121,500 32,000 15,200 |
| TOTAL | | | | | | \$ 168,700 |
| | | | | | | |

ESTIMATED COST OF OXNARD-PORT HUENEME CONVEYANCE AND PUMPING SYSTEM

(Based on prices prevailing in spring of 1953)

Capacity of conduit: 40 second-feet Length of conduit: 50,700 lineal feet

| | | | : | Unit : | | |
|------------------------------|----------|---------|----|----------------------|----------------|----------|
| Item | Qua | ntity | : | price : | Cost | |
| CAPITAL COSTS | | | | | | |
| Pipe line | | | | | | |
| Excavation | 79,400 | cu.yd. | \$ | 0.95 \$ | 75,400 | |
| Backfill | 65,900 | cu.yd. | | 0.45 | 29,700 | |
| Pipefurnish and install, | | | | | | |
| reinforced concrete | | | | | | |
| cylinder | | | | | | |
| 42-inch dia. | 18,550 | lin.ft. | | 14.25 | 264,300 | |
| 36-inch dia. | 4,250 | lin.ft. | | 11.65 | 49,500 | |
| 30-inch dia. | 15,600 | lin.ft. | | 9.30 | 145,100 | |
| 24-inch dia. | 12,300 | lin.ft. | | 7.15 | 87,900 | |
| Fittings (elbows, re- | | | | | | |
| ducers, enlargers, etc.) | | | | lump sum | 15,400 | |
| Valves-furnish and install | | | | 000 00 | | |
| Gate 30-inch dia. | 1 | each | | 1,800.00 | 1,800 | |
| Gate 30-inch dia. | 3 | each | | 1,200.00 | 3,600 | |
| Gate 24-inch dia. | 1 | each | | 800.00 | 7 000 | |
| Alr release | 5 | each | | 400.00 | 1,200 | |
| Line motors | 5 | each | | 500.00 | 1,500 r 800 | |
| Boad surfacing | | lin ft | | 0.80 | 7,600 | |
| Railroad arossings | 2,500 li | TTU-TO- | | 200.00 | 800 | |
| Bight of way | 4 | each | | | 10 000 \$ | 700 1.00 |
| itento or way | | | | rump sum | φ | 100,400 |
| Pumping system | | | | | | |
| Well, gravel packed, drilled | 16 | each | | 3.750.00 | 60,000 | |
| and cased, 18-inch dia. | | 00.011 | - | JJJJJJJJJJJJJ | ooyooo | |
| Pump and motor installed | 16 | each | 1 | 4.330.00 | 69.300 | |
| Pipefurnish and install | | 0.000 | | | -,,,,, | |
| reinforced concrete cylind | er | | | | | |
| 42-inch dia. | 1,500 | lin.ft. | | 14.25 | 21,400 | |
| 30-inch dia. | 2,880 | lin.ft. | | 9.30 | 26,800 | |
| 18-inch dia. | 3,220 | lin.ft. | | 5.25 | 16,900 | |
| Valvesfurnish and install | | | | | · | |
| Gate 18inch dia. | 7 | each | | 600.00 | 4,200 | |
| Check 18-inch dia. | 16 | each | | 300.00 | 4,800 | |
| Land acquisition | 40 | acres | | 3,000.00 | 120,000 | |
| Fencing | 9,600 | lin.ft. | | 1.00 | 9,600 | 333,000 |
| | | | | | | |

Subtotal

\$1,033,400

ESTIMATED COST OF OXNARD-PORT HUENEME CONVEYANCE AND PUMPING SYSTEM (Continued)

| Tt om | : | | : | Unit | : | Coat |
|--|----------------------------|----------------------------|---|-------|---|--|
| ICem | : | Quantity | | price | | COST |
| CAPITAL COSTS | | | | | | |
| Administration and eng Contingencies, 15% Interest during constr | ineering uction | , 10% | | | | \$ 103,300 155,000 25,800 |
| TOTAL | | | | | | \$1,317,500 |
| ANNUAL COSTS | | | | | | |
| Interest, 4% Amortization, 40-year Replacement, 30-year s Electrical energy Operation and maintena | sinking inking f nce | fund at 4% Sund at 3.5% | | | | <pre>\$ 52,700 13,900 1,300 20,400 6,000</pre> |
| TOTAL | | | | | | \$ 94,300 |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |

ESTIMATED COST OF PIRU-LAS POSAS DIVERSION CONDUIT

(Based on prices prevailing in spring of 1953)

apacity of conduit: 80 second-feet Length of conduit: 67,500 lineal feet

| Item : | : Quantity : | Unit : price : | Cost |
|---|----------------------------------|--------------------------------|----------------------------------|
| APITAL COSTS | | | |
| xcavation | 167,840 cu.yd. | \$ 0.95 | \$ 159,400 |
| ackfill | 123,820 cu.yd. | 0.45 | 55,700 |
| ipe, lock joint concrete cylinder, furnish and install 60-inch diameter 54-inch diameter | 54,800 lin.ft. 12,700 lin.ft. | 39•53 32•17 | 2,166,200 408,600 |
| alves Air release, 4-inch dia. Blowoff, 8-inch dia. Gate, 42-inch dia. | 9 each 9 each 4 each | 490.00 1,650.00 7,200.00 | 4,400 14,900 28,800 |
| ittings | | lump sum | 128,700 |
| ine meters | | lump sum | 15,000 |
| tructural concrete | 220 cu.yd. | 90.00 | 19,800 |
| iscellaneous metal | 14,600 lbs. | 0.55 | 8,000 |
| oad resurfacing | 25,000 lin.ft. | 1.55 | 38,800 |
| iver crossings | 700 lin.ft. | 40.65 | 28,500 |
| appy Camp Canyon Tunnel | 13,500 lin.ft. | 165.00 | 2,227,500 |
| ight of way | | lump sum | 50,000 |
| Subtotal | | | \$5,354,300 |
| dministration and engineering, 10% ontingencies, 15% nterest during construction | 16 | | \$ 535,400 803,100 267,700 |
| TOTAL | | | \$6,960,500 |
| INUAL COSTS | | | |
| iterest, 4% mortization, 40-year sinking fund peration and maintenance | at 4% | | <pre></pre> |
| TOTAL | | | \$ 369,000 |

ESTIMATED COST OF SPREADING WORKS IN EAST LAS POSAS, AND SIMI BASINS

(Based on prices prevailing in spring of 1953)

| Item : | Quantity | : Unit : price | : | Cost |
|-----------------------------------|------------------|-------------------|-------|----------------------------|
| CAPITAL COSTS | | | | |
| Happy Camp Spreading Works - Ca | pacity 50 second | l-feet | | |
| Levees | 89,220 cu.yd. | 0.35 | \$ | 31,200 |
| Strip checking grounds | 50 acres | \$300.00 | | 15,000 |
| Riprap Feeder system to basing | 2,600 lin ft | 7.60 | | 19,800 |
| Stilling wells | 3 each | 700.00 | | 2,100 |
| Corrugated metal culverts, | <i>y</i> 00000 | 100000 | | - , 200 |
| in place, including | | | | |
| end sections, and toe- | | | | |
| plates | 15 each | 240.00 | | 3,600 |
| Right of Way | 50 acres | 500.00 | | 25,000 \$ 101,50 |
| Happy Camp - Simi Lateral - Cap | acity 30 second- | feet | | |
| Excavation | 96,000 cu.yd. | 0.95 | | 91,200 |
| Backfill | 67,200 cu.yd. | 0.45 | | 30,200 |
| Pipefurnish and install | 1.8 000 Jan St | 02 77 | | 11.7 000 |
| 42-inch diameter concrete | 40,000 LINeI (* | | ل و ل | 7 100 7.269.80 |
| Valves | | ranb sau | | 1,400 3.,207,00 |
| Dry Canyon Spreading Works - Ca | pacity 30 second | l-feet | | |
| Levees | 121,200 cu.yd. | 0.35 | | 42,400 |
| Strip checking ground | 70 acres | 300.00 | | 21,000 |
| Stilling well | 1 each | 700.00 | | 700 |
| Corrugated metal culverts, | | | | |
| and sections and toe- | | | | |
| plates | 18 each | 240.00 | | 4.300 |
| Right of Way | 70 acres | 2,000.00 |] | 140,000 208,40 |
| | | | - | 83 F70 70 |
| Subtotal | | | |) و 7) 5 و ⊥ ۞ |
| Administration and engineering, | 10% | | | \$ 158,00 |
| Contingencies, 15% | | | | 237,00 |
| Interest during construction | | | | |
| ΤΟΤΑΤ. | | | | \$2.014.20 |
| 101 | • | | | T T T T T T T T T T |
| ANNUAL COSTS | | | | |
| Interest 1.9 | | | | ¢ 80.60 |
| Amortization, 10-year sinking f | und at 11% | | | 21,20 |
| Operation and maintenance | | | | 5.00 |
| | | | | |
| TOTAL | | | | \$ 106,80 |
| | | | | |

ESTIMATED COST OF FILLMORE WELL FIELD

(Based on prices prevailing in spring of 1953)

Capacity of pumps: 55 second-feet bross seasonal pumpage: 22,000 acre-feet

| | • | Unit | • |
|---|--------------------------------|---------------|------------------------------|
| Item | Quantity : | price | : Cost |
| APITAL COSTS | | | |
| ell, gravel packed, drilled and cased, 18-inch diameter | 18 each | \$3,080.00 | \$ 55,400 |
| ump, motor and equipment, installed | 18 each | 4,330.00 | 77,900 |
| 'ipe, welded steel 18-inch diameter 30-inch diameter | 3,600 lin.ft. 3,800 lin.ft. | 5.25 10.10 | 18,900 38,400 |
| alves | | lump sum | 5,000 |
| ence | 7,200 lin.ft. | 1.00 | 7,200 |
| egulating reservoir | | lump sum | 20,000 |
| ight of way | 30 acres | 1,500.00 | 45,000 |
| Subtotal | | | \$267,800 |
| dministration and engineering, 10 ontingencies, 15% nterest during construction | 0% | | \$ 26,800 40,200 3,300 |
| TOTAL | | | \$338,100 |

NNUAL COSTS

| nterest, 4% | \$ 13,500 |
|--|-----------|
| mortization, 40-year sinking fund at 4% | 3,600 |
| eplacement, 30-year sinking fund at 3.5% | 2,600 |
| lectric energy | 30,800 |
| peration and maintenance | 5,000 |
| TOTAL | \$ 55,500 |

ESTIMATED COST OF VENTURA COUNTY AQUEDUCT TO CONNECT WITH FACILITIES OF METROPOLITAN WATER DISTRICT OF SOUTHERN CALIFORNIA

(Based on prices prevailing in spring of 1953)

Capacity of conduit: 25 second-feet Length of conduit: 438,800 lineal feet

| : | 0 1 1 | : | Unit | : | 0 |
|---|---------------------------|-------------------------------|--------------------------------|-----|-----------------------------------|
| Item : | Quanti | ty : | price | | Cost |
| CAPITAL COSTS | | | | | |
| Excavation | 625,100 | cu.yd. | \$ 1.70 | \$ | 1,062,700 |
| Backfill | 516 , 500 | cu.yd. | 0.76 | | 392,500 |
| Pipe, lock joint concrete cylinder furnish and install, 36-inch diameter 24-inch diameter 18-inch diameter | 414,800 5,000 3,800 | lin.ft. lin.ft. lin.ft. | 17.30 8.50 6.50 | | 7,176,000 42,500 24,700 |
| Valves-furnish and install Air release - 3-inch diamete Blowoff - 6-inch diameter Gate | er 49 46 6 | each each each | 325.00 1,250.00 2,200.00 | | 15,900 57,500 13,200 |
| Venturi meter and equipment | 2 | each | 18,000.00 | | 36,000 |
| Fittings (Elbows, redu- cers, enlargers, man- holes, passholes, etc.) | | | lump sum | | 334,900 |
| Road surfacing Temporary Permanent | 16,600 23,700 | tons tons | 4.50 6.00 | | 74,700 142,200 |
| River crossings Railroad crossings | 3,450 620 | lin.ft. lin.ft. | 36.70 38.00 | | 126,600 23,600 |
| Santa Susana tunnel | 15,200 | lin.ft. | 165.00 | | 2,508,000 |
| Pumping plant and equipment | 2 | each | 145,500.00 | | 291,000 |
| Right of way | | | lump sum | | 76,000 |
| Subtotal | | | | \$1 | L2,398,000 |
| Administration and engineering Contingencies, 15% Interest during construction | , 10% | | | \$ | 1,239,800 1,859,700 929,800 |
| TOTAL | | - | | \$1 | 16,427,300 |

ESTIMATED COST OF VENTURA COUNTY AQUEDUCT TO CONNECT WITH FACILITIES OF METROPOLITAN WATER DISTRICT OF SOUTHERN CALIFORNIA

(Based on prices prevailing in spring of 1953)

Capacity of conduit: 50 second-feet. Length of conduit: 438,800 lineal feet.

| Ttem | : | Quant | itv | : | Unit price | : : Cost | |
|---|-------------------------------|--------------------|----------------------|---|--------------------------------|--|----------|
| CAPTTAL COSTS | | | | | | | |
| Franchien | | 442 600 | a)) | đ | 1 70 | ¢ 1 KO2 100 | ` |
| EXCAVALION | | 805,000 | cu.ya. | φ | T• (0 | φ 1,502,100 | |
| Backfill | | 675,200 | cu.yd. | | 0.76 | 513,200 |) |
| Pipe, lock joint concrete cylinder, furnish and install 48-inch diameter 28-inch diameter | | 414,800 5,000 | lin.ft. lin.ft. | | 32.65 9.60 | 13,543,200 48,000 |) |
| zd-men drameter | | 000 و (| TTITETOE | | 0,00 | 52,500 | |
| Valves-furnish and i Air release - 4-in Blowoff - 8-inch d Gate | nstall ch diame iameter | eter 49 46 6 | each each each | | 360.00 1,550.00 6,450.00 | 17,600 71,300 38,700 | |
| Venturi meter and equipment | | 2 | each | | 18,000.00 | 36,000 |) |
| Fittings (Elbows, re cers, enlargers, ma holes, passholes, e | du- n- tc.) | | | | lump sum | 645,300 |) |
| Road surfacing | | | | | | | |
| Temporary Permanent | | 30,300 42,400 | tons tons | | 4.50 6.00 | 136,400 254,400 |) |
| River crossings Railroad crossings | | 3,450 620 | lin.ft. lin.ft. | | 39.90 58.00 | 137,700 36,000 |) |
| Santa Susana tunnel | | 15,200 | lin.ft. | | 165.00 | 2,508,000 |) |
| Pumping plant and | | 2 | each | 2 | 244,900,00 | 489.800 |) |
| Dight of your | | ~ | | ~ | | 76.000 |) |
| right of way | | | | | Tumb anu | | - |
| Subtotal | | | | | | \$20,086,000 | J |
| Administration and e Contingencies, 15% Interest during cons | ngineer. | ing, 10% n | | | | \$ 2,008,600 3,012,900 1,506,400 | |
| TOTAL | | | | | | \$26,613,900 |) |

ESTIMATED COST OF VENTURA COUNTY AQUEDUCT TO CONNECT WITH FACILITIES OF METROPOLITAN WATER DISTRICT OF SOUTHERN CALIFORNIA

(Based on prices prevailing in spring of 1953)

Capacity of conduit: 75 second-feet Length of conduit: 438,800 lineal feet

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Lân Con Leti

| : Ttem | Quanti | tv | Unit price | : | Cost |
|--|----------------------|----------------------|--------------------------------|----|-------------------------------------|
| CAPITAL COSTS | quarter | <u> </u> | | | |
| Excavation | 1.008.900 | C12. VA. | \$ 1.70 | | \$ 1.715.100 |
| Beckfill | 763,000 | ou vd | φ <u>1.</u> ,0 | | 570 000 |
| DACKITT | 103,000 | cu.yu. | 0.10 | | 519,900 |
| Pipe, lock joint concrete cylinder, furnish and install, 54-inch diameter 30-inch diameter | 414,800 5,000 | lin.ft. lin.ft. | 38.90 10.10 | | 16,135,700 50,500 |
| 20-Inch diameter | 3,000 | 110.10. | 9.00 | | 30,500 |
| Valves-furnish and install Air release - 5-inch diam Blowoff - 10-inch diamete: Gate | eter 49 r 46 6 | each each each | 425.00 1,650.00 6,450.00 | | 20,800 75,900 38,700 |
| Venturi meter and equipment | 2 | each | 20 ,0 00.00 | | 40,000 |
| Fittings (elbows, redu- cers, enlargers, man- holes, passholes, etc.) | | | lump sum | | 771,300 |
| Road surfacing Temporary Permanent | 30,100 48,500 | ton ton | 4.50 6.00 | | 135,400 291,000 |
| River crossings Railroad crossings | 3,450 620 | lin.ft. lin.ft. | 43.47 67.00 | | 150,000 41,500 |
| Santa Susana tunnel | 15,200 | lin.ft. | 165.00 | | 2,508,000 |
| Pumping plant and equipment | 2 | each | 331,900.00 | | 663,800 |
| Right of way | | | lump sum | _ | 76,000 |
| Subtotal | | | | \$ | 23,330,100 |
| Administration and engineeri Contingencies, 15% Interest during construction | .ng, 10% | | | ¢. | 2,333,000 3,499,500 1,749,800 |
| TOTAL | | | | \$ | 30,912,400 |

ESTIMATED COST OF VENTURA COUNTY AQUEDUCT TO CONNECT WITH FACILITIES OF METROPOLITAN WATER DISTRICT OF SCUTHERN CALIFORNIA (Based on prices prevailing in spring of 1953)

Capacity of conduit: 100 second-feet Length of conduit: 438,800 lineal feet

| Ttom | : Ouent: | | : Unit | : | ' Goot | , |
|---|-------------|----------|------------|---|--------------|---|
| | : Quant. | <u> </u> | : price | | COST | |
| CAPITAL COSTS | | | | | | |
| Excavation | 1,151,200 | cu.yd. | \$ 1.70 | | \$ 1,957,000 | |
| Backfill | 851,000 | cu.yd. | 0.76 | | 646,800 | |
| Pipe, lock joint concrete cylinder, furnish and install, | | | | | | |
| 60-inch diameter | 414,800 | lin.ft. | 45.60 | | 18,914,900 | |
| 36-inch diameter | 5,000 | lin.ft. | 13.00 | | 65,000 | |
| 30-inch diameter | 3,000 | TTU•10• | 10.10 | | 30,400 | |
| Valves-furnish and insta | .11 | | | | 00 900 | |
| Air release - 5-inch d | liameter 49 | each | 425.00 | | 20,800 | |
| Gate | 40 | each | 14,680,00 | | 88,100 | |
| | Ŭ | 00011 | 1,,000,000 | | 00,100 | |
| equipment | 5 | each | 27,000.00 | | 54,000 | |
| Fittings (elbows, redu- cers, enlargers, man- holes, passholes, etc.) | | | lump sum | | 906,800 | |
| Road surfacing | | | | | | |
| Temporary | 37.000 | ton | 4.50 | | 166,500 | |
| Permanent | 54,250 | ton | 6.00 | | 325,500 | 6 |
| River crossings | 3,450 | lin.ft. | 47.11 | | 162,500 | |
| Railroad crossings | 620 | lin.ft. | 75.00 | | 46,500 | |
| Santa Susana tunnel | 15,200 | lin.ft. | 165.00 | | 2,508,000 | |
| Pumping plant and | | | | | | |
| equipment | 2 | each | 411,600.00 | | 823,200 | |
| Right of way | | | lump sum | | 76,000 | |
| Subtotal | | | | | \$26,875,900 | |
| Administration and engin | eering, 10% | | | | \$ 2,687,600 | |
| Contingencies, 15% | | | | | 4,031,400 | |
| Interest during construc | tion | | | | 2,015,700 | |
| TOTAL | | | | | \$35,610,600 | |
| | | | | | | |

ESTIMATED COST OF VENTURA COUNTY AQUEDUCT TO CONNECT WITH FACILITIES OF METROPOLITAN WATER DISTRICT OF SOUTHERN CALIFORNIA

(Based on prices prevailing in spring of 1953)

Capacity of conduit: 150 second-feet Length of conduit: 438,800 lineal feet

| | : : | Unit : | |
|--|---|---------------------------------|--|
| Item | : Quantity : | price : | Cost |
| CAPITAL COSTS | | | |
| Excavation | 1,464,300 cu.yd. | 1.70 | \$ 2,489,300 |
| Backfill | 1,035,000 cu.yd. | 0.76 | 786,600 |
| Pipe, lock joint concrete cylinder, furnish and install 72-inch diameter 42-inch diameter 36-inch diameter | 414,800 lin.ft. 5,000 lin.ft. 3,800 lin.ft. | 60.40 21.10 13.00 | 25,053,900 105,500 49,400 |
| Valves-furnish and install Air release - 6-inch dia. Blowoff - 12-inch dia. Gate | 49 each 46 each 6 each | 490.00 1,750.00 14,680.00 | 24,000 80,500 88,100 |
| Venturi meter and equipment | 2 each | 27,000.00 | 54,000 |
| Fittings (elbows, reducers, enlargers, manholes, pass- holes, etc.) | | lump sum | 1,200,400 |
| Road surfacing Temporary Permanent | 44,300 tons 66,400 tons | 4.50 6.00 | 199,400 398,400 |
| River crossings Railroad crossings | 3,450 lin.ft. 620 lin.ft. | 50.70 90.00 | 174,900 55,800 |
| Santa Susana tunnel | 15,200 lin.ft. | 165.00 | 2,508,000 |
| Pumping plant and equipment | 2 each | 501,650.00 | 1,003,300 |
| Right of way | | lump sum | 76,000 |
| Subtotal | | | \$34,347,500 |
| Administration and engineering, Contingencies, 15% Interest during construction | 10% | | \$ 3,434,800 5,152,100 3,434,800 |
| TOTAL | | | \$46,369,200 |

ESTIMATED COST OF OAK CANYON LATERAL

(Based on prices prevailing in the spring of 1953)

Capacity of conduit: 40 second-feet

Length of conduit: 4,010 lineal feet

| :Item : | Quantity | : | Unit price | : | Cost |
|--|----------------------------|----|--------------------------------|---|-----------------------------|
| CAPITAL COSTS | | | | | |
| Excavation | 7,100 cu.yd. | \$ | 0.90 | ģ | \$ 6,400 |
| Backfill | 5,700 cu.yd. | | 0.45 | | 2,600 |
| Pipe, lock joint concrete cylinder, furnish and install, | | | | | |
| 42-inch diameter | 4,010 lin.ft. | | 22.30 | | 89,400 |
| Valves - furnish and install Air release, 4-inch diameter Blowoff, 8-inch diameter Gate, 36-inch diameter | 2 each 2 each 1 each |] | 350.00 L,550.00 6,500.00 | | 700 3,100 6,500 |
| Fittings (elbows, reducers, enlargers, etc.) | |] | Lump sum | | 9,200 |
| Right of way | | נ | Lump sum | | 4,000 |
| Pumping plant and equipment . | |] | Lump sum | | 38,500 |
| Subtotal | | | | | \$160,400 |
| Administration and engineering, Contingencies, 15% Interest during-construction; no | 10% ne | | | ģ | <pre>\$ 16,000 24,100</pre> |
| TOTAL | | | | | \$200,590 |

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ESTIMATED COST OF OAK CANYON DAM AND RESERVOIR WITH STORAGE CAPACITY OF 7,500 ACRE-FEET

(Based on prices prevailing in spring of 1953)

Elevation of crest of dam: 1,110 feet Capacity of reservoir to crest Elevation of crest of spillway: 1,100 feet of spillway: 7,500 acre-feet Height of dam to spillway crest, Capacity of spillway with 5-foot above stream bed: 170 feet freeboard: 2,000 second-feet Unit : : : Cost Item Quantity price : : : CAPITAL COSTS Dam Exploration lump sum \$ 10,000 Diversion of stream and dewatering of foundation 1,000 lump sum 25,000 cu.yd. \$ Stripping topsoil 0.60 15,000 Foundation excavation Abutment 22,000 cu.yd. 1.50 33,000 Channel 0.60 91,000 cu.yd. 54,600 Embankment Impervious 586,400 cu.yd. 0.65 381,200 0.55 Random 1,000,300 cu.yd. 550,200 4.00 157,600 Rock riprap 39,400 cu.yd. Drilling grout holes 6,400 lin.ft. 3.00 19,200 Pressure grouting 4,200 cu.ft. \$1,238,600 4.00 16,800 Spillway Excavation 900 cu.yd. 1.50 1.400 Concrete Weir and cutoff 230 cu.yd. 35.00 8,000 Floor 420 cu.yd. 30.00 12,600 Walls 420 cu.yd. 16,800 40.00 75,600 Reinforcing steel lbs. 0.15 11,300 50,100 Outlet Works Tower concrete 580 cu.yd. 80.00 46,400 Concrete encasement 470 cu.yd. 40.00 18,800 Steel pipe, 48-inch dia. 1,020 lin.ft. 25.00 25,500 Tower inlet valve, 30inch dia. 4 each 3,000.00 12,000 Needle valve, 42-inch dia. 1 12,000.00 12,000 each 17,500 lbs. Miscellaneous metal work 0.40 7,000 121,700 Reservoir 48,000 Land acquisition lump sum 160 acres 1,600 49,600 Clearing 10.00 \$1,460,000 Subtotal Administration and engineering, 10% 146,000 219,000 Contingencies, 15% 36,000 Interest during construction TOTAL \$1,861,000

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