

DRAFT

System Reoperation Study
Forecast-Based Operations Analysis

Technical Report

Prepared for
Department of Water Resources

by



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Nomenclature

F-BO	forecast-based operation
gross pool	maximum top of conservation space
operation zone	reservoir storage is divided into operation zones, or pools, which are each governed by a specific set of prioritized rules
spill	outflow in excess of the typical operations release
top-con	top of conservation zone in reservoir
USACE	United States Army Corps of Engineers

1

Background

1.1 System Reoperation

The System Reoperation Study (“Reoperation Study”) is investigating changes in operations and other strategies that may result in improved system performance in terms of additional water supply, flood hazard reduction, and ecosystem protection and restoration. It is recognized that existing reservoir operations are typically based on meeting all or some of these three objectives. As such, the Reoperation Study focuses on modifications to existing operations and expansion of these benefits. The Reoperation Study team has developed several reoperation components to evaluate at select river/reservoir systems to quantify benefits and improve understanding of the trade-offs involved.

MBK Engineers is providing technical, strategic, and coordination support for the Reoperation Study. An analysis of Forecast-Based Operation (F-BO) is part of that support. The following report summarizes this effort.

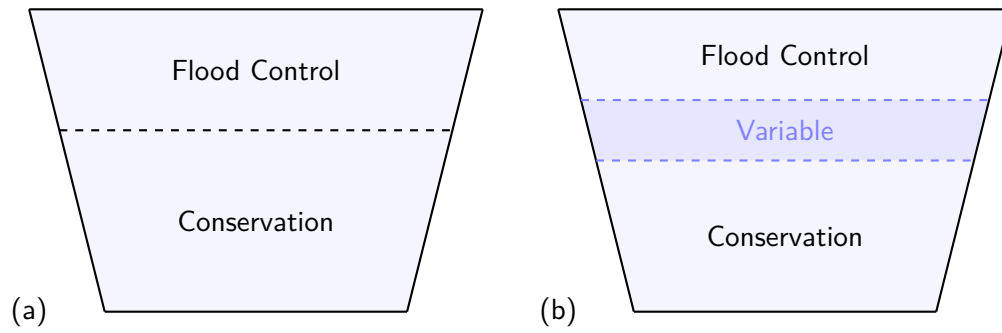
1.2 Forecast-Based Operation

F-BO can be applied to any reservoir with a traditional water supply and flood control space allocation paradigm. The incorporation of weather forecasts allows for greater flexibility in the management of the reservoir’s space (flood control versus conservation) and inherently increases the opportunities for gaining additional water supply and flood control benefits through its implementation.

System reoperation is informed by historical flood patterns and utilizes records of inflow, outflow, and storage in the reservoirs to develop modified rules for flood releases. Changes to historical operations of the reservoirs is expected to reveal potential for additional water supply storage. With improvements to many of the reservoir facilities, such as increased release capacity, operations are no longer required to be as conservative as they once were. By reducing outflow and allowing an increase in water storage, significant improvements in water supply can be achieved. However, care must be taken so that sufficient water can still be evacuated in the event of a significant forecasted flood.

Figure 1.1 depicts the typical, rigid flood/conservation space division and flexible division under F-BO.

Figure 1.1. Depiction of reservoir space allocations under (a) typical operating paradigm and (b) forecast-based operation.



2

Purpose

The F-BO portion of the system reoperation study evaluated the potential in reoperating the system reservoirs for two primary purposes:

1. Enhancement of water supply through the relaxation of traditional reservoir flood control rules
2. Enhancement of the flood protection provided by each reservoir.

2.1 Water Supply Enhancement

Reoperation allows additional water to be stored in the reservoir temporarily. Reducing regions of “spill,” or times when outflow exceeds the standard operations release, offers a potential for increased storage throughout the year. This may improve water supply after the reservoir fills in the spring. Using spill to define potential times for reoperation resulted in increased storage during periods where a historical spill was deemed unnecessary.

It is proposed that the portion of the analysis aimed to enhance water supply through the implementation of F-BO consist of the following components:

1. Review historical reservoir storage data and flood space requirements to determine when relaxed flood control space requirements could have potentially provided additional water supply during the subsequent season.
2. Develop parameters for F-BO flood space relaxation. Parameters will represent a reasonable level of assumed flood risk (e.g., flood space reduction will be of comparable volume to that which can be evacuated within several days when a significant flood event is forecast and the reserved flood space must increase).
3. Evaluate historical benefits possible with proposed F-BO flood space relaxation approach.
4. Integrate flood space reduction results (end-of-month flood control targets) with system-wide modeling to make use of anticipated water supply gained through F-BO.

2.2 Flood Control Enhancement

The flood management benefits of F-BO vary greatly by river system. The portion of the analysis aimed to enhance flood control through the implementation of F-BO is proposed to consist of the following components:

1. Review and compare historical reservoir storage and river flow data with flood rules and space requirements to determine when additional flood control storage or modification of flood rule parameters would have been beneficial to enhance flood control.
2. Identify factors at each reservoir potentially limiting F-BO effectiveness (e.g., limited outlet capacity or insufficient flood space).

2: Purpose

3. Coordinate with National Weather Service/State California-Nevada River Forecast Center to assess level of confidence in using operational weather and reservoir inflow forecasts for decision-making at reservoirs.
4. Develop comparison of each reservoir's potential for use of F-BO to enhance flood control. The comparison will identify the reservoirs' potential in each of several areas (flood space, outlet capacity, channel capacity, etc.).

3

Watershed and Reservoir Characteristics

The following rivers and reservoirs were included in the forecast-based operations analysis performed for the system reoperation study:

- Sacramento River / Shasta Dam
- Feather River / Oroville Dam
- North Yuba River / New Bullards Bar Dam
- American River / Folsom Dam
- Merced River / New Exchequer Dam

Figure 3.1 shows the basin outlines and locations of the reoperation reservoirs within these watersheds. The watersheds experience seasonal flooding primarily influenced by winter rainfall events, with some augmentation due to snowmelt. Rainfall floods in these regions are characterized by smaller volumes but large peaks, while snowmelt-induced floods generally have small peaks spread over much longer durations.

Figure 3.1. Study Region



3.1 Sacramento River Watershed (Shasta Reservoir)

Shasta Dam and Reservoir (Shasta) are located on the Sacramento River below its confluence with the Pit River, about 10 miles north of Redding, CA. The mainstem originates in the Klamath Mountains and flows through Central California. The Sacramento River drains over 27,500 square miles and ends in the Delta. Shasta is a key unit of the Central Valley Project (CVP); irrigation diversions and power generation at Shasta are integrated into the CVP.

Though a number of small privately owned irrigation dams and several private hydroelectric power generation facilities exist on the Pit River and its tributaries, none of these contribute significantly to storage upstream of Shasta Reservoir.

Shasta is owned and operated by the United States Bureau of Reclamation (Reclamation). Flood control operation procedures are coordinated with input from United States Army Corps of Engineers (USACE). The California Department of Water Resources (DWR) coordinates irrigation diversions from Shasta and also provides hydrologic forecasting services. The California Department of Fish and Wildlife (CDFW) coordinates fishery releases with Reclamation. The National Weather Service (NWS) provides hydrologic forecast support.

3.2 Upper Feather River Watershed (Oroville Reservoir)

Oroville Dam and Reservoir (Oroville) were constructed as a unit of the Feather River Project (part of the State Water Project, or SWP) for the purposes of developing water supply, flood control, power generation, recreation and conservation on the Feather River, a tributary of the Sacramento River. The Upper Feather River Watershed drains 3,611 square miles of the eastern side of the Sacramento River Valley and northern end of the Sierra Nevada Range. Flood peaks on streams in the basin above Oroville are often delayed by numerous upstream checkdams, diversions and reservoirs. When high flows in the Feather coincide with large flows in downstream tributaries, channel capacity may be exceeded. Many of the storage reservoirs upstream of Oroville are maintained for power generation facilities managed by PG&E. The combined capacity of all upstream reservoirs is close to 2,000,000 acre-feet. Several of the upstream reservoirs may influence runoff volumes at Oroville during large floods, though most are negligible.

DWR operates Oroville with input from USACE for flood control procedures and CDFW for fishery releases. The operation of Oroville by DWR is closely coordinated with YCWA's operation of New Bullards Bar in order to meet safe objective downstream channel capacities during the flood season.

3.3 Yuba River Watershed (New Bullards Bar Reservoir)

New Bullards Bar Dam and Reservoir (New Bullards Bar) regulate over a third (36%) of the Yuba River Watershed, a drainage area of about 1,350 square miles which lies entirely on the western slope of the Sierra Nevada. There are no reservoirs upstream of New Bullards Bar on the Yuba, but nearly 20 reservoirs with over 1,000 acre-feet of storage exist in the Yuba Watershed. These reservoirs contain no flood control space but may significantly reduce volumes of rainfall runoff to New Bullards Bar early in the season.

The operation of New Bullards Bar by the Yuba County Water Agency (YCWA) is closely coordinated with DWR's operation of Oroville in order to meet safe objective channel capacities downstream of

3: Watershed and Reservoir Characteristics

the two reservoirs during flood season. DWR provides assistance with hydrologic forecasting. Flood control operations are coordinated with USACE, which also provides hydrologic forecasting. Fishery releases are coordinated with CDFW.

3.4 American River Watershed (Folsom Reservoir)

Folsom Dam and Reservoir (Folsom) regulate the majority of the water passing through the American River Watershed. This watershed originates in the Sierra Nevada range and drains toward the delta in California's Central Valley. The Upper American River Watershed houses several reservoirs that were developed primarily for hydropower production but also provide some flood protection and water supply. Hell-Hole, French Meadows, and Union Valley Reservoirs can be used to transfer flood protection credit to Folsom Reservoir, thus reducing Folsom's flood space requirement.

The operation of Folsom is managed by Reclamation, with flood control operations input provided by USACE and the Sacramento Area Flood Control Agency (SAFCA). The United States Fish and Wildlife Service (USFWS) and CDFW coordinate fishery releases for Folsom with Reclamation. Hydrologic forecasting is provided by NWS and DWR, (USACE-SPK, 1987).

3.5 Merced River Watershed (New Exchequer Reservoir)

New Exchequer Dam and its reservoir, Lake McClure, are operated by the Merced Irrigation District (MID). The facilities are located 60 miles upstream of Merced River's confluence with the San Joaquin River and regulate the 1,276-square mile Merced River Watershed on the western slopes of the Sierra Nevada. MBK has prepared a separate report (MBK, 2013) which details forecast-based system reoperation analysis for New Exchequer and its associated facilities.

4

Reservoir Operation Zones

Storage space in each reservoir is divided into specific operation zones, which are defined in the reservoir water control manuals, e.g., USACE-SPK, 1987. The zones are defined by water level in the reservoir. Each zone has a specific set of prioritized rules that govern its storage and operations. For some of the facilities, different government entities are responsible for operation of the dam for different zones. Operation zones are identified and defined in Table 4.1.

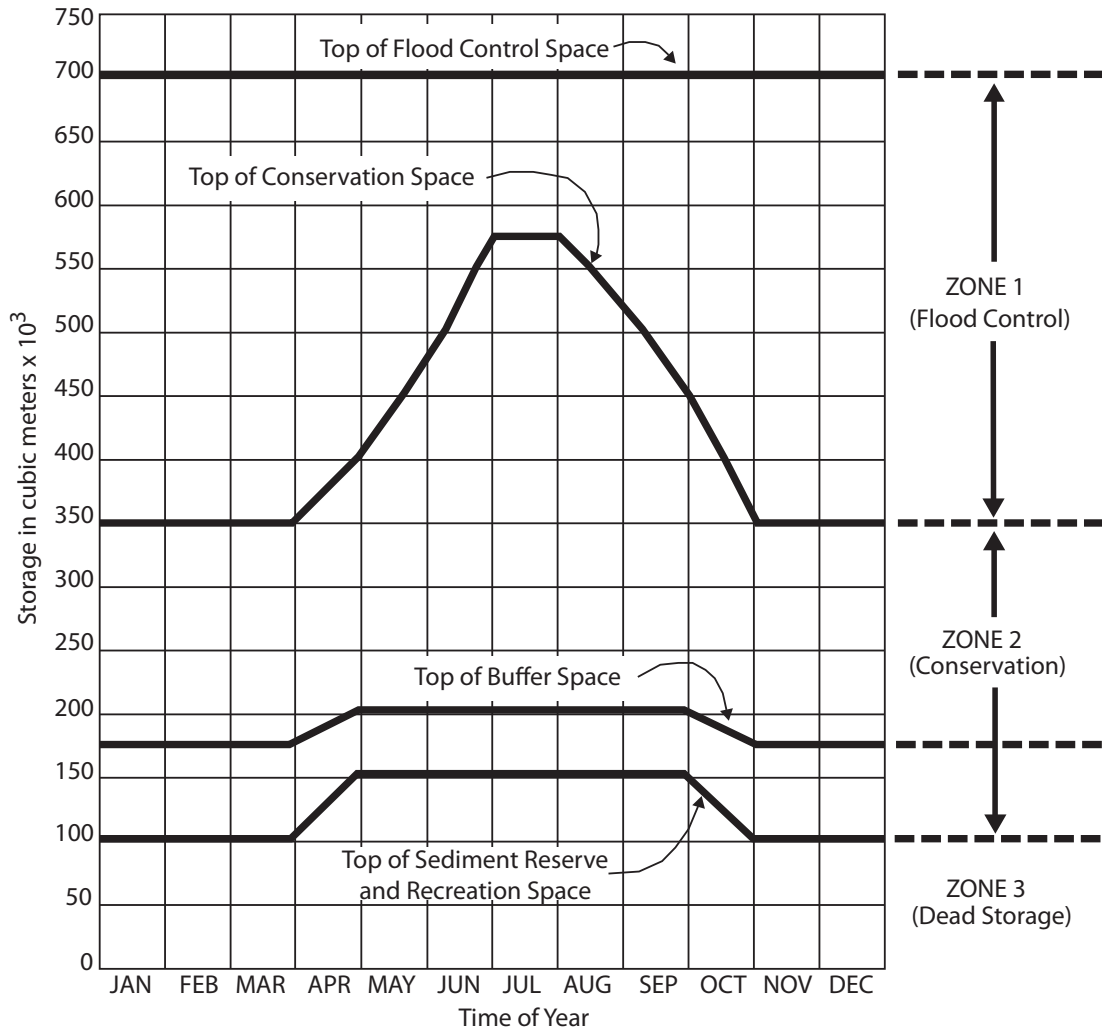
Table 4.1. Reservoir Operation Zones

Zone	Description
Inactive "Dead" Zone	The first zone is referred to as the inactive zone, or dead storage. This region lies below the outlet gates, so it is effectively dead water that cannot be released.
Conservation Zone	Above the inactive zone lies the conservation zone, which is important for storing water supply. The top of the conservation zone is called top-con. Maximum top-con, which occurs after the reservoir fills in the spring, is referred to as gross pool.
Flood Control Zone	The flood control zone, which sits above top-con, is the space reserved for accommodating flood waters. Ideally, all flood waters will be stored in this zone during high flood risk seasons. Encroachment of storage into the flood control zone may be used during forecast low flood-risk periods in order to provide additional water supply. The USACE designates operation practices for the flood control zone for the study reservoirs.
Surcharge Zone	The surcharge zone is the region between the top of the flood control zone and the top of dam. Surcharge is the term given to water stored temporarily above the reservoir capacity. Jurisdiction over the surcharge zone is maintained by the primary operating agency for each reservoir. Surcharge is operated with the main priority of maintaining dam safety.

Figure 4.1 provides graphical representation of how operation zones may vary throughout the year. Note that the top of conservation (top-con) reaches a maximum elevation (gross pool) in the drier months of summer, when the least amount of reserved flood space is necessary (lowest threat of flooding).

4: Reservoir Operation Zones

Figure 4.1. Typical Reservoir Operation Diagram (USACE, 1997)



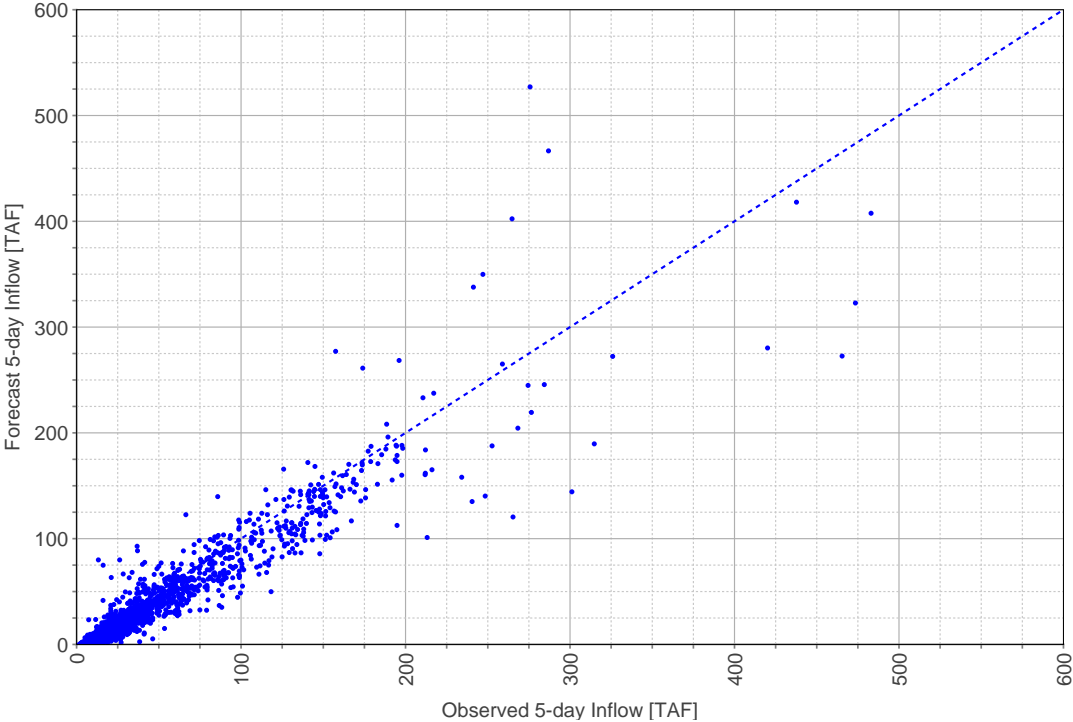
5

Evaluation of Forecast Reliability

Reservoir operators already use reservoir inflow and downstream flow forecasts to make operational decisions. In order for the use of forecasts to be formalized in reservoir operational rules, such forecasts must be reasonably accurate and reliable. The California-Nevada River Forecast Center (CNRFC) is the agency responsible for producing these forecasts. Forecasts are produced at least once and up to four times daily. Historical forecasts and the corresponding observations from 2005 to the present were made available by the CNRFC for the five reservoirs analyzed in this study.

In Figure 5.1 through Figure 5.4, observed five-day accumulated inflow volumes in thousand acre-feet (TAF) were plotted against the corresponding forecasted volumes. A line with a one-to-one slope was overlaid on each of the plots to indicate where perfect forecasts would plot. The tight clustering of the data points around this ideal trend line indicates that there exists an acceptable level of forecast accuracy for using forecasts to make real-time operational decisions, as long as the forecast-based action only risks a portion of the forecast volume and can be corrected if the forecast trends significantly change.

Figure 5.1. Forecast Accuracy for Folsom (12/02/2005 to 04/01/2013)



5: Evaluation of Forecast Reliability

Figure 5.2. Forecast Accuracy for New Bullards Bar (12/02/2005 to 04/01/2013)

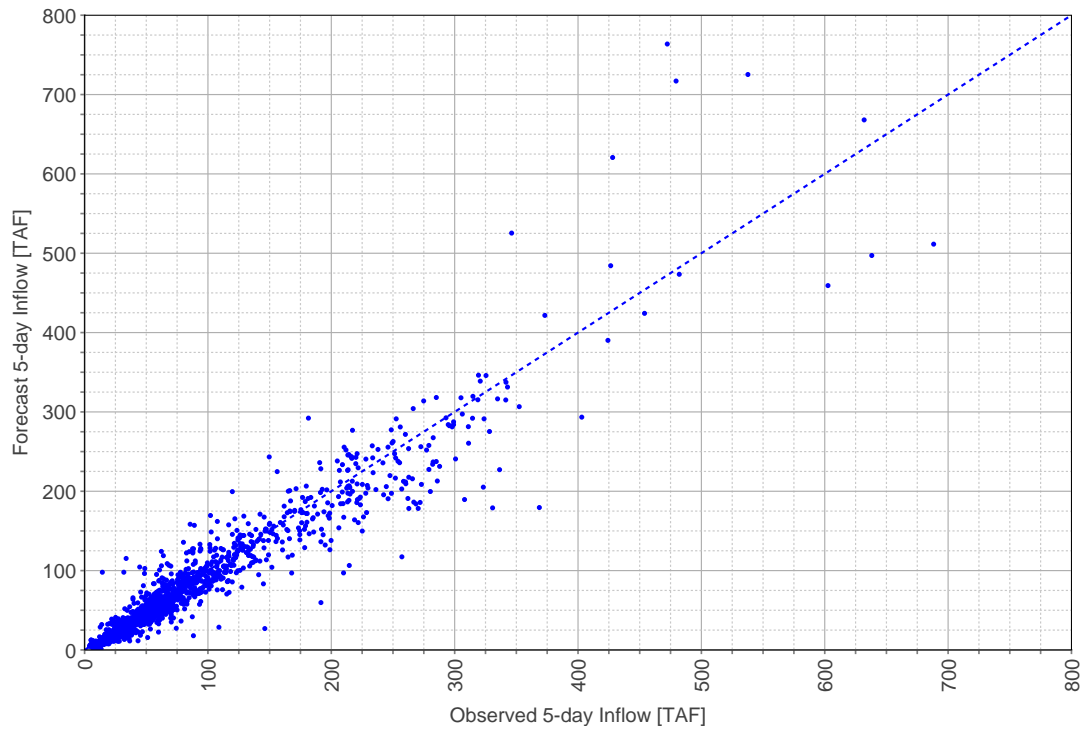


Figure 5.3. Forecast Accuracy for Oroville (12/02/2005 to 04/01/2013)

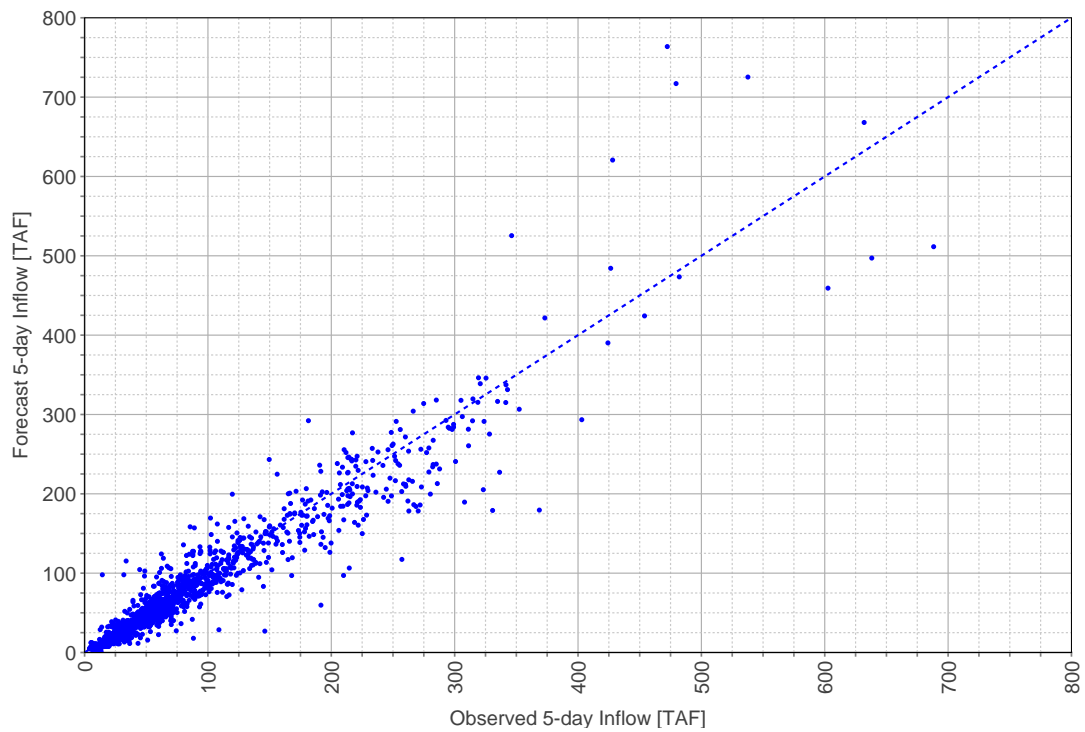
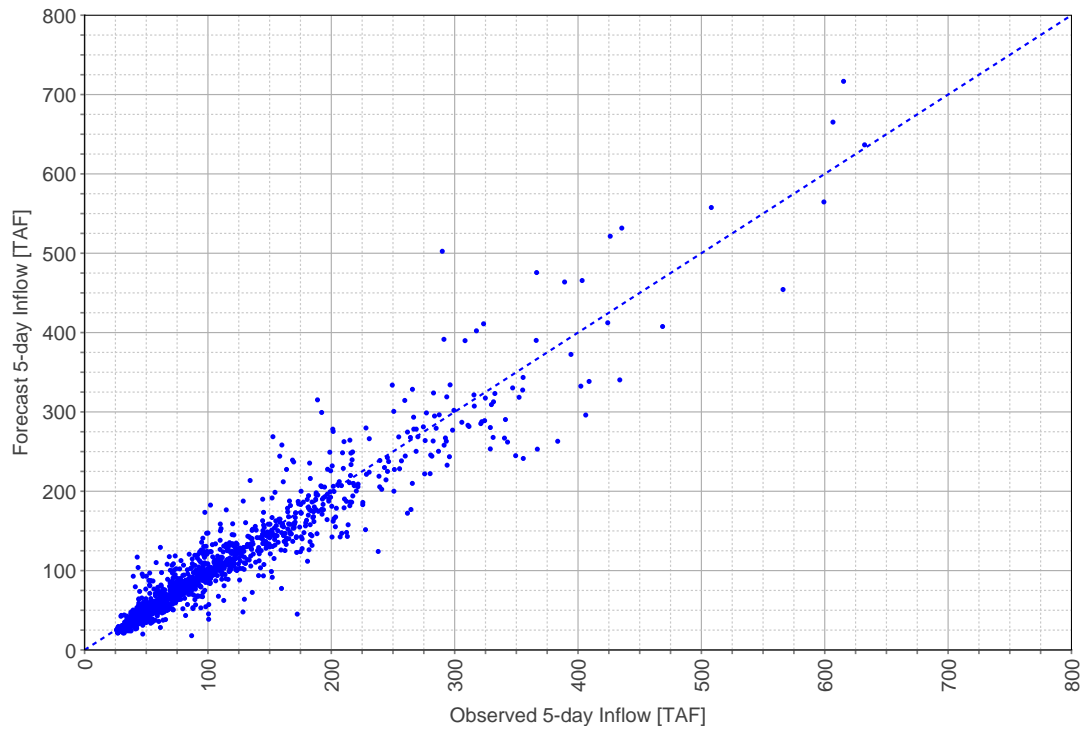


Figure 5.4. Forecast Accuracy for Shasta (12/02/2005 to 04/01/2013)



6

Water Supply Enhancement

F-BO can be used to enhance water supply at each reservoir. This is achieved by storing water above traditional flood control limits during times when significant inflow volume to the reservoir is not predicted. A means to draw the reservoir back down to the traditional flood control limit is provided as part of the reoperation procedure for times when significant inflow volume is forecast.

The following chapter details the reservoir model development and the methods for quantifying increased water supply with F-BO.

6.1 Methodology

The model used for this analysis was developed by MBK Engineers based on historical reservoir inflow and outflow records. The following sections describe the constraints and assumptions used for developing the criteria applied to obtain reoperated reservoir outflow and storage values.

6.2 Analysis Assumptions

This section provides background on some of the key modeling and hydrologic assumptions used for this analysis.

1. Daily Reoperation

Forecast-based system reoperation for the four reservoirs was analyzed in a daily operations model. Operational decisions based on forecast inflow are made on a daily basis, so a daily model is most appropriate for this simulation. This timestep provided a suitable level of detail for the required analysis.

2. Perfect Forecasting

Perfect foresight of historical inflow values was used to perform forecasting for the daily reservoir operation simulations. As discussed in Chapter 5, the strong correlation between observed and forecast data justifies extending the reoperation methodology to real-time operations.

In a real-time operational environment, operators will not have the luxury of perfect forecasts. The use of perfect forecasts for this analysis only pertains to the drawdown trigger. The fact that the forecasts are perfect is not considered a critical factor in this analysis. That is, the proposed procedure for water supply enhancement should work equally well with uncertain forecasts in the real-time operational environment, provided the trigger levels are adjusted to account for forecast uncertainty.

3. Reservoir Fill Constraints

For the purpose of this study, gross pool storage was used as the maximum reservoir storage constraint. Historically, reservoir refill was occasionally allowed to encroach above the level of

gross pool storage. Outflow was required to exceed the rate necessary to maintain storage below gross pool levels. For several of the reservoirs, the top-con curve has been revised over time. Thus, gross pool was noted each water year to set this constraint.

4. Spill Threshold Values

A typical operation level for each reservoir was selected to provide a baseline outflow value for the reoperation routine. When the flood pool is encroached, typical reservoir operations procedures dictate that outflow should increase so as to draw down storage to the acceptable level (top-con). This may involve increasing release to above typical operation levels, termed “spill” for this study. Reoperation was targeted to times when outflow exceeded the spill threshold value.

5. Flood Space Evacuation Targets

Although the goal of the reoperation was to increase water supply storage in the reservoirs, flood management goals must also be met. As such, a flood space evacuation routine was written into the reoperation so that temporary storage could subsequently be evacuated if a large enough inflow was forecast. This process involved selection of accumulated inflow targets. If forecast inflow over a selected time window exceeded the target for a particular reservoir, the reoperated outflow immediately switched to releasing this accumulated inflow volume divided evenly over the window, which ensured that the storage curve would draw back down to top-con. For this study, the evacuation targets were compared to the five-day accumulated inflow volumes at each timestep.

6. Flood Pool Encroachment Constraints

Historically, many of the reservoirs have been allowed to encroach into the flood pool. There are various reasons for this. In order to be consistent with historical practices, some allowance for this was made in the simulation. Flood pool encroachment was limited to the maximum of either one quarter of the available flood space or the historical storage level. This essentially resulted in a modified top-con curve, which was plotted along with the reoperation results in Appendix A. Outflow was required to exceed the rate necessary to maintain storage levels below the encroachment limit.

7. Physical Constraints of Reservoir Facilities

Physical constraints of the reservoir outlets also constrain the releases. Maximum reservoir outflow capacity and Rate of Increase (ROI) limitations were important for setting maximum limits on outflow, while Rate of Decrease (ROD) set the minimum outflow limit.

Table 6.1 summarizes a number of parameters that were used in the calculation of outflow and storage values for the reoperation procedure.

Table 6.1. Parameters for Reoperation Procedure

	Folsom	New Bullards Bar	Oroville	Shasta
Model Parameter				
Spill Threshold [cfs]	5,000	8,000	10,000	10,000
5-day Inflow Evacuation Trigger [acre-feet]	400,000	280,000	700,000	650,000
Analysis Period	1956-2010	1971-2010	1969-2010	1954-2010
Outflow Constraint				
Reservoir Fill Limit, Gross Pool [AF]	1,010,000 ^a	966,000	3,538,000	4,493,000 ^d
	974,500 ^b			4,552,100 ^e
	977,000 ^c			
Flood Pool Encroachment Limit [%]	25	25	25	25
Rate of Increase Limit [cfs/hr]	7,500	5,000	5,000	7,500
Rate of Decrease Limit [cfs/hr]	5,000	5,000	2,500	2,000
Reservoir Release Capacity [cfs]	115,000	50,000	150,000	79,000

Notes: a.1956-1992; b.1993; c.1994-2010; d.1954-1970; e.1971-2010

Each reservoir storage constraint was converted to a maximum or minimum release value for comparison against a base outflow quantity. The base outflow quantity for the reservoir reoperation model consisted of either the historical release, a typical operation level (threshold release), or an evacuation draw-down release, as described in Section 6.3.3.

6.3 Reservoir Operations Model

This section briefly outlines the reservoir operations model developed for the reservoir reoperation protocols.

6.3.1 Historical Reservoir Data

The inputs for the reoperation routine consisted of DSS records of daily historical inflow, outflow and storage values obtained from USACE. These series were then read into the reoperation routine where potential water supply enhancement and flood space evacuation zones were identified using comparisons of the inflow and outflow data to specific threshold values detailed in Section 6.2. Within the reoperation routine, a number of constraints based on reservoir storage levels and physical release limitations were applied to the base outflow value to obtain a new reoperation outflow value. Reoperated storage levels were then calculated from the historical inflow and storage data and the simulated reoperation outflow.

6.3.2 Water Supply Enhancement Zones

For the purpose of this analysis, outflow releases in excess of the typical threshold operation levels were considered “spill,” as described in Section 6.2. This spill was used as a metric to determine

the appropriate times to apply a reoperation procedure in order to enhance water supply in the reservoir. Zones of continuous spill, or water supply enhancement zones, were tabulated in order to define where reoperation would activate. If a timestep fell within a spill zone and did not meet the criteria for flood space evacuation, the reoperation proceeded. Otherwise, the outflow was held to the historical release, with certain constraints applied. In the resulting reoperation plots, the water supply enhancement zones are highlighted in purple (Appendix A).

6.3.3 Flood Space Evacuation Zones

In certain cases, holding reservoir outflow at a constant threshold value caused storage in the reservoir to surpass acceptable limits. As such, a flood space evacuation trigger was included in the procedure in order to prevent excessive encroachment. During the simulation, a forecast window was selected so that current storage levels could be compared with future top-con storage. If the accumulated inflow volume over the duration of the forecast window exceeded the reservoir evacuation target, then the window was added to a flood space evacuation zone. Deactivation of water supply enhancement protocols also occurred when reoperated storage exceeded top-con.

When evacuation was triggered, outflow was shifted to a conversion of the accumulated evacuation volume spread out as a constant release for the duration of the evacuation period. The release volume was calculated as the accumulated inflow over the evacuation zone plus the difference between the reoperated storage value and the level of top-con at the end of the evacuation window. This ensured release of the volume necessary to draw the reoperated storage curve back down to top-con. If reoperation had never previously occurred in the water year, outflow in the evacuation region was held at the historical level, constrained by additional limitations described below. Flood space evacuation zones in the resulting storage and flow plots are highlighted in orange (Appendix A).

6.3.4 Other Simulation Regions

If a particular timestep did not fall into either a water supply enhancement or flood space evacuation region, the flow was held to historical outflow, with the inclusion of the outflow constraints described in Table 6.1.

6.3.5 Reoperated Outflow

Designating the water supply enhancement and flood space evacuation zones allowed the appropriate base outflow value to be selected for each timestep in the model. The following table summarizes the conditions for determining the base outflows.

Table 6.2. Summary of Base Outflow Conditions

Water Supply Enhancement Zone?	Previous "Spill" in Water Year?	Flood Space Evacuation Zone?	Storage Above Top-Con?	Base Outflow
N	N	N	Y/N	max of flood pool encroachment constraint & historical release
Y/N	N	Y	Y/N	max of flood pool encroachment constraint & historical release
N	Y	Y	N	max of flood pool encroachment constraint & historical release
N	Y	N	Y/N	max of flood pool encroachment constraint & historical release
Y	Y/N	N	Y/N	max of flood pool encroachment constraint & threshold release
Y/N	Y	Y	Y	evacuation release

After determination of the base outflow, the constraints detailed in Table 6.1 were applied. The resulting reoperation outflow release value was calculated by the following formula:

$$\text{reoperated outflow} = \max(\text{reservoir fill limit}, \min(\text{ROI}, \text{reservoir release capacity}, \max(\text{ROD}, \text{base outflow})))$$

Reoperated storage was then calculated using the sum of the previous storage value, the historical daily change in storage and the difference in reoperated outflow from historical outflow over one day, converted to acre-feet.

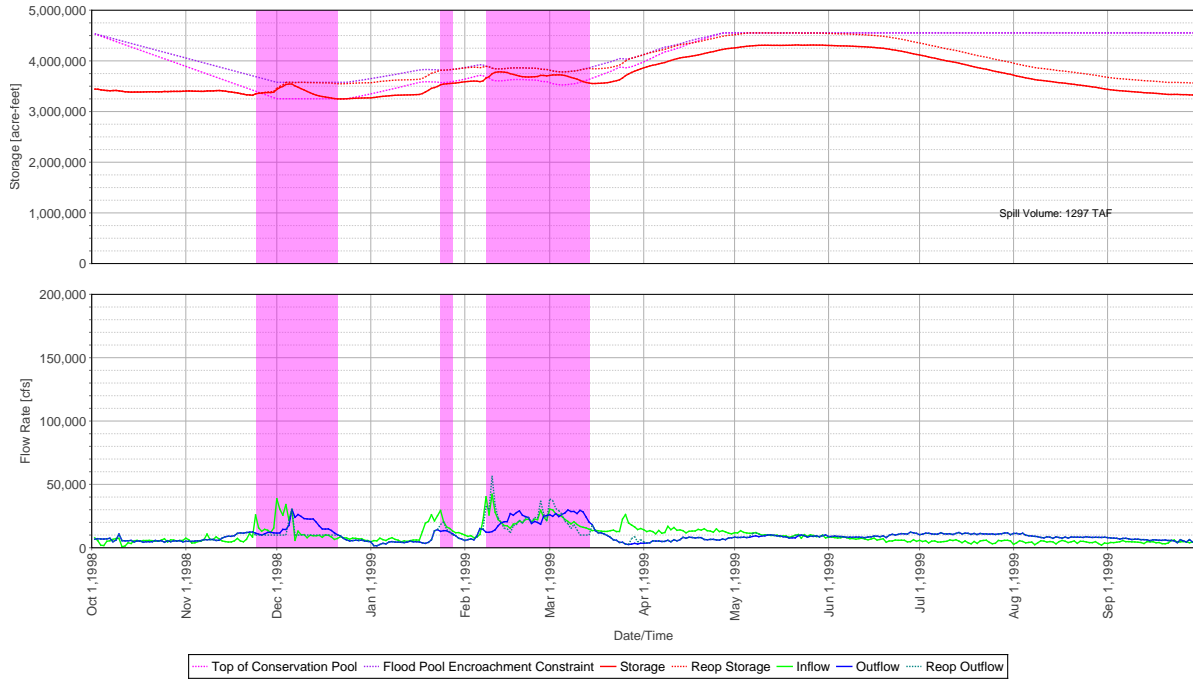
6.4 Results

For each water year, a plot detailing the daily storage and flow values was produced. An example plot for each reservoir is presented in the following section. Plots for the full period of record are included in Appendix A.

6.4.1 Reoperation Outflow and Storage

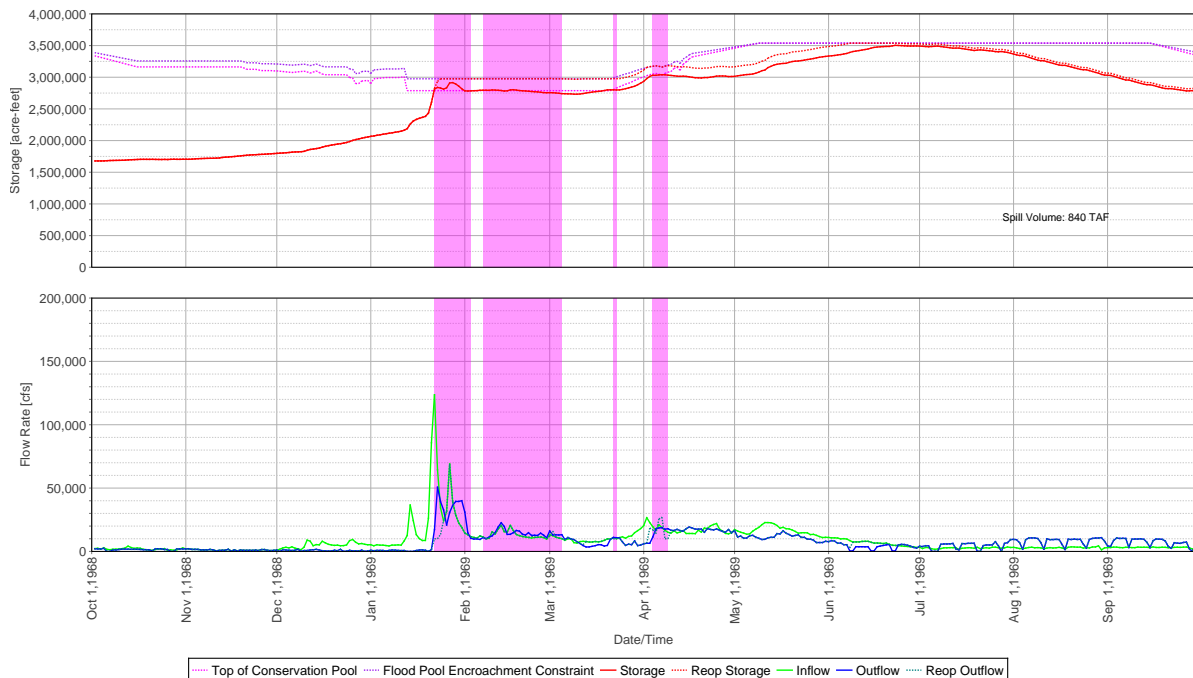
Figure 6.1 demonstrates how reoperation of Shasta Reservoir can enhance the water supply. The highlighted purple regions mark where historical outflow exceeds the threshold spill level. In December, the divergence of the historical and reoperated outflow resulted in a storage difference at Shasta that was maintained throughout the rest of the water year. Cutting the outflow to the threshold value allowed storage to encroach further into the flood control zone, though not higher than one-quarter of the available flood space, as per the constraints detailed in Section 6.2.

Figure 6.1. Example Reservoir Operations Plot for Shasta (1999)



In Figure 6.2, Oroville’s reoperated outflow is rarely reduced to the threshold value due to the flood space encroachment constraint which is graphically depicted as the modified top-con curve. As such, once the reoperated storage curve meets the modified top-con, reoperated outflow is held at inflow, so that the storage volume can be maintained.

Figure 6.2. Example Reservoir Operations Plot for Oroville (1969)



6: Water Supply Enhancement

Additional storage in New Bullards Bar Reservoir due to reoperation is modest, as seen in Figure 6.3. The reoperation potential is limited, as outflow releases from New Bullards Bar are generally small and do not exceed the selected threshold spill value. By allowing slight encroachment into the flood pool, reoperated storage reaches a higher value than the historical pattern. The highlighted orange zone in this plot designates a potential evacuation zone. However, since reoperation has not occurred prior to this point, outflow is maintained at the historical levels.

Figure 6.3. Example Reservoir Operations Plot for New Bullards Bar (1980)

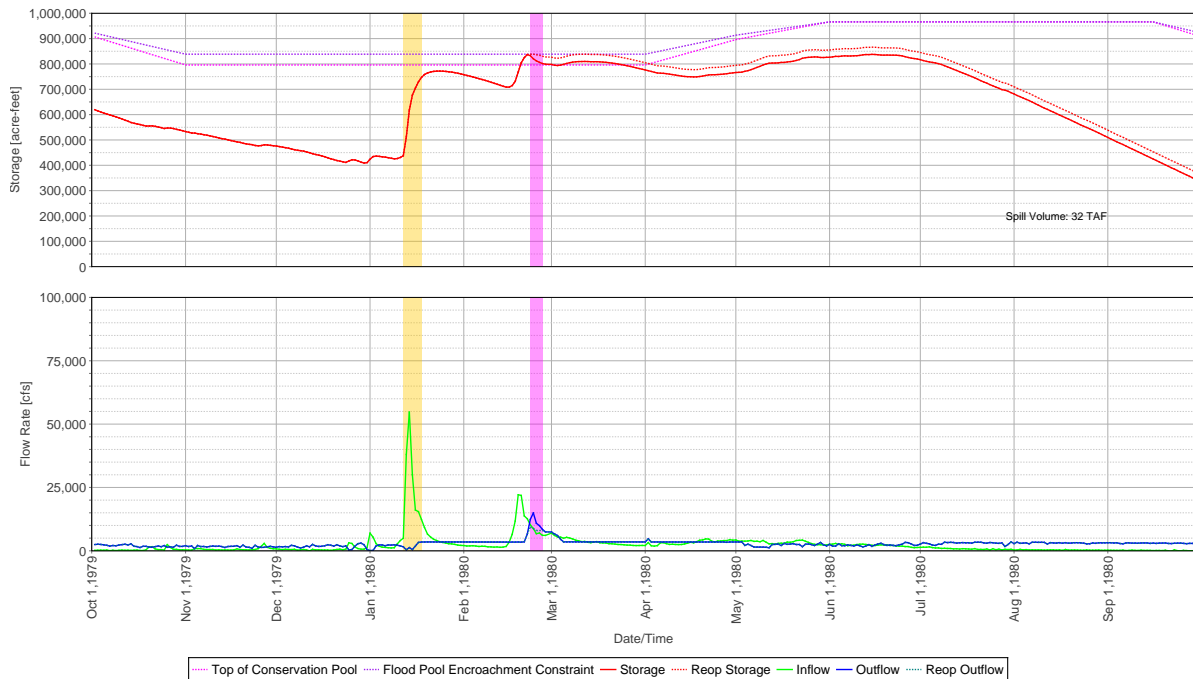
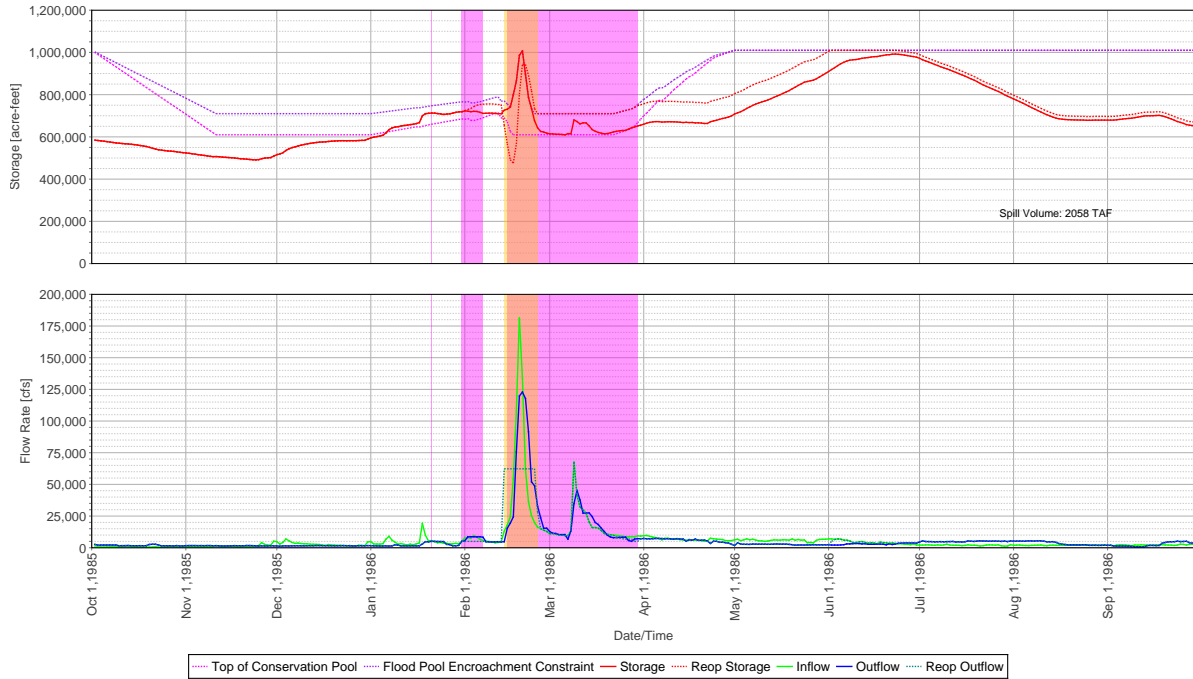


Figure 6.4 demonstrates how reoperation of Folsom Reservoir can result in increased water supply storage. As in the previous plots, the highlighted purple regions mark water supply enhancement zones, while the orange zone indicates evacuation of the flood space has been triggered. Note the flat outflow plateau in the orange zone, which causes storage to draw back down to top-con by the end of the evacuation window. Additionally, the reoperated storage curve reached gross pool nearly a month before the historical pattern. This fulfills the goal of additional water supply storage.

Figure 6.4. Example Reservoir Operations Plot for Folsom (1986)



6.4.2 Refill Potential

Table 6.3 shows the differences between historical and reoperated reservoir fill frequencies. The increased frequency of fill in the reoperation scenarios is a potential measure of their success.

Table 6.3. Summary of Reoperation Results

Water Year	Storage Increased by Reoperation?				Spring Refill?				Sacramento Valley Water Supply	
	SHA	ORO	NBB	FOL	SHA	ORO	NBB	FOL	Index[MAF]	Year-Type
1954					■				8.51	AN
1955									6.14	D
1956					■			■	11.38	W
1957					■			■	7.83	AN
1958					■			■	12.16	W
1959	▲								6.75	BN
1960								■	6.20	D
1961	▲								5.68	D
1962				▲				□	6.65	BN

▲ Maximum reservoir storage increased by reoperation.

□ Spring refill occurs only with reoperation.

■ Spring refill occurs historically.

● Shasta(SHA) ● Oroville(ORO) ● New Bullards Bar(NBB) ● Folsom(FOL)

6: Water Supply Enhancement

Water Year	Storage Increased by Reoperation?				Spring Refill?				Sacramento Valley Water Supply	
	SHA	ORO	NBB	FOL	SHA	ORO	NBB	FOL	Index[MAF]	Year-Type
1963					■			■	9.63	W
1964	▲							■	6.41	D
1965					■			■	10.15	W
1966					■				7.16	BN
1967					■			■	10.20	W
1968	▲			▲					7.24	BN
1969				▲	■	■		□	11.05	W
1970	▲	▲		▲				□	10.40	W
1971					■	■	■	■	10.37	W
1972	▲							■	7.29	BN
1973	▲				□	■		■	8.58	AN
1974					■	■	■	■	12.99	W
1975				▲	■	■	■	□	9.35	W
1976									5.29	C
1977									3.11	C
1978		▲		▲	■	□		□	8.65	AN
1979						■		■	6.67	BN
1980	▲	▲	▲	▲		□			9.04	AN
1981	▲								6.21	D
1982					■	■	■	■	12.76	W
1983					■	■	■	■	15.29	W
1984	▲	▲				□	■	■	10.00	W
1985	▲			▲					6.47	D
1986		▲					■	■	9.96	W
1987									5.86	D
1988									4.65	C
1989				▲			■		6.13	D
1990									4.81	C
1991									4.21	C
1992									4.06	C
1993					■	■	■	■	8.54	AN
1994									5.02	C
1995	▲					■	■	■	12.89	W
1996					■	■	■	■	10.26	W
1997		▲	▲	▲					10.82	W

▲ Maximum reservoir storage increased by reoperation.

□ Spring refill occurs only with reoperation.

■ Spring refill occurs historically.

 Shasta(SHA)
  Oroville(ORO)
  New Bullards Bar(NBB)
  Folsom(FOL)

6: Water Supply Enhancement

Water Year	Storage Increased by Reoperation?				Spring Refill?				Sacramento Valley Water Supply	
	SHA	ORO	NBB	FOL	SHA	ORO	NBB	FOL	Index[MAF]	Year-Type
1998					■	■	■	■	13.31	W
1999	▲	▲	▲		□	□		■	9.80	W
2000	▲	▲		▲					8.94	AN
2001									5.76	D
2002									6.35	D
2003					■	■	■	■	8.21	AN
2004	▲	▲		▲					7.51	BN
2005			▲		■	■	□	■	8.49	AN
2006			▲		■	■		■	13.2	W
2007		▲							6.19	D
2008									5.16	C
2009									5.78	D
2010					■		■	■	7.08	BN
Total	15	10	5	12	24 ^a 22 ^b	19 ^a 15 ^b	15 ^a 14 ^b	31 ^a 26 ^b		

^a Total years with spring refill under reoperated and historical conditions.

^b Total years with spring refill under historical conditions.

▲ Maximum reservoir storage increased by reoperation.

□ Spring refill occurs only with reoperation.

■ Spring refill occurs historically.

● Shasta (SHA) ● Oroville (ORO) ● New Bullards Bar (NBB) ● Folsom (FOL)

6.4.3 Monthly Storage Effects

In order to track the success of the reoperation routine at water supply enhancement, the probabilities of exceedance for storage at various times during the year were plotted and presented in Figure 6.5 through Figure 6.12. These plots detail the exceedance probabilities for the points of maximum storage in the reservoirs compared to the maximum historical storage. The end-of-month plots draw comparisons for historical and reoperated storage levels at the end of March, April and May of each year in the study.

Figure 6.5. Exceedance Plots for Spring Refill Storage Values at Folsom

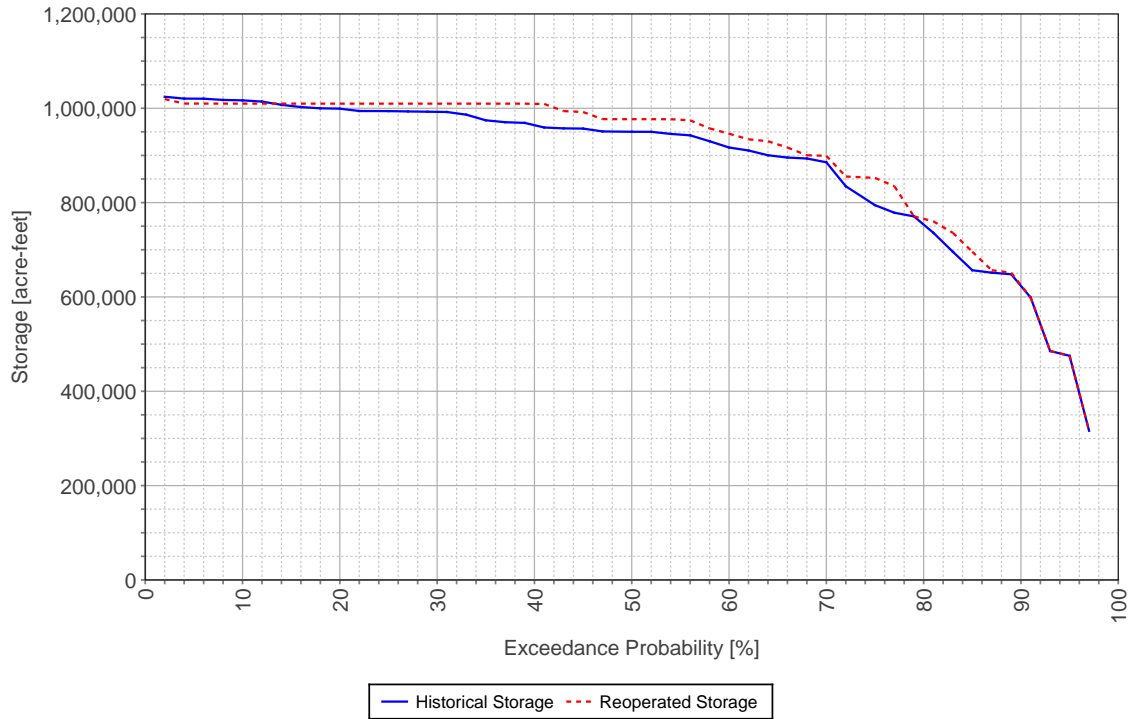


Figure 6.6. Exceedance Plots for Spring Refill Storage Values at New Bullards Bar

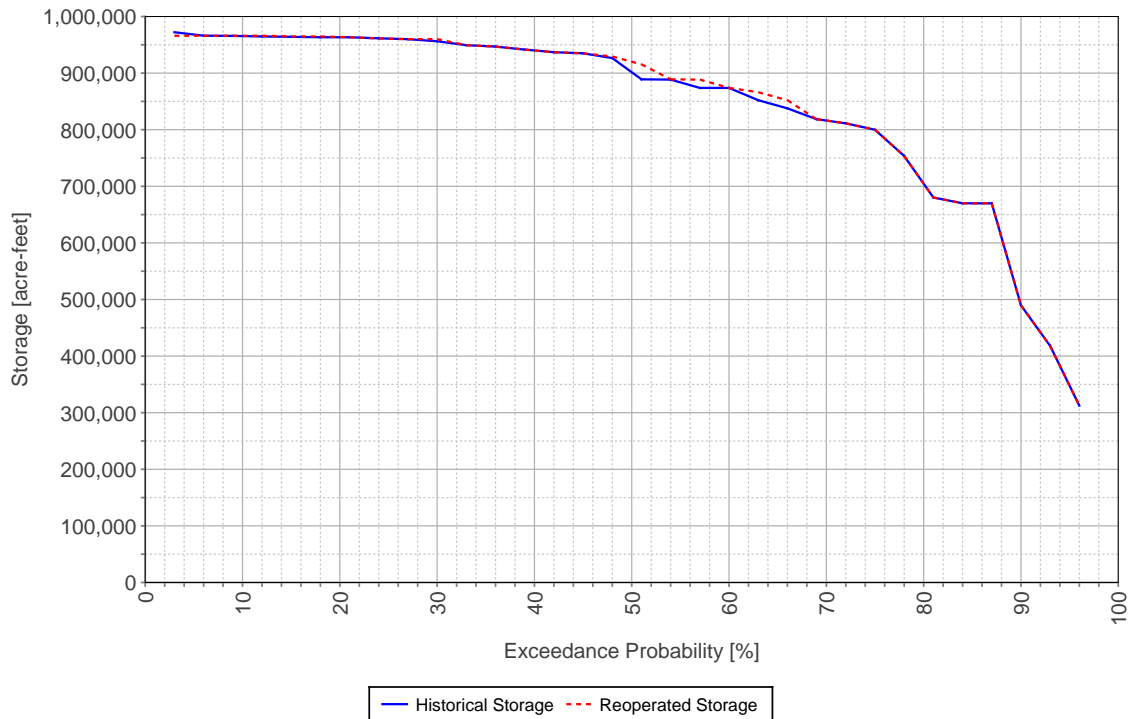


Figure 6.7. Exceedance Plots for Spring Refill Storage Values at Oroville

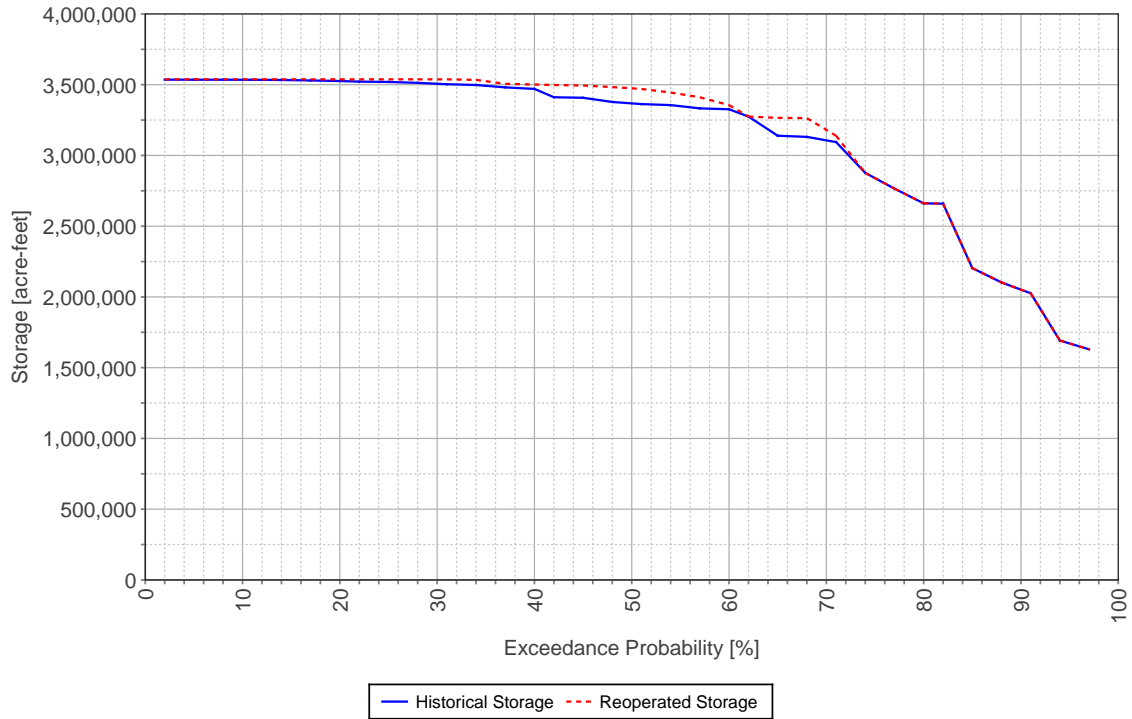


Figure 6.8. Exceedance Plots for Spring Refill Storage Values at Shasta

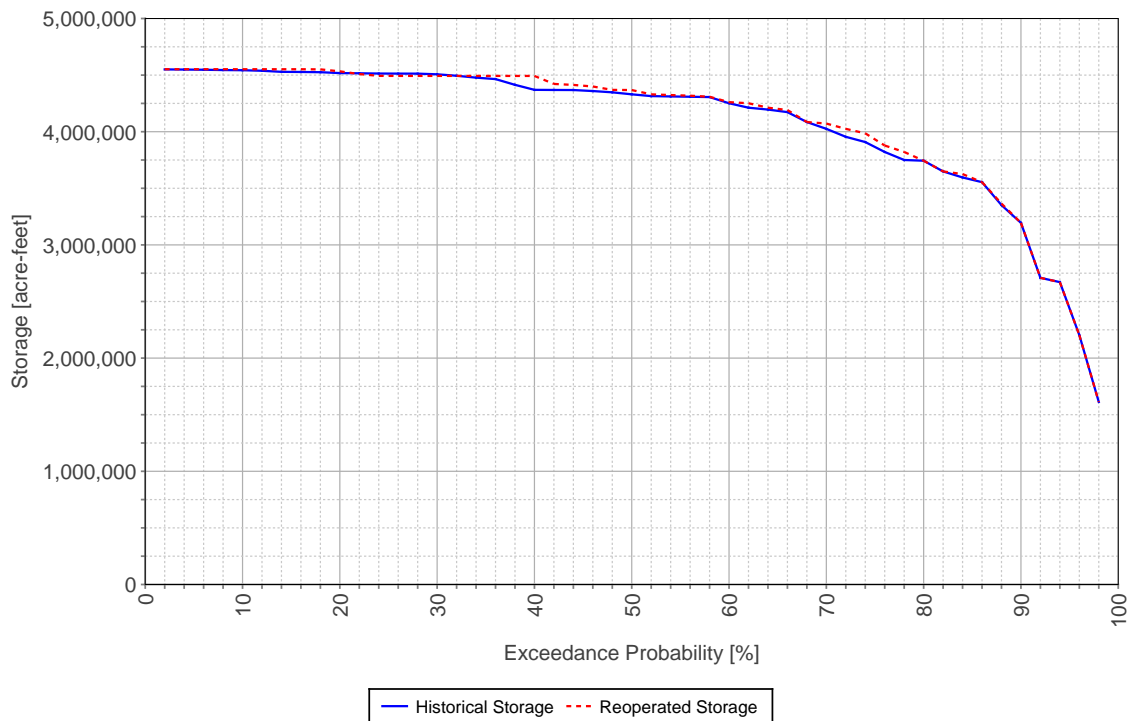


Figure 6.9. Exceedance Plots for End-of-Month Refill Storage Values at Folsom

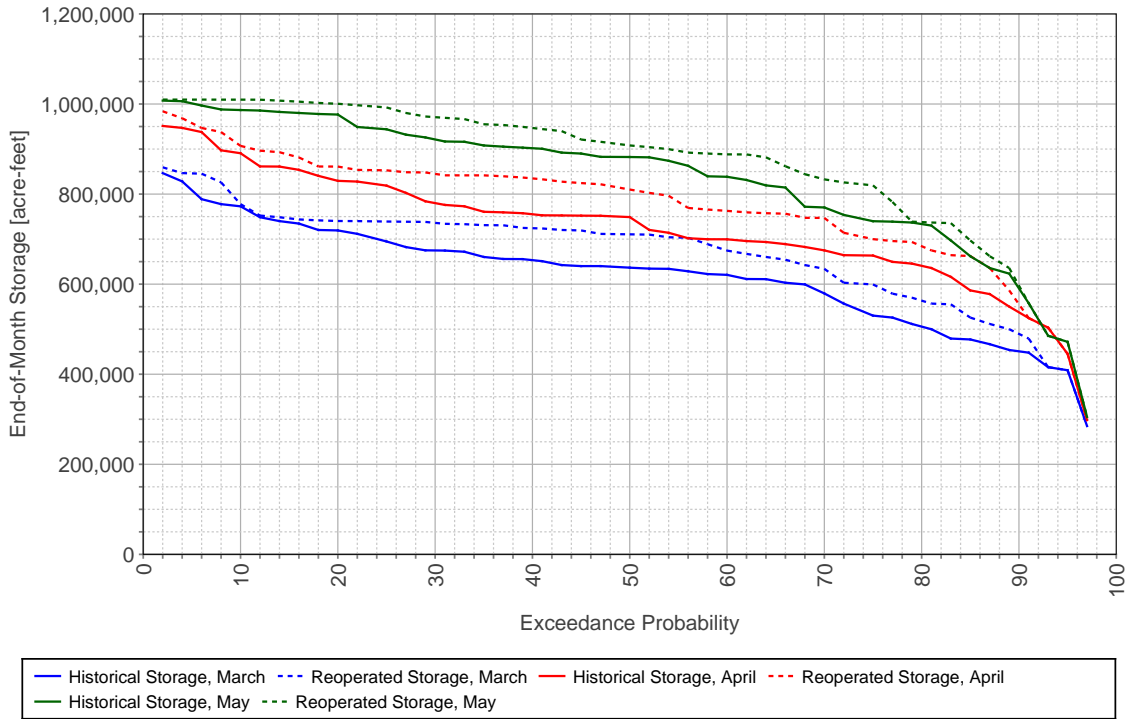


Figure 6.10. Exceedance Plots for End-of-Month Storage Values at New Bullards Bar

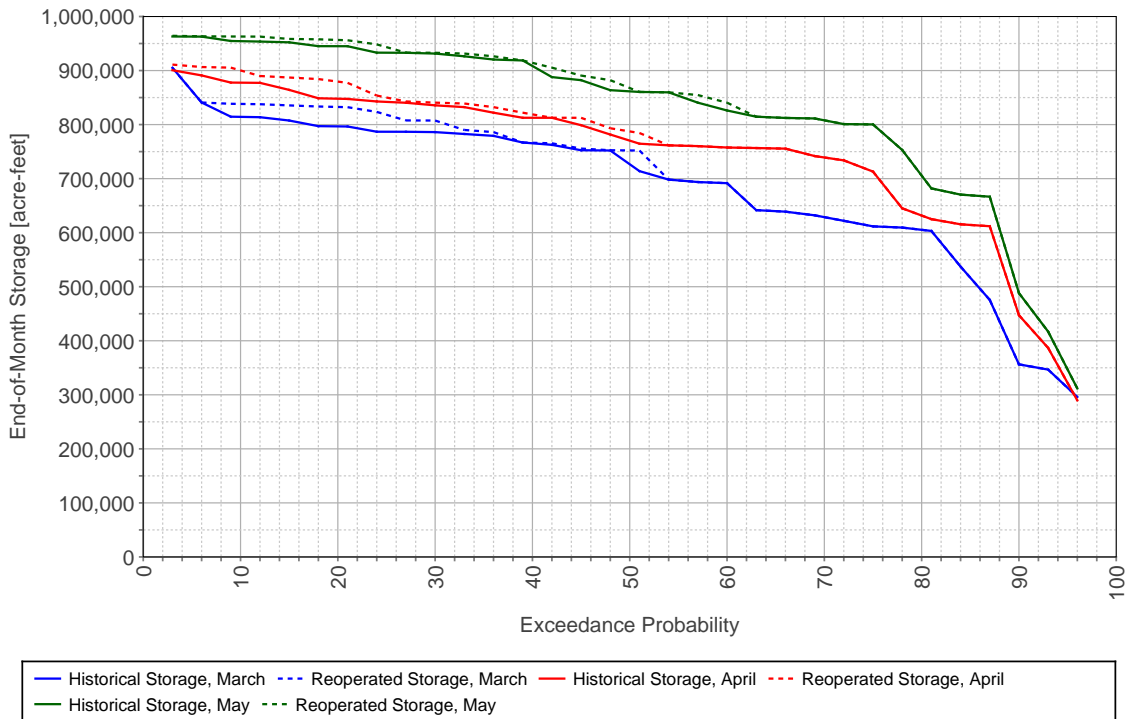


Figure 6.11. Exceedance Plots for End-of-Month Storage Values at Oroville

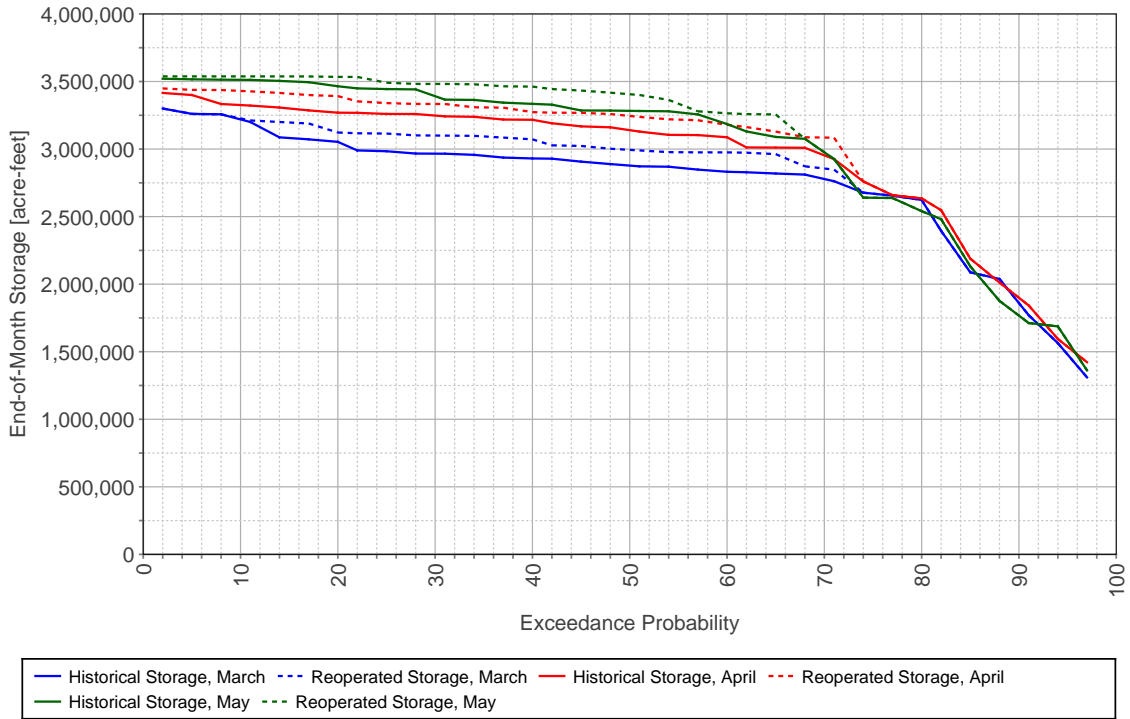
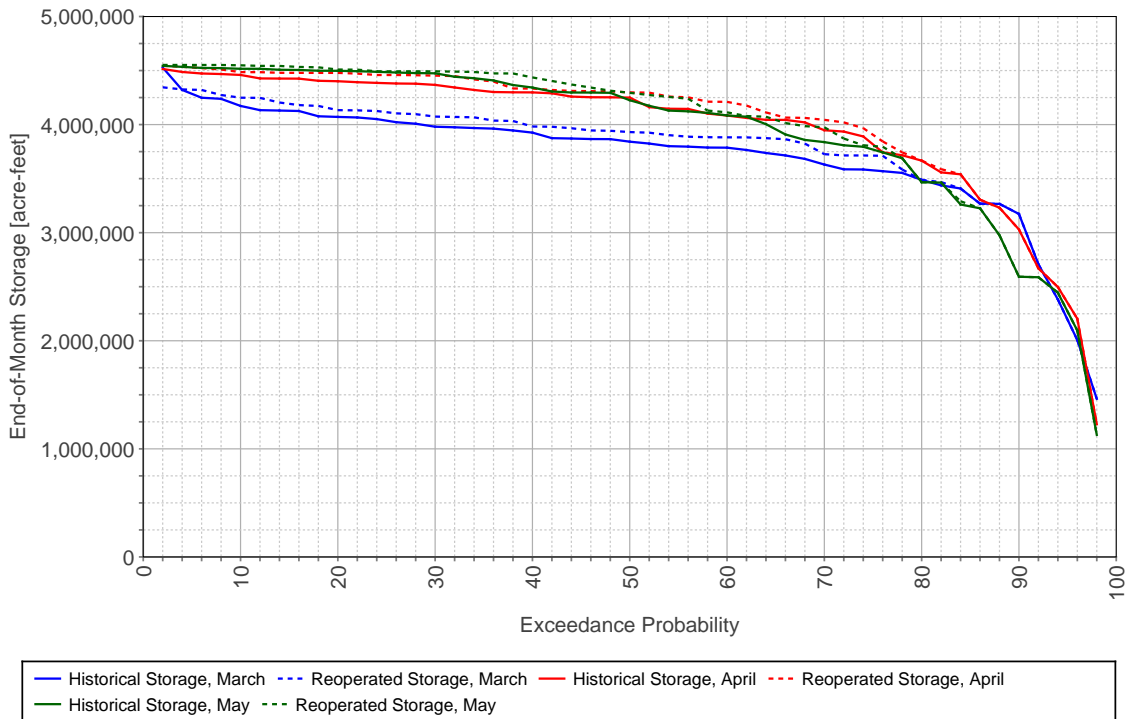


Figure 6.12. Exceedance Plots for End-of-Month Storage Values at Shasta



6.5 Evaluation of Benefits

The previously presented summary of results demonstrated the storage changes made possible by using F-BO for water supply enhancement. In addition, the systemwide benefits of utilizing this enhanced water supply were evaluated using CalLite. Appendix B details how the daily reoperation methodology was approximated in this model and shows the effects of operation with current rules as well as the enhanced water supply made available through reoperation.

6.6 Recommendations for Further Study

The operational modifications suggested in this chapter show there is potential to increase water supply yield through system reoperation. The reoperation parameters selected for this study were developed with careful consideration of the factors that would exist when making such a decision (e.g. impending flood risk). These parameters will require fine-tuning as the procedure to enhance water supply through system reoperation becomes more advanced. Another component of the reoperation that warrants further study is the relationship between the system reservoirs. For the study described herein, all the reservoirs reoperated independently. Pursuing a more comprehensive coordination in reoperation of the reservoirs may yield significant gains. This should be considered in any additional study performed.

7

Flood Control Enhancement

Just as F-BO was utilized to enhance water supply, F-BO can be used in a slightly different framework to enhance flood control objectives at each reservoir. Utilizing available inflow forecasts for releasing water ahead of a main flood wave offers the opportunity to create additional flood space to manage an impending flood.

As part of this assessment, historical reservoir storage and river flow data was reviewed and compared with flood rules and space requirements to determine when additional flood control storage or modification of flood rule parameters would have been beneficial to enhance flood control. Factors potentially limiting F-BO effectiveness were identified at each reservoir (e.g., limited outlet capacity or insufficient flood space). A comparison of each reservoir’s potential use of F-BO to enhance flood control was developed. This comparison, shown in Table 7.1, identifies each reservoir’s potential in each of several areas (flood space, outlet capacity, and channel capacity).

Table 7.1. Evaluation of Watershed Flood Control System Components with Regard to Ability to Implement F-BO

Reservoir	Flood Space	Outlet Capacity	Channel Capacity
Shasta	Adequate	Adequate	Limited
Oroville	Adequate	Adequate	Adequate
New Bullards Bar	Adequate	Limited	Adequate
Folsom	Adequate	Adequate*	Adequate

* denotes adequate outlet capacity will be available when Folsom Joint-Federal Project is completed in 2017.

7.1 Shasta

Shasta’s flood control rules limit reservoir releases such that flows do not exceed 79,000 cfs below Keswick Dam or 100,000 cfs in the Sacramento River at Bend Bridge. It is recommended that the appropriateness of the Sacramento River at Bend Bridge flow target of 100,000 cfs be assessed. The Bend Bridge target significantly limits Shasta’s ability to release water during a flood event due to the significant tributary inflows to the Sacramento River between Shasta and Bend Bridge. The tributary flows have the potential to approach or reach 100,000 cfs themselves, thus causing Shasta to severely limit releases or stop releasing water altogether.

In comparison to other flow targets on similar systems, the Bend Bridge target seems excessively low. On average, it is exceeded approximately every 5 years. If this flow target were raised, the additional volume of water Shasta could release during a flood event would significantly increase. This would result in extra flood storage capacity during the main flood event which would then likely contribute to a reduction in the peak flood release necessary for these events. It is anticipated that raising the Bend Bridge flow target to 125,000 or 150,000 cfs would significantly affect events in the 1-in-100 to 1-in-200 year frequency range. It is recommended the Bend Bridge flow target be assessed. If

the investigation of this target shows that there is little reason (e.g., damageable property) above 100,000 cfs, it is suggested that a higher flow target be considered.

7.2 Oroville

Oroville Dam coordinates flood release with New Bullards Bar on the North Yuba River. This coordination is mandated in the water control manuals for the two reservoirs. As such, any flood control improvements that can be made to Oroville also have the potential to provide flood control benefits at New Bullards Bar. For example: if Oroville makes a forecast-based release of water in advance of a flood event, additional flood space is provided in Oroville. This otherwise unavailable flood space could be used to do any of the following:

- manage a larger flood than otherwise manageable
- reduced release rates and lower Feather River flows
- reduced release rates and allow New Bullards Bar to perform comparable increase in release rates, thus reducing its flood management burden.

Oroville has adequate flood space, reservoir release capacity, and downstream channel capacity to accommodate F-BO for flood control improvements.

7.3 New Bullards Bar

As mentioned in Section 7.2, New Bullards Bar and Oroville coordinate flood control releases. As such, much of the discussion on the use of F-BO for flood control at Oroville Dam pertains to New Bullards Bar. One constraint for potential application of F-BO at New Bullards Bar is the limited reservoir outlet capacity compared to outlet capacity at Oroville. This limited outlet capacity at New Bullards Bar inhibits the effectiveness of a pre-event flood release, since a significant volume of water is not able to be released within the applicable time window (e.g., 3-5 days ahead of peak). It is thus suggested that improvements to the outlet capacity of New Bullards Bar be considered along with F-BO in the context of improving flood control.

7.4 Folsom

As part of the Joint-Federal Project (JFP), Folsom Dam is required to utilize forecasts in its updated operating rules when the project comes online in 2017. As part of that change, USACE is studying the ways forecasts can be used to enhance flood control.

Once the auxiliary spillway at Folsom is constructed, adequate release capacity will exist to implement F-BO for flood control benefits, e.g., a pre-event release. Folsom has no other operational constraints which significantly reduce the effectiveness of F-BO for use in flood control.

7.5 Recommendations for Further Study

While the flood control modifications suggested are promising for each individual reservoir, in order to warrant additional consideration, the systemwide impacts and benefits of the suggested measures

7: Flood Control Enhancement

must be better understood. It is recommended that additional investigation be performed for the elements deemed most promising from this study. A key consideration of the additional study should be how operational changes suggested for individual reservoirs affect flows and stages throughout the flood system.

References

MBK Engineers (2013). *System Reoperation Study: Merced River Analysis Technical Report*.

United States Army Corps of Engineers (1997). *Engineering Manual 1110-2-1420: Hydrologic Engineering Requirements for Reservoirs*.

United States Army Corps of Engineers, Sacramento District (SPK) (1987). *Folsom Dam and Lake, American River, California: Water Control Manual*.

Appendices

A

Reoperation Plots

This appendix details the plots that resulted from the daily reoperation routine that was performed in this analysis. Each plot contains the following timeseries:

- inflow
- outflow (baseline)
- outflow (reoperated)
- storage (baseline)
- storage (reoperated)

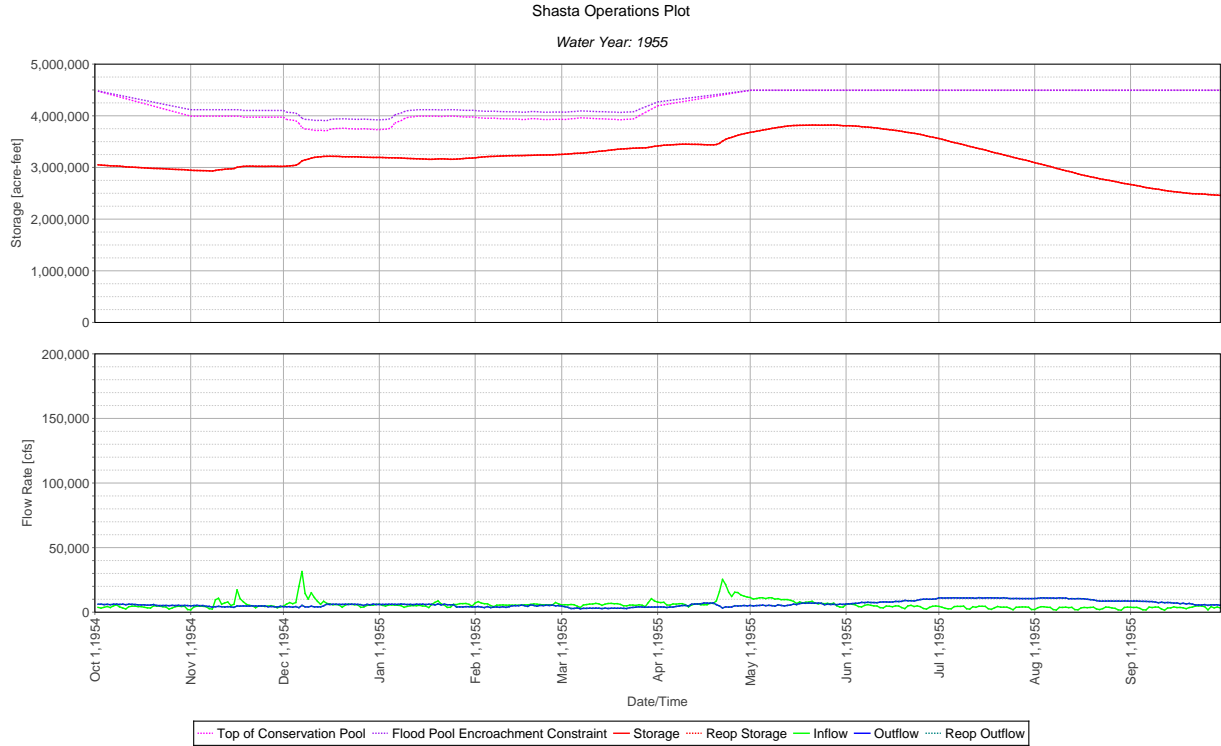
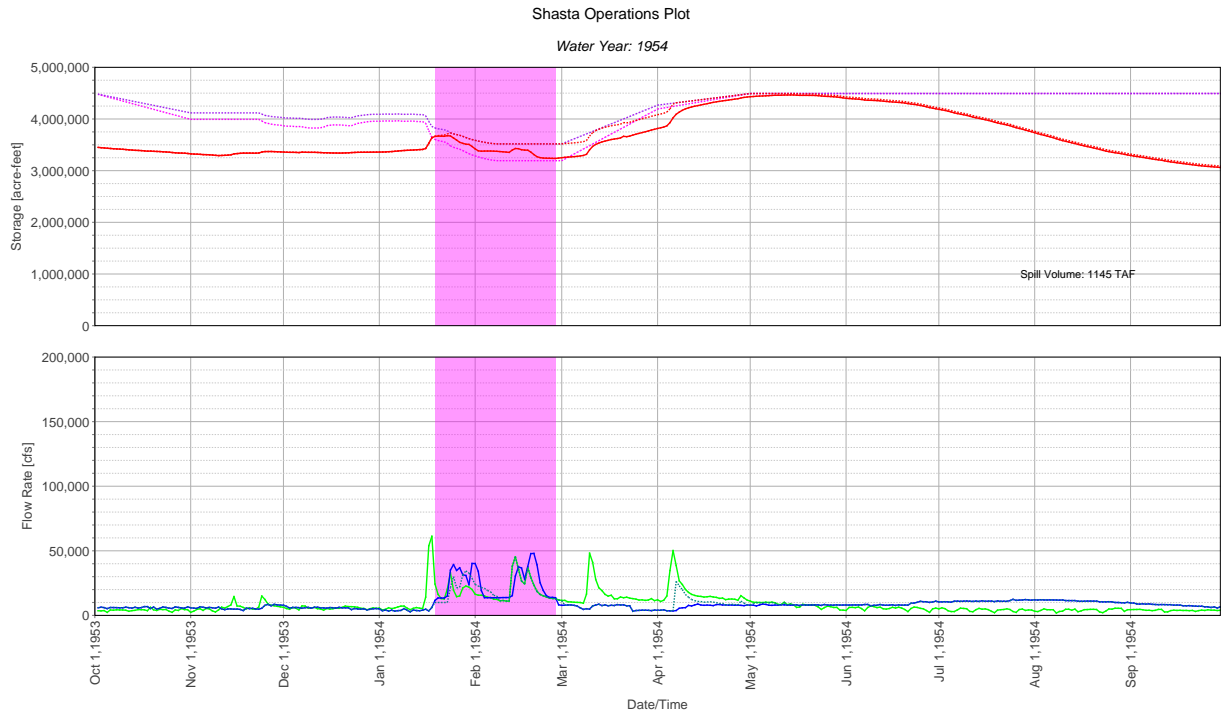
In addition the following regions are color-coded to indicate the different operational regimes:

- deactivation (orange)
- reoperation (purple)

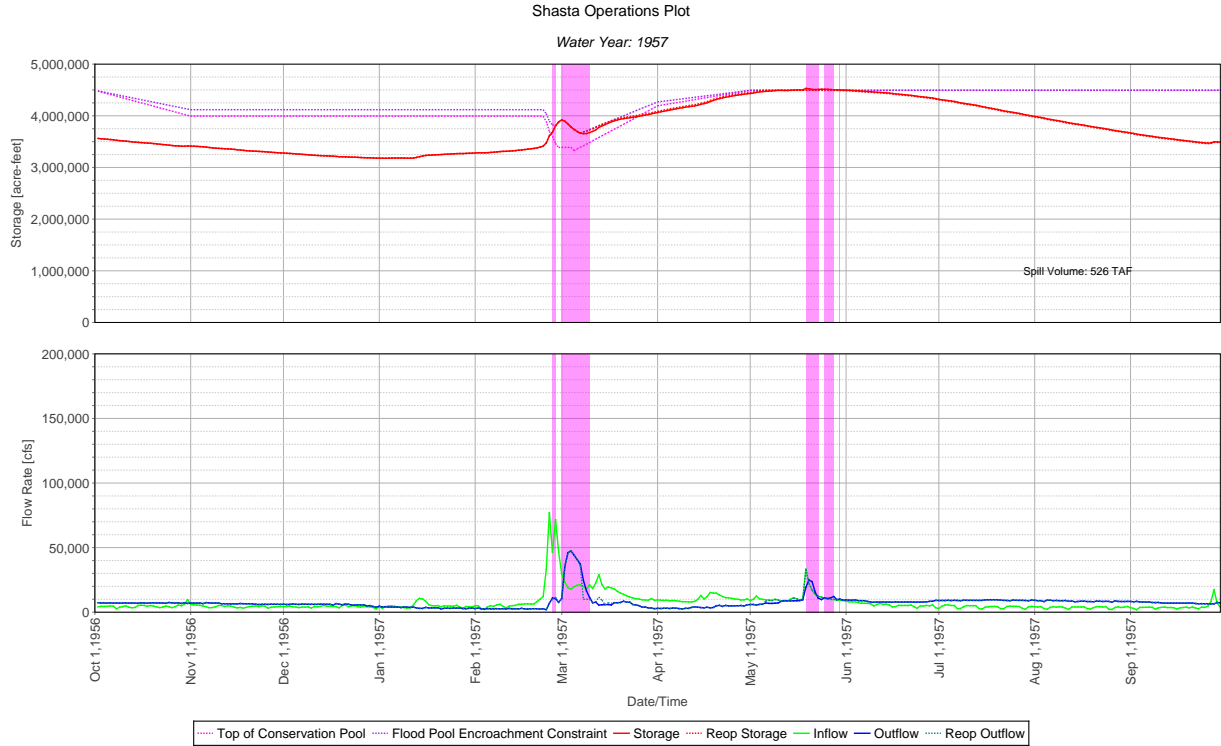
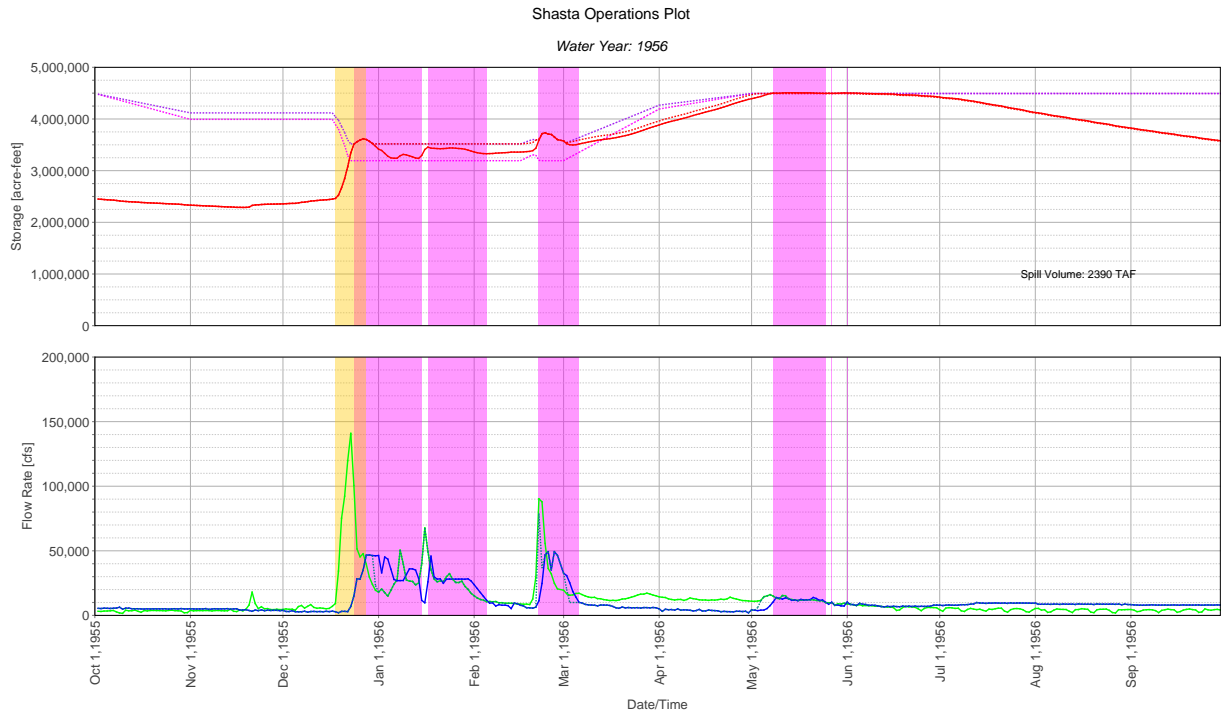
A.1 Shasta

The following section includes the reoperation plots for Shasta Dam.

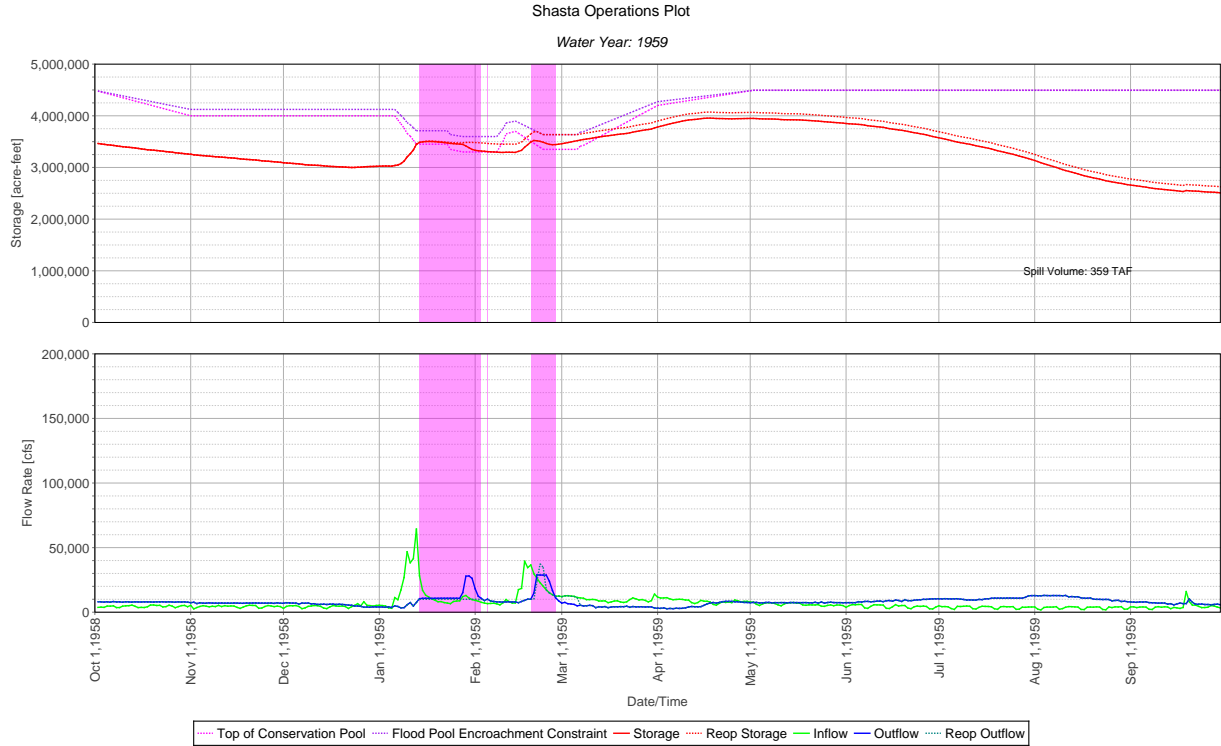
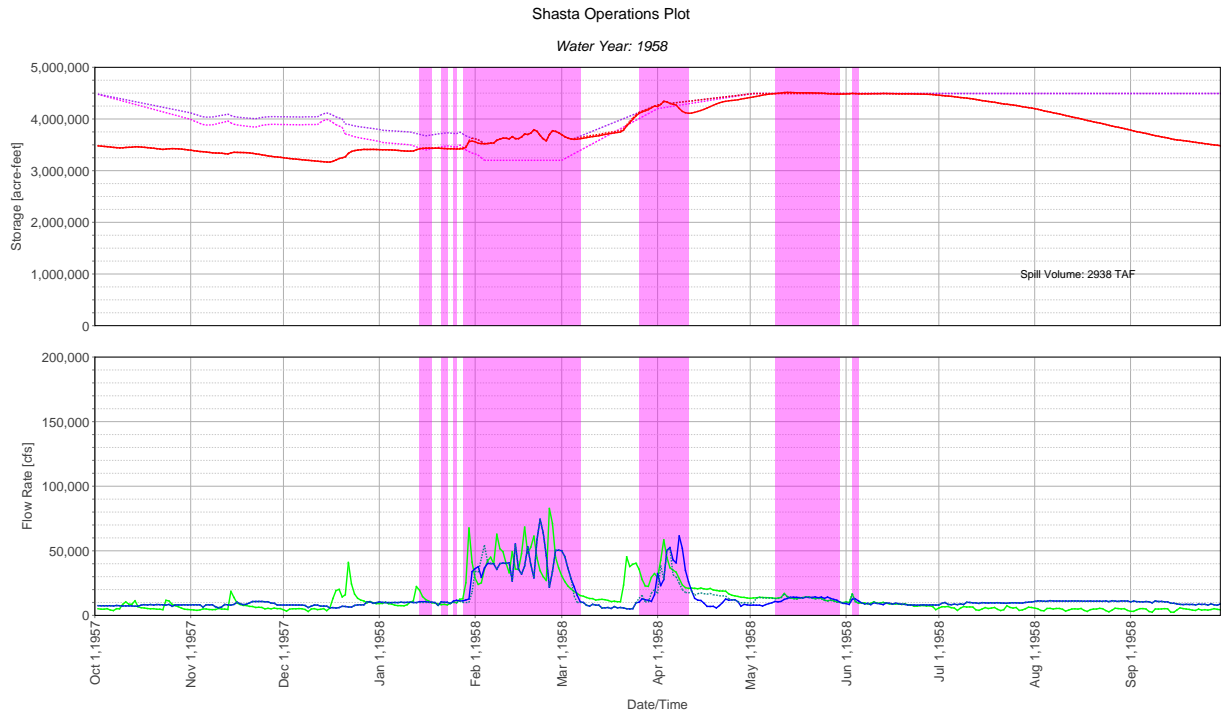
A: Reoperation Plots



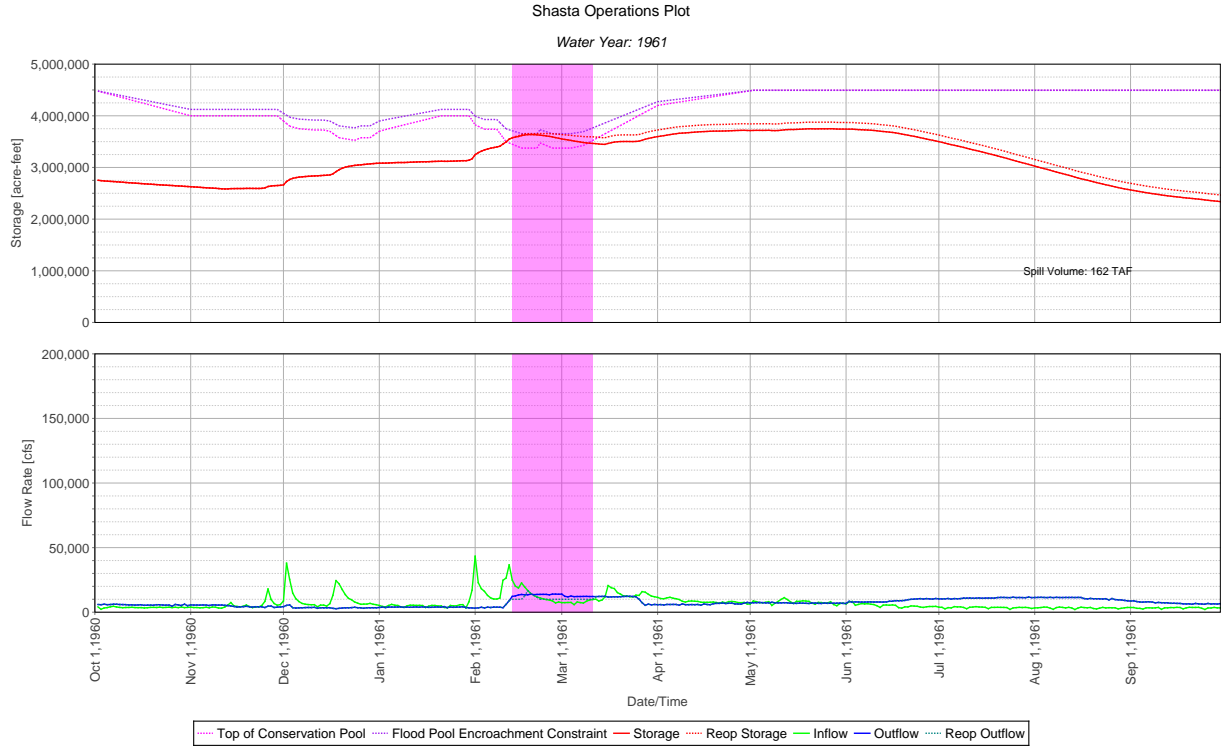
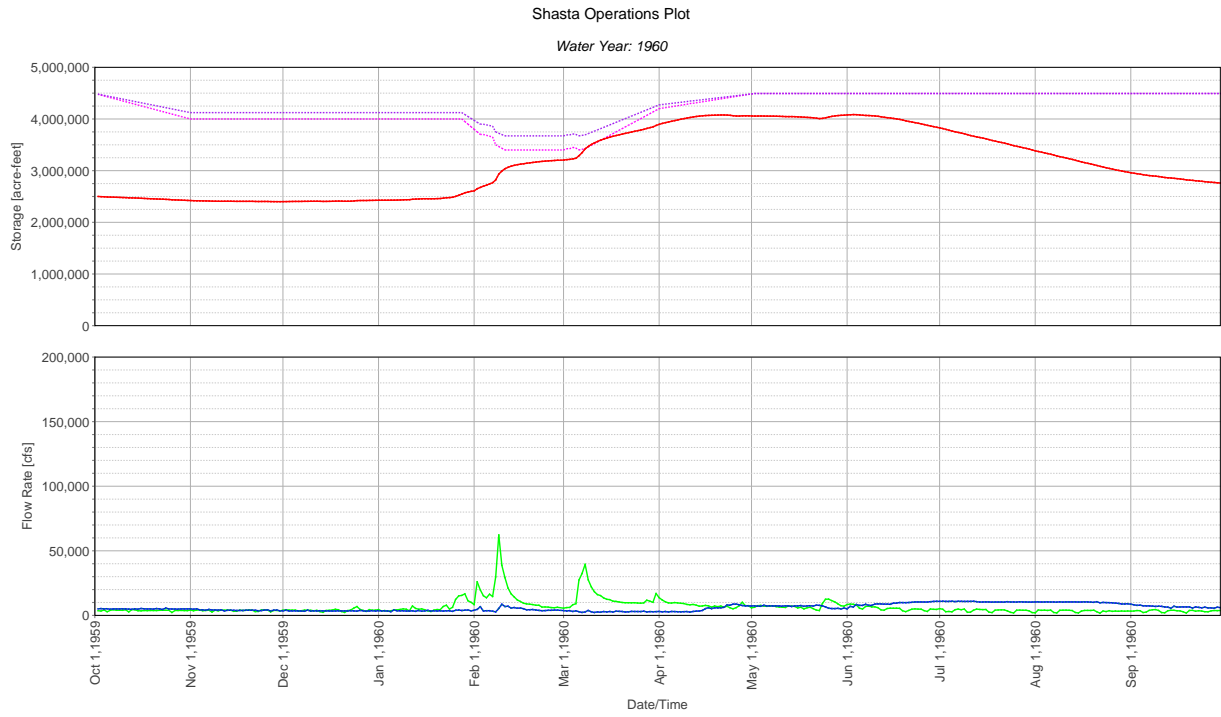
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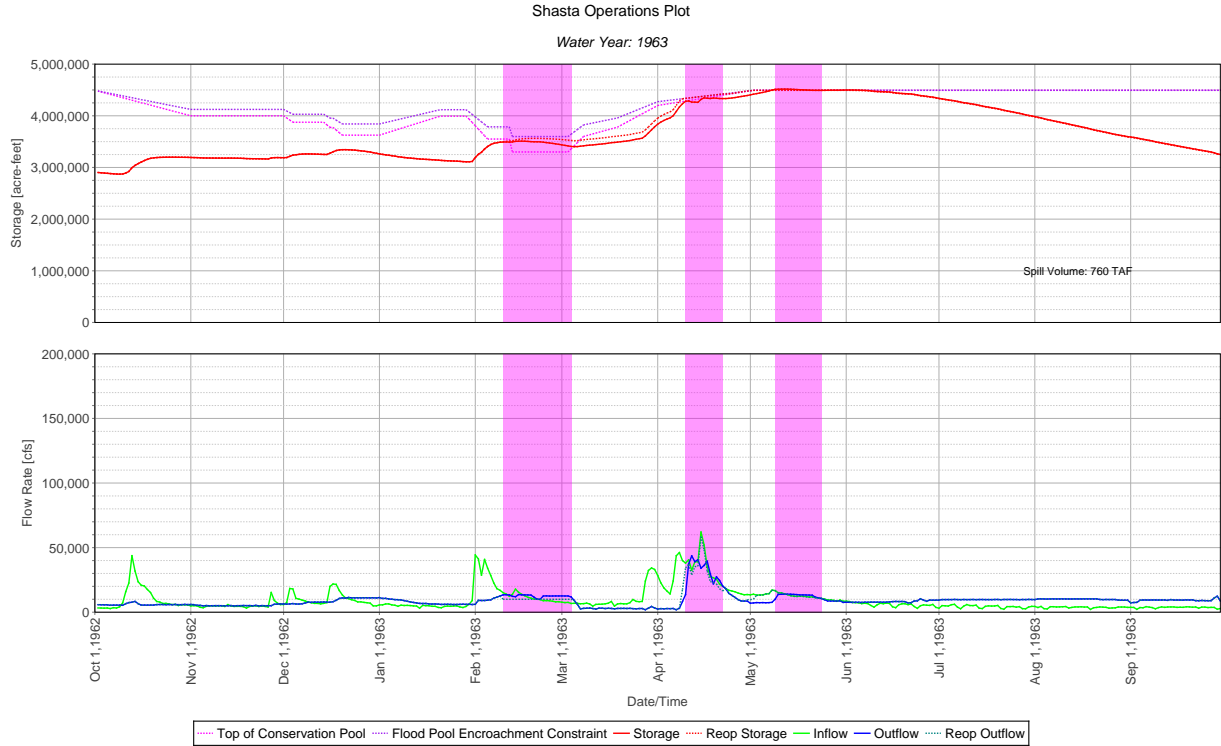
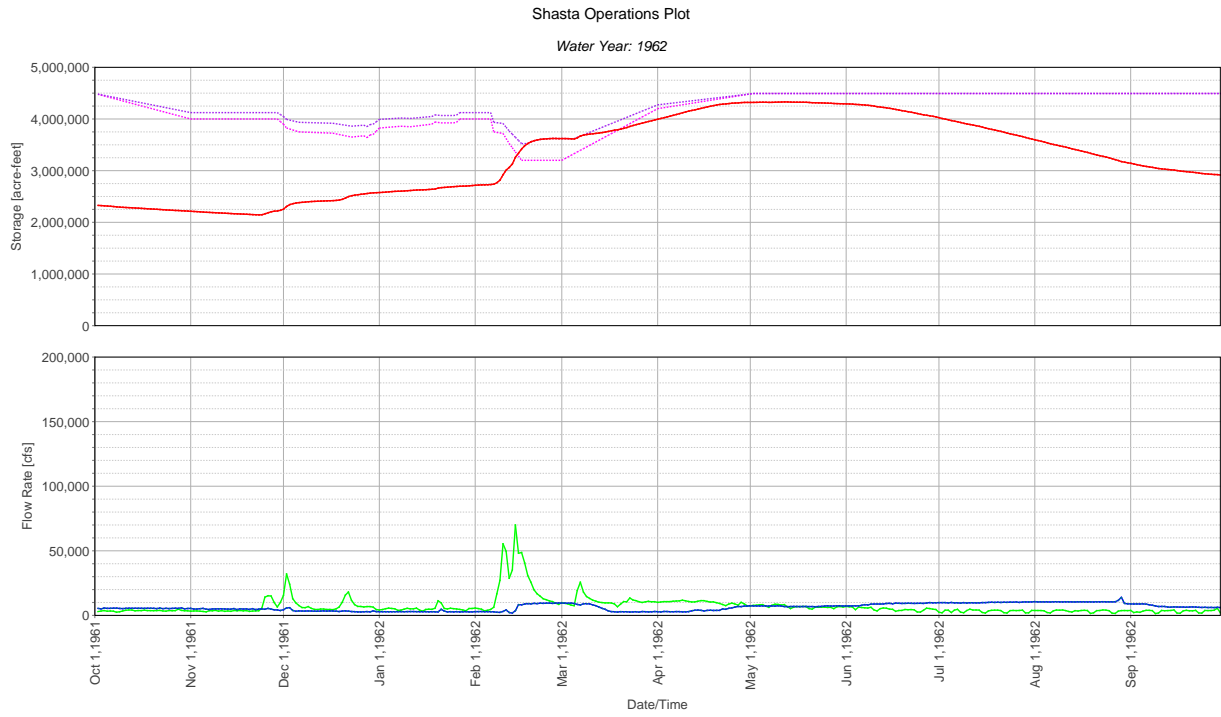
A: Reoperation Plots



A: Reoperation Plots



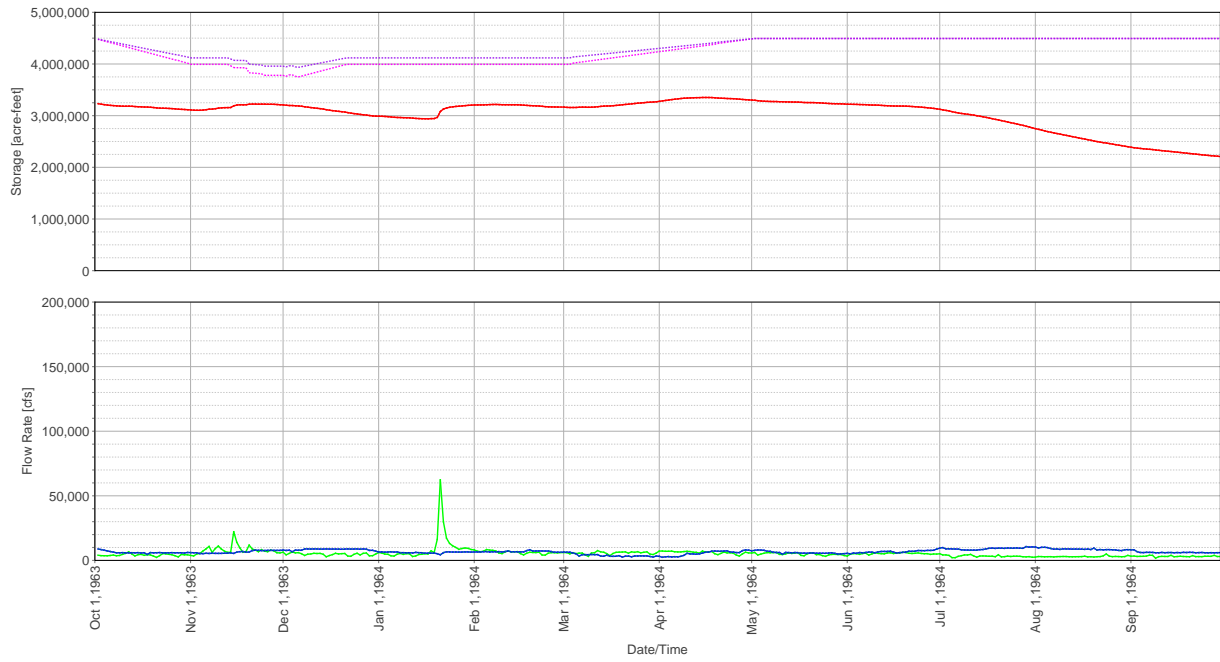
A: Reoperation Plots



A: Reoperation Plots

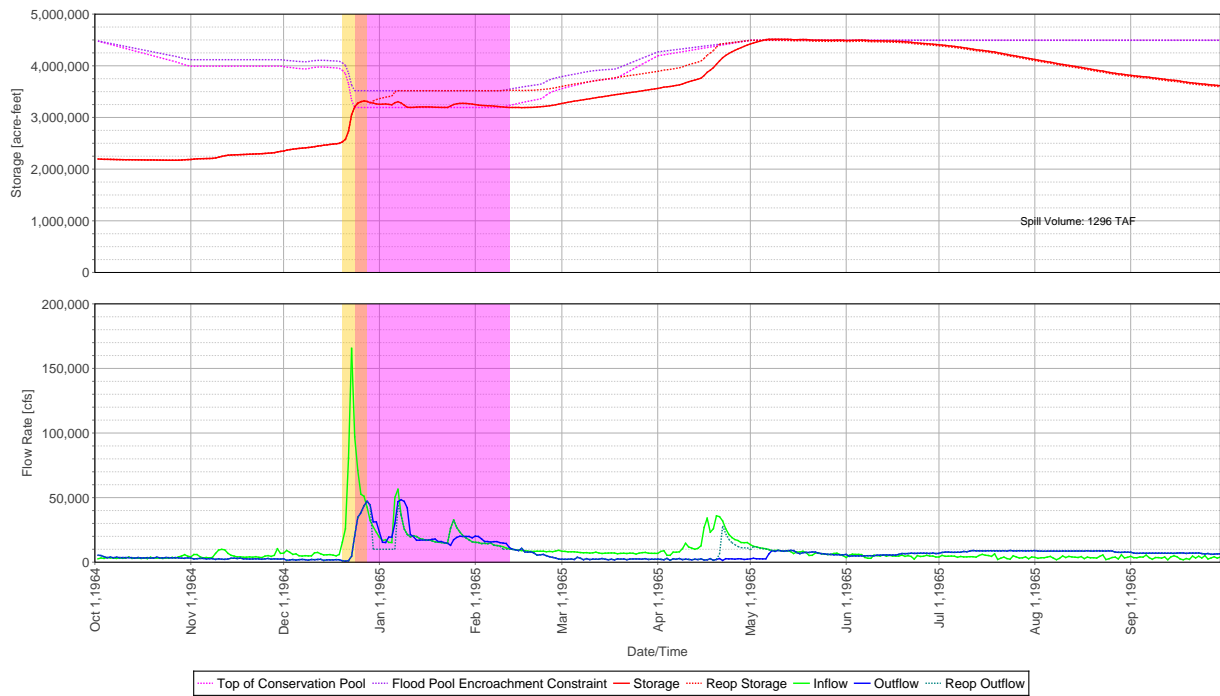
Shasta Operations Plot

Water Year: 1964



Shasta Operations Plot

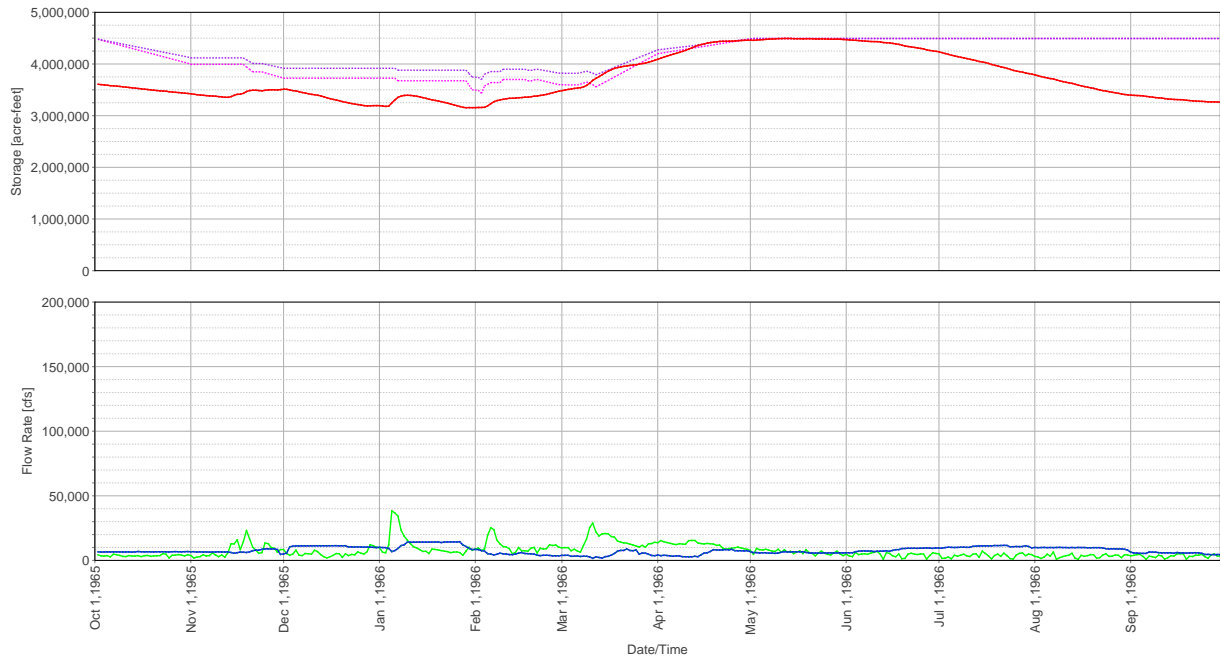
Water Year: 1965



A: Reoperation Plots

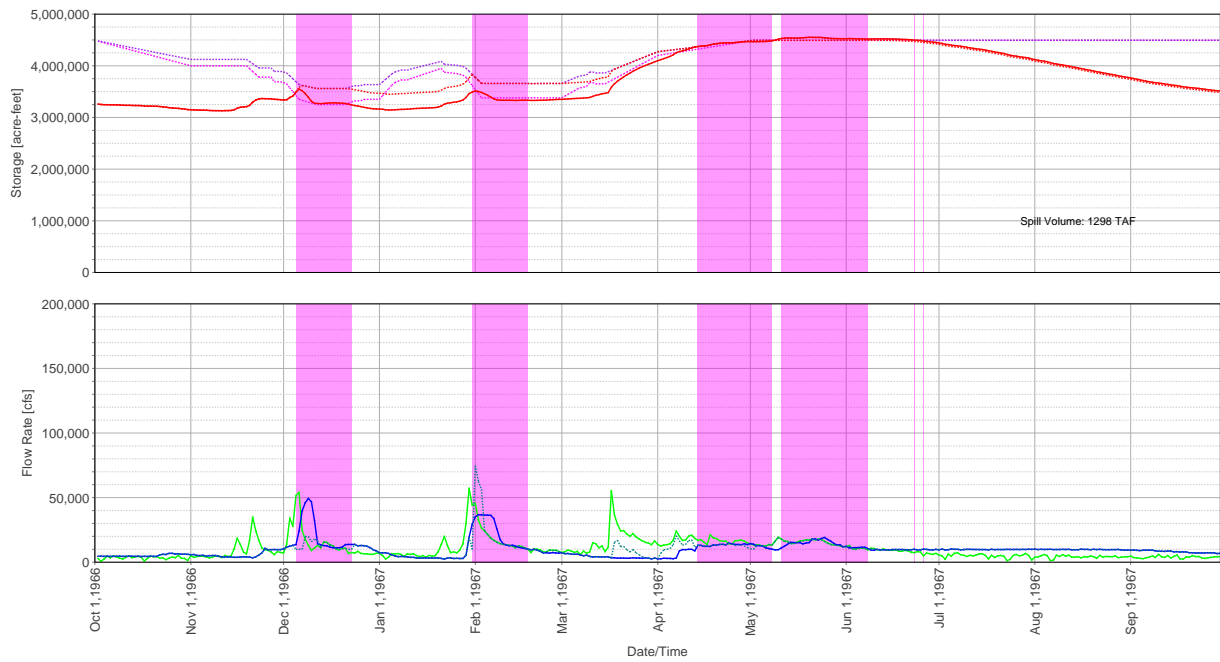
Shasta Operations Plot

Water Year: 1966



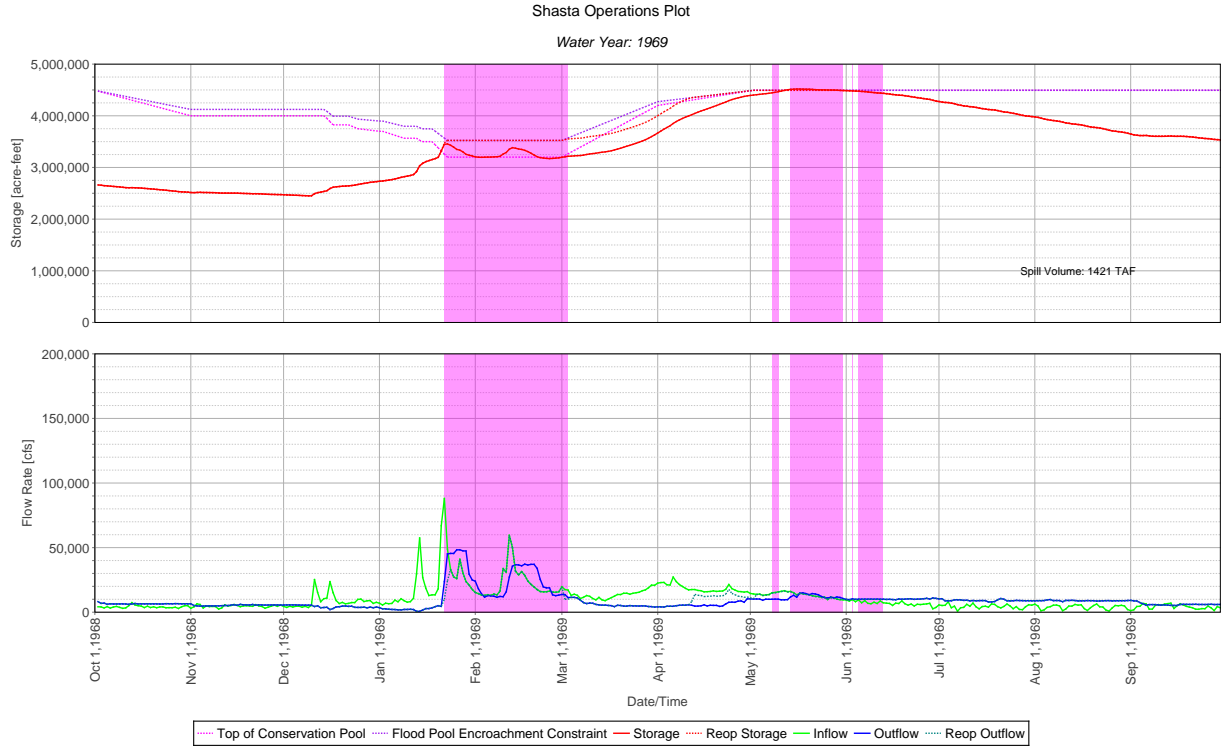
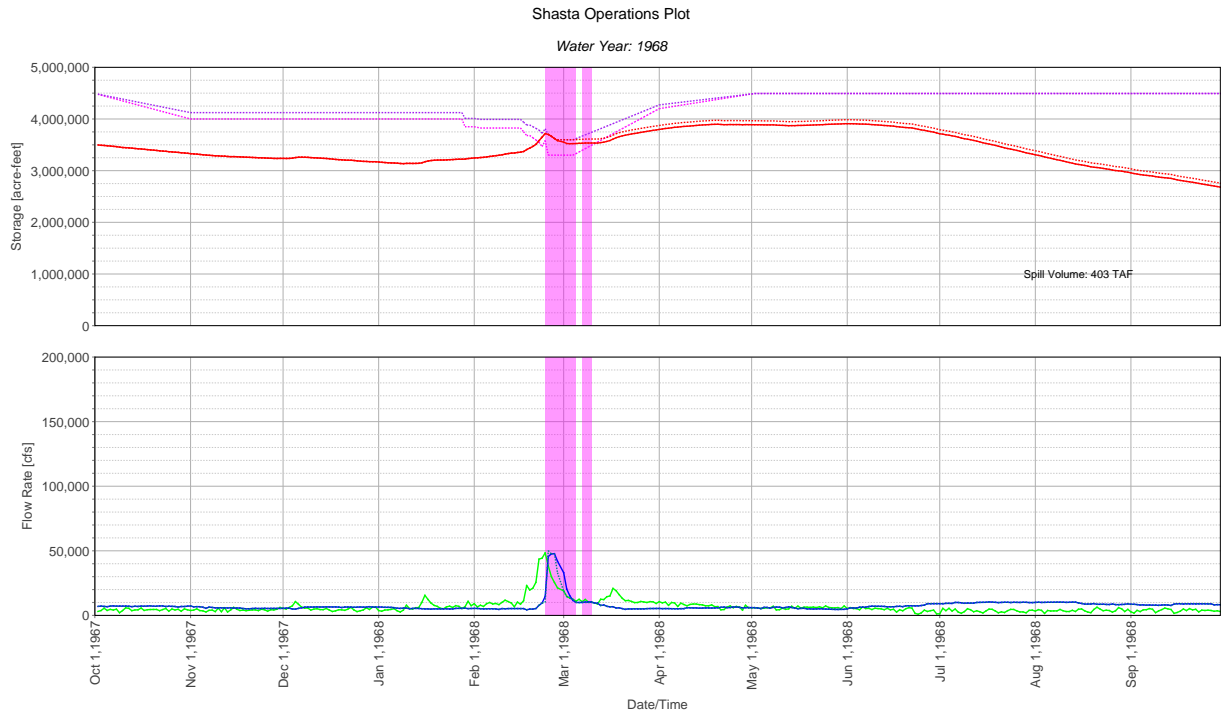
Shasta Operations Plot

Water Year: 1967

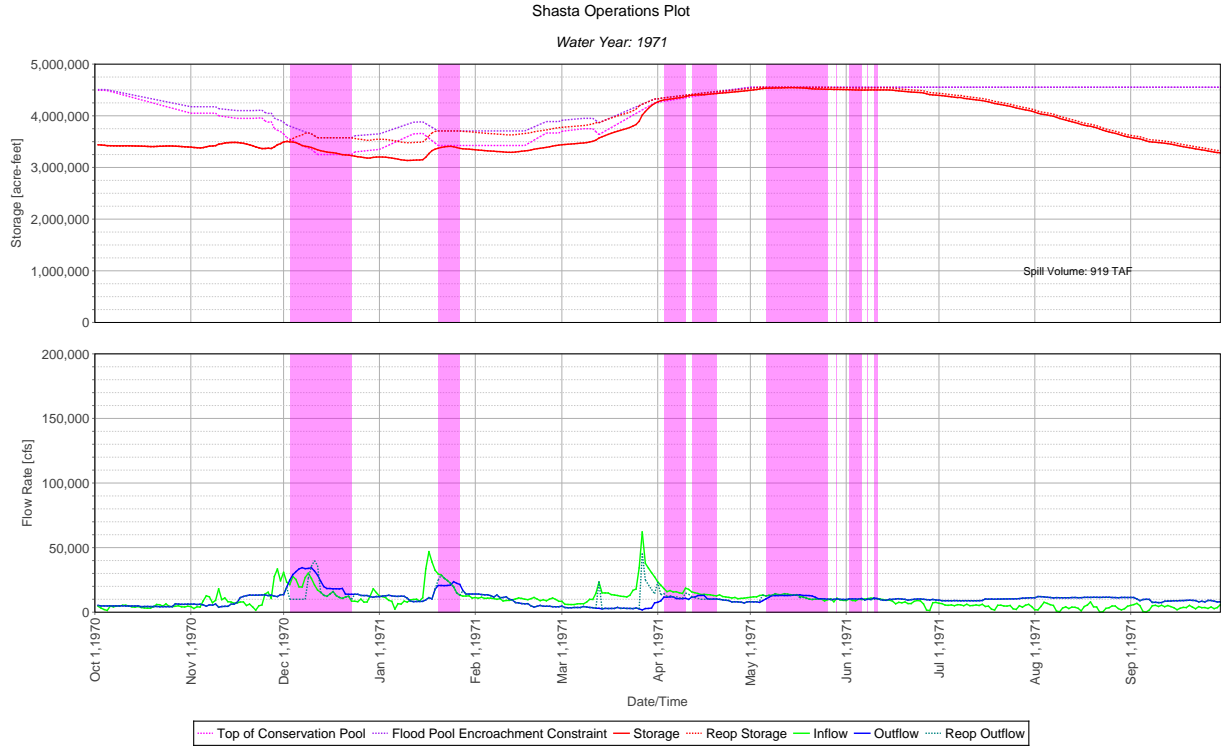
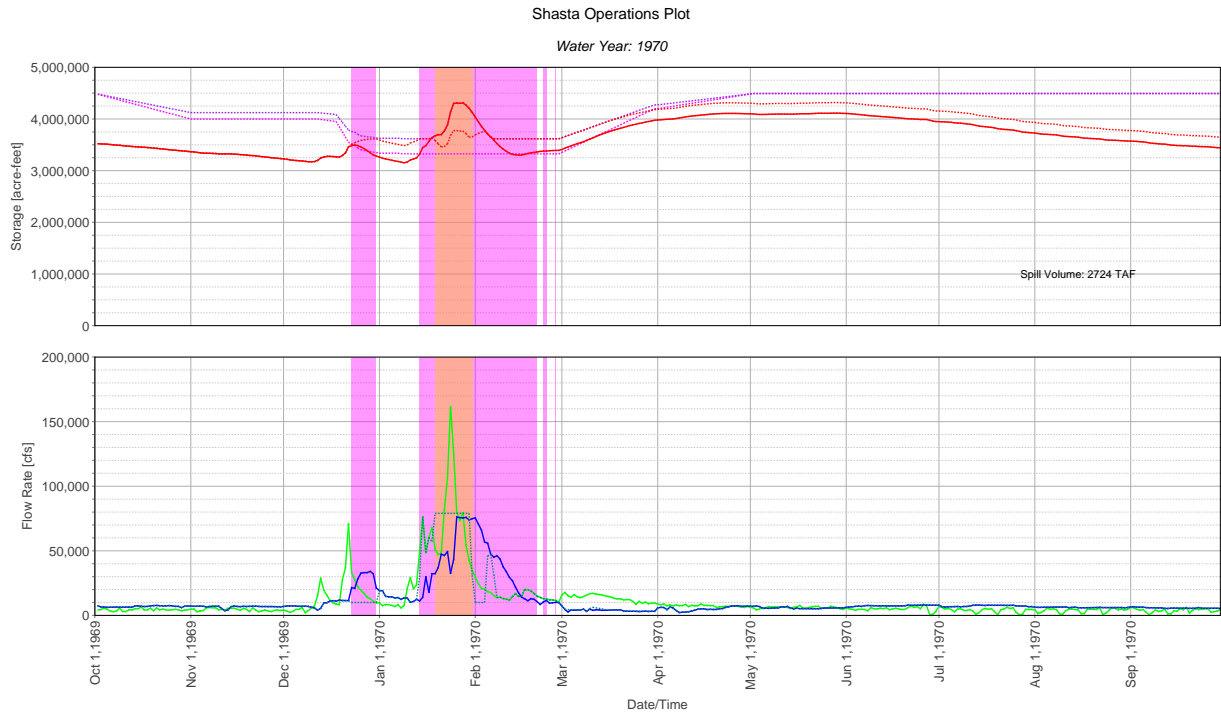


..... Top of Conservation Pool
 Flood Pool Encroachment Constraint
 —— Storage
 Reop Storage
 —— Inflow
 —— Outflow
 Reop Outflow

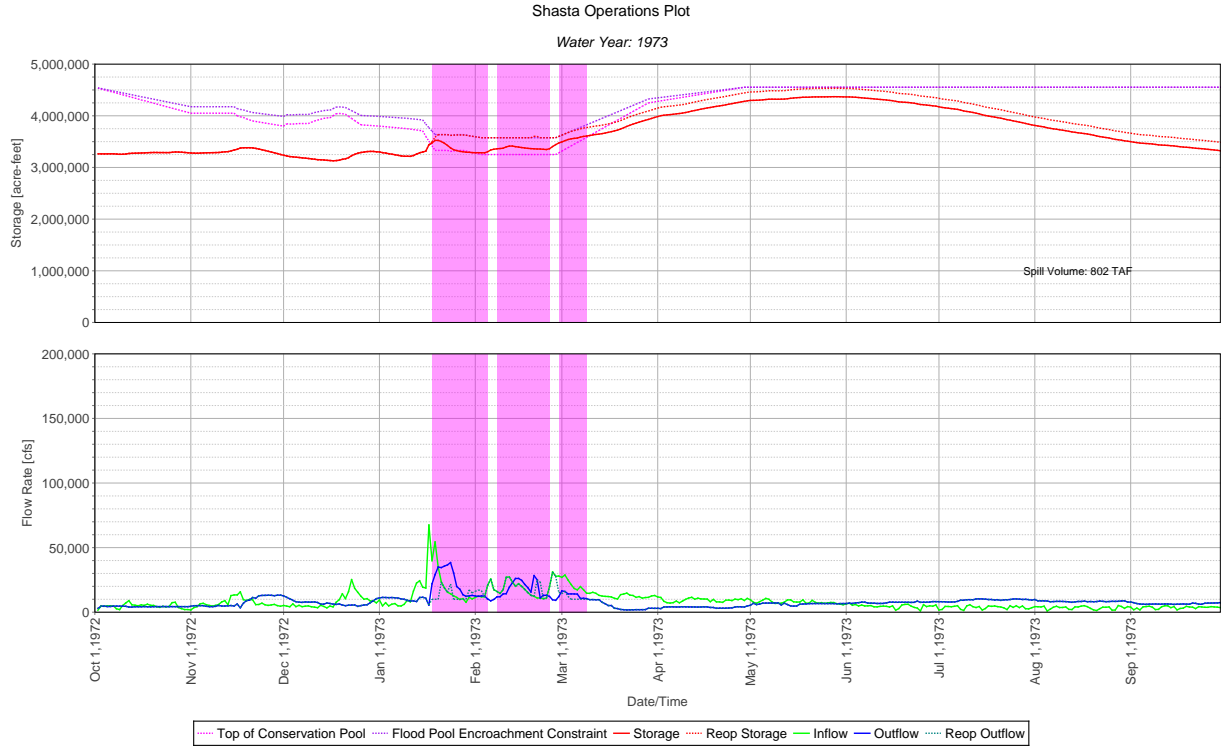
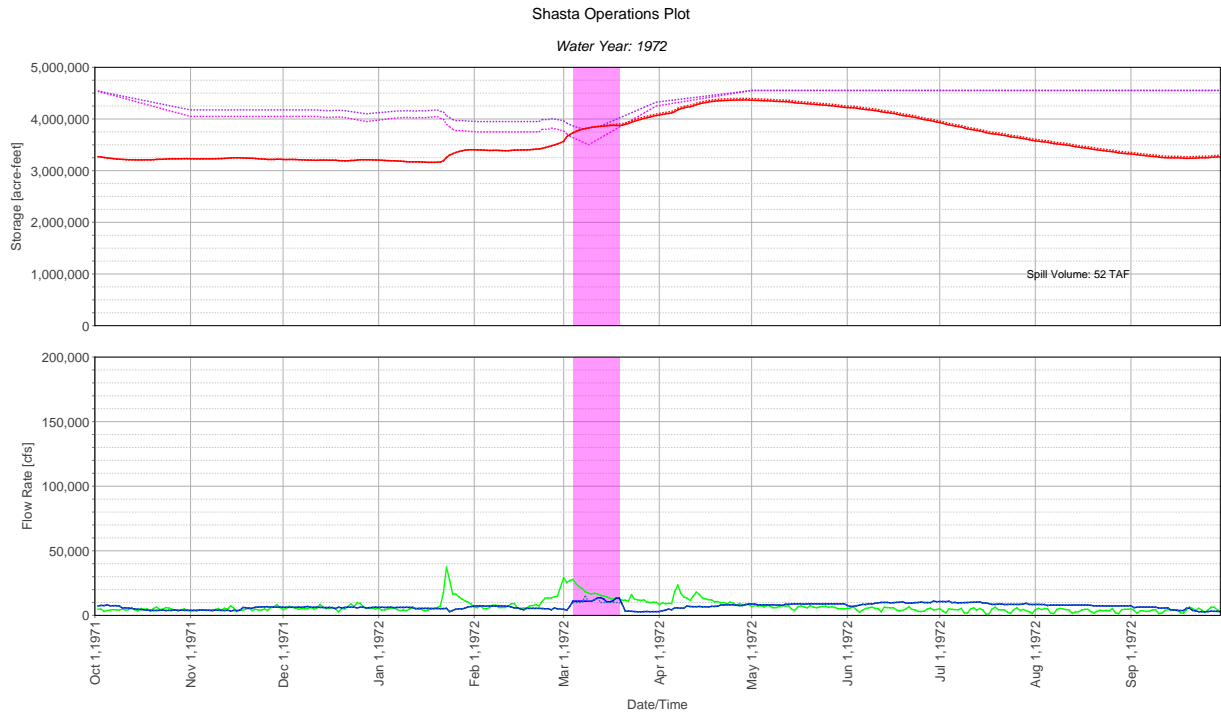
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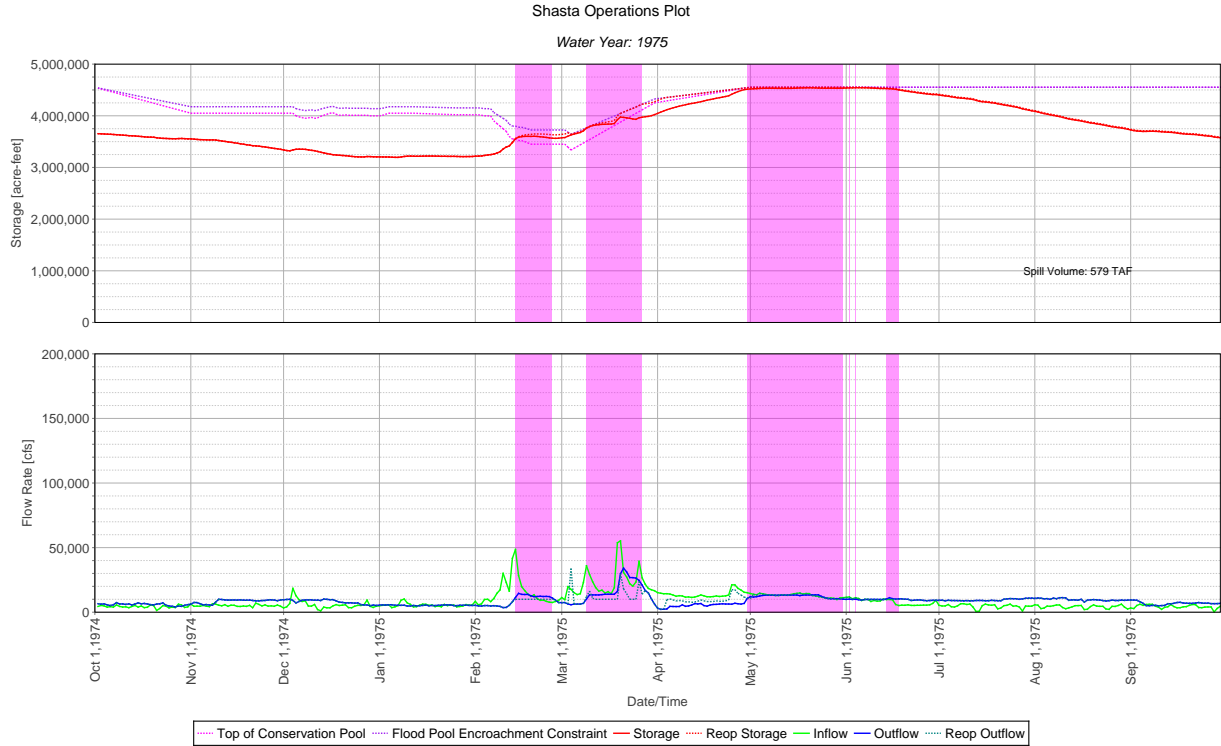
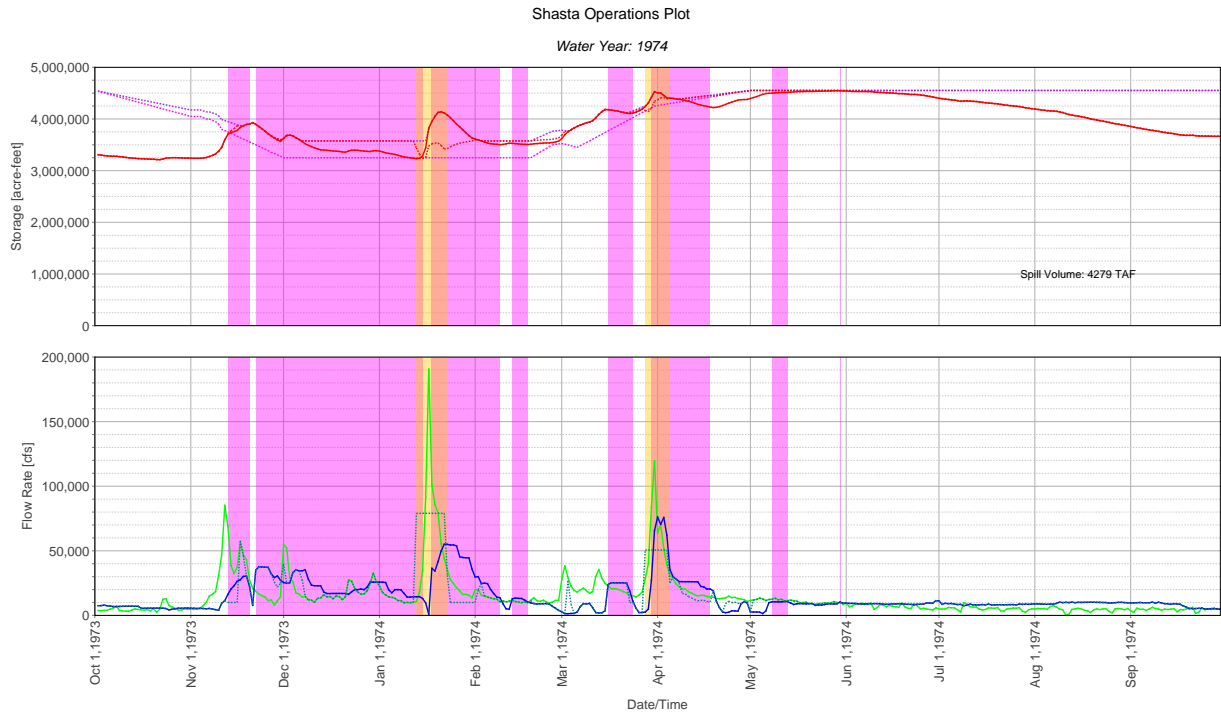
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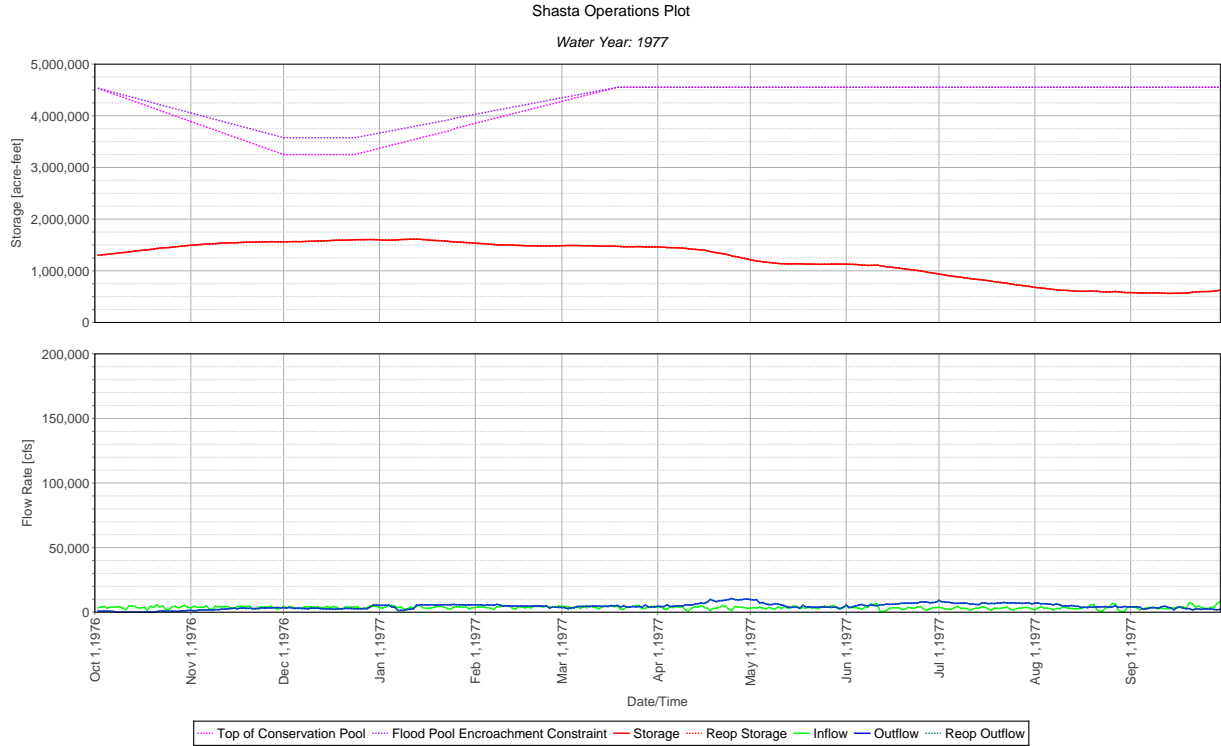
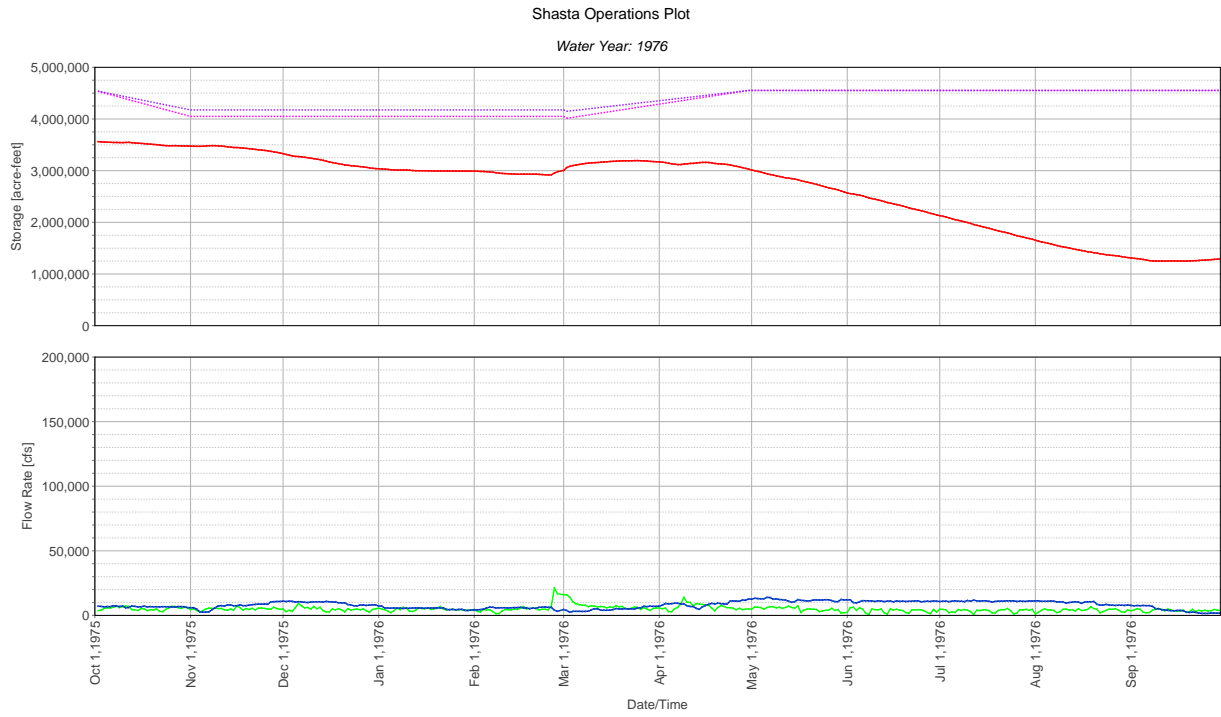
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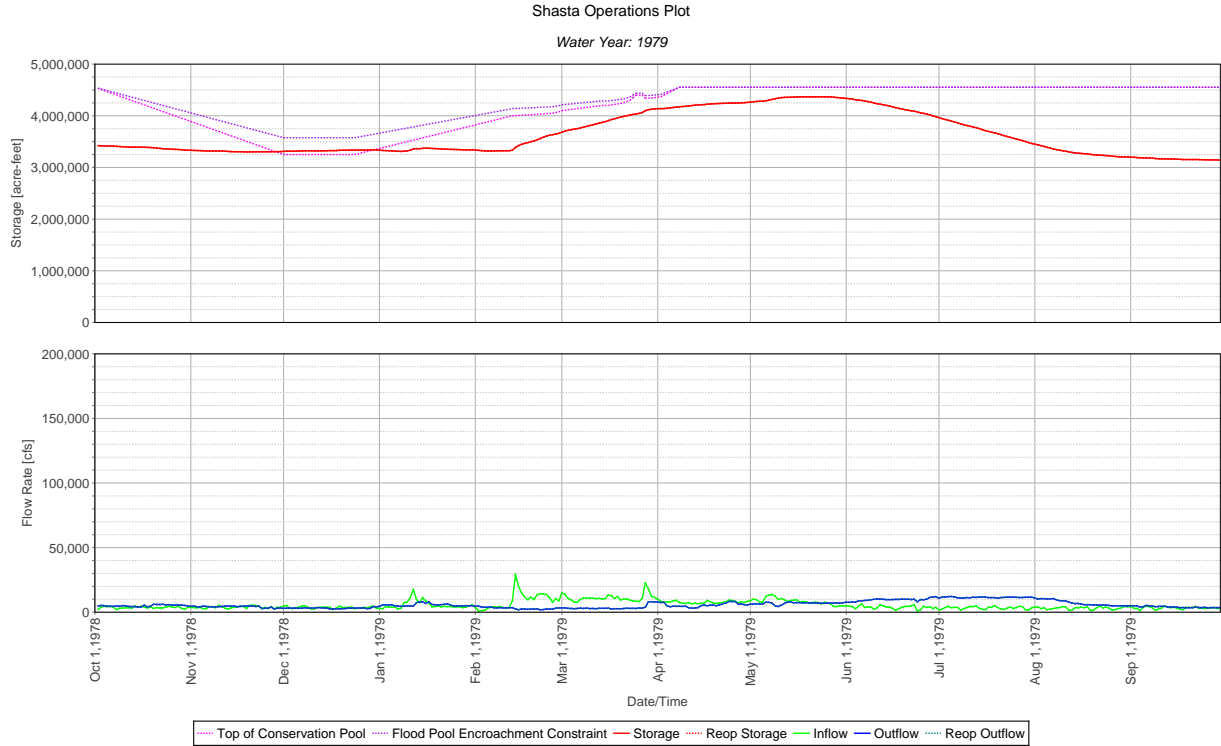
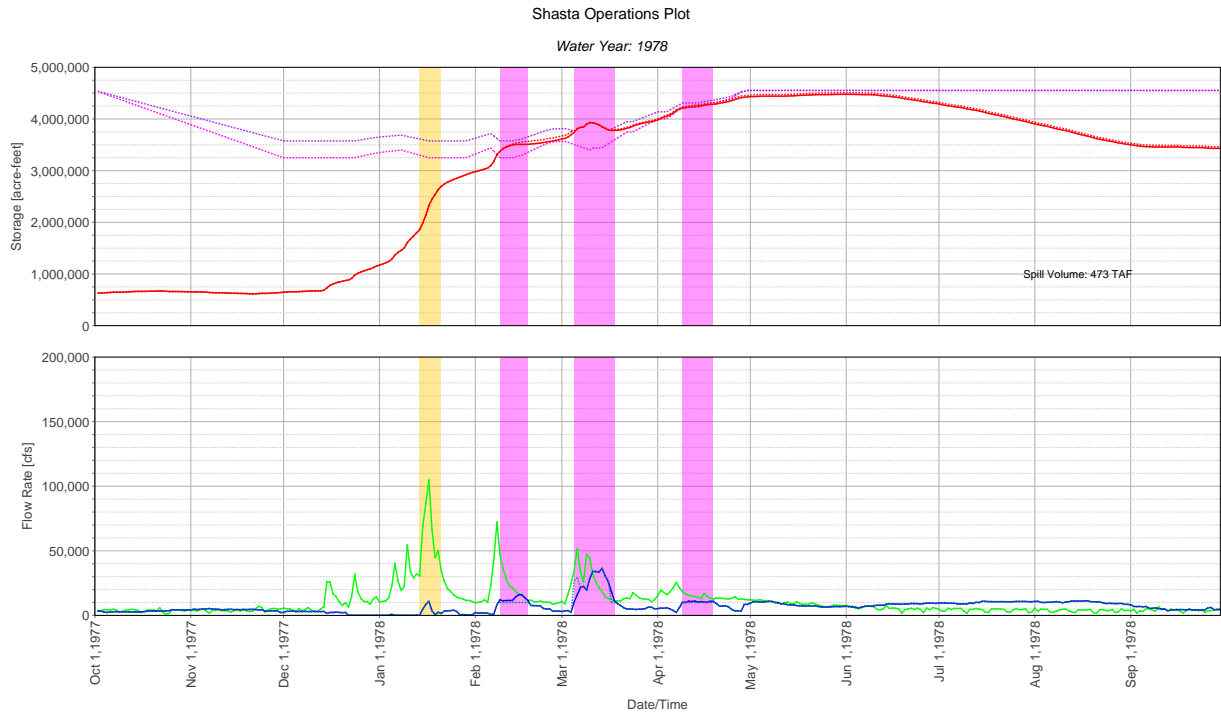
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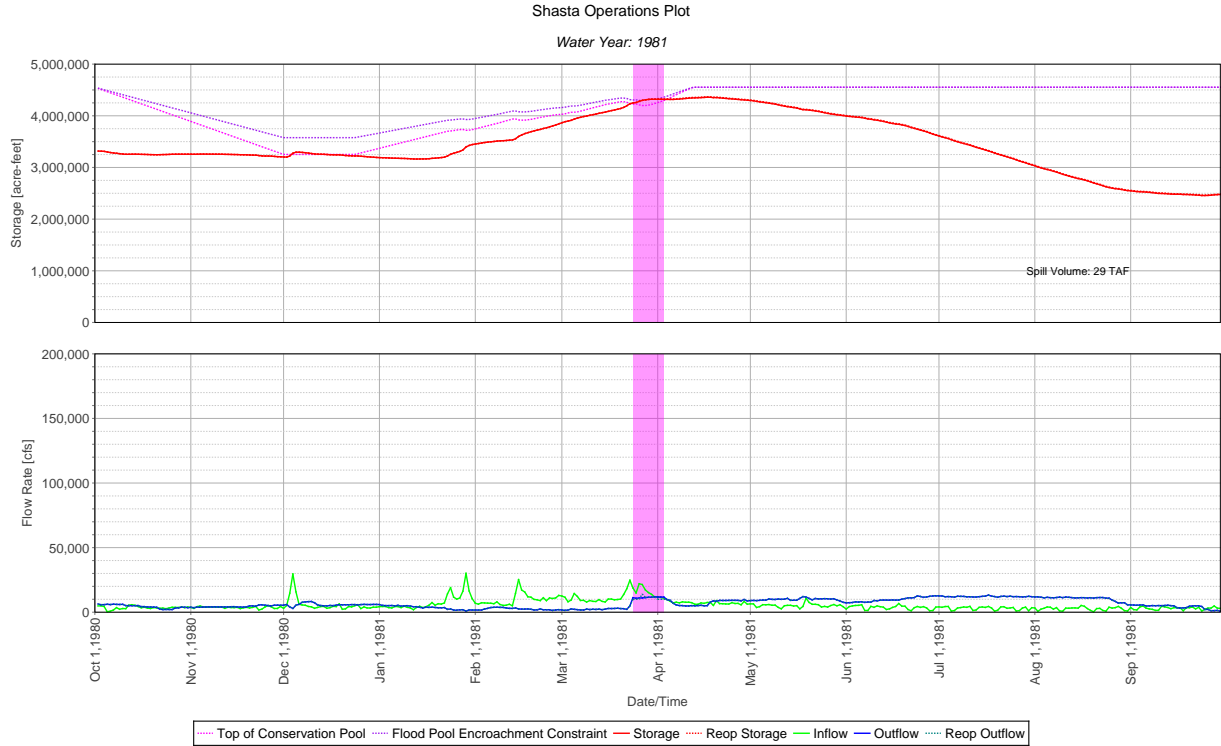
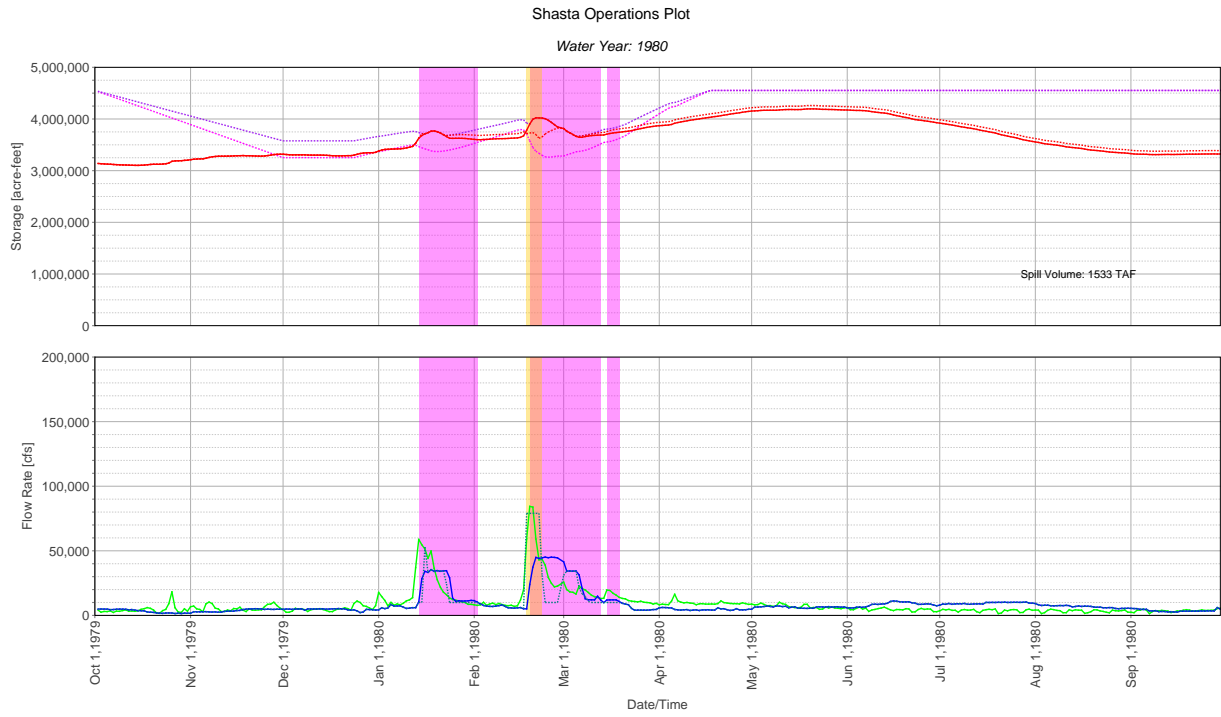
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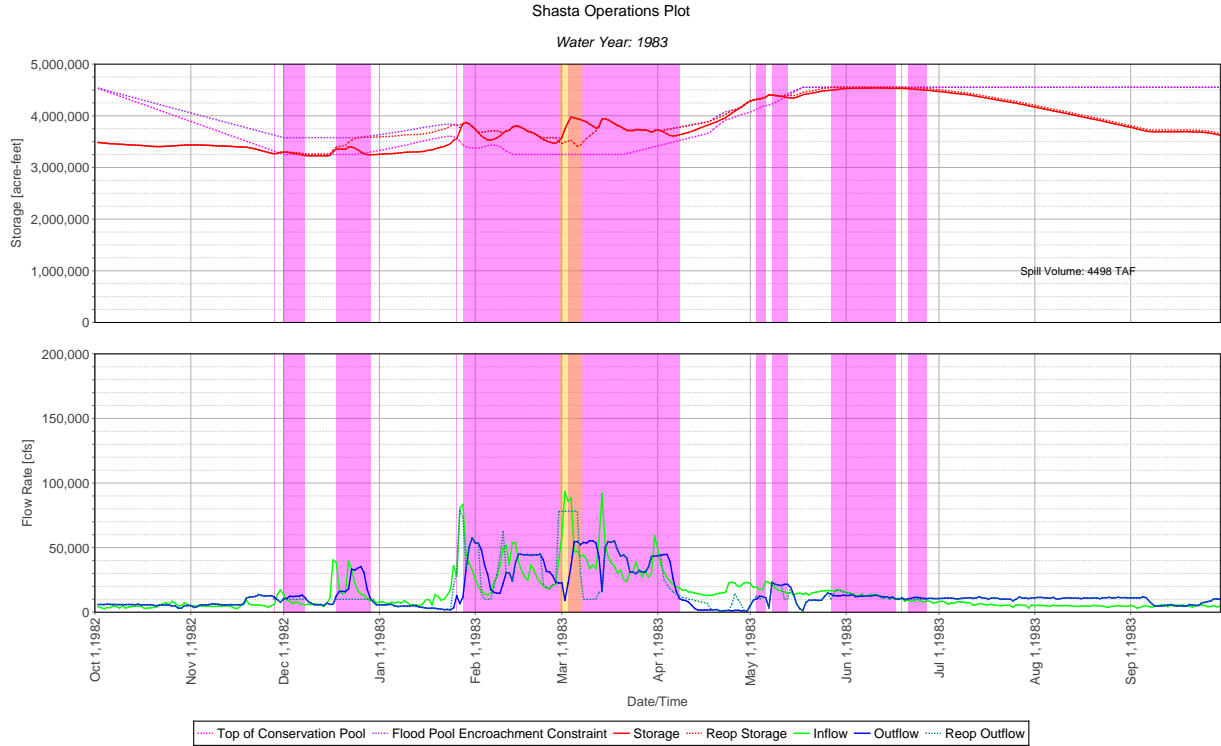
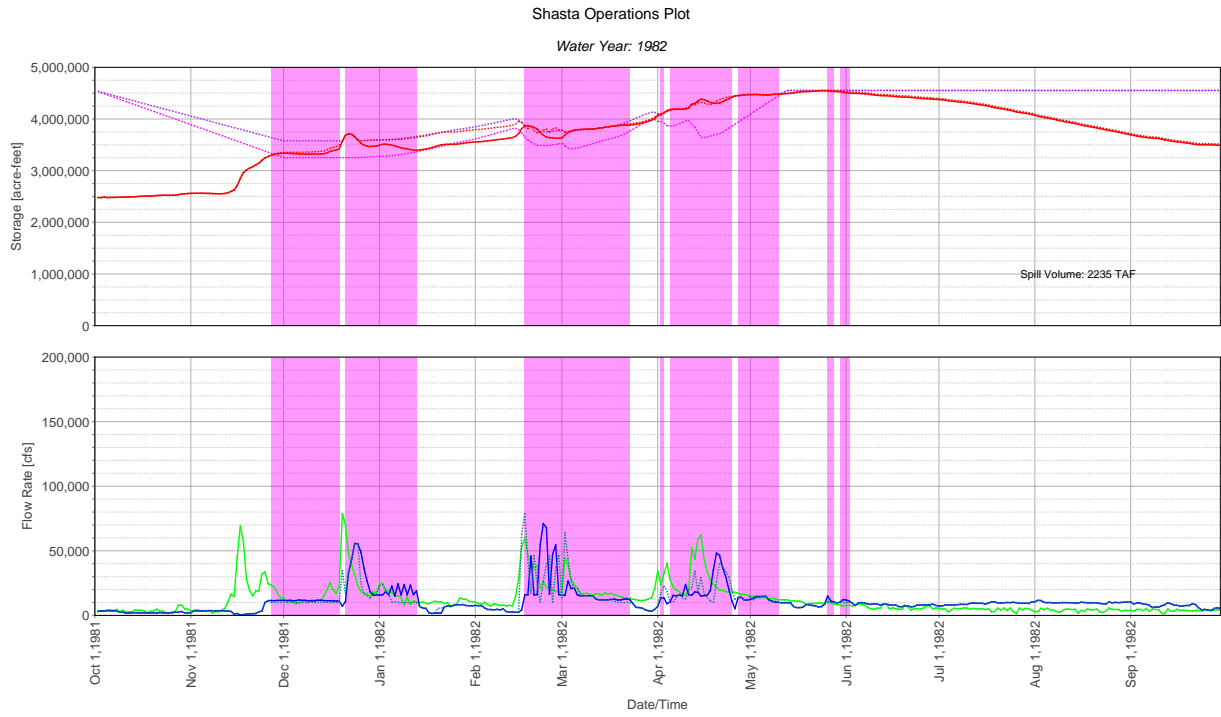
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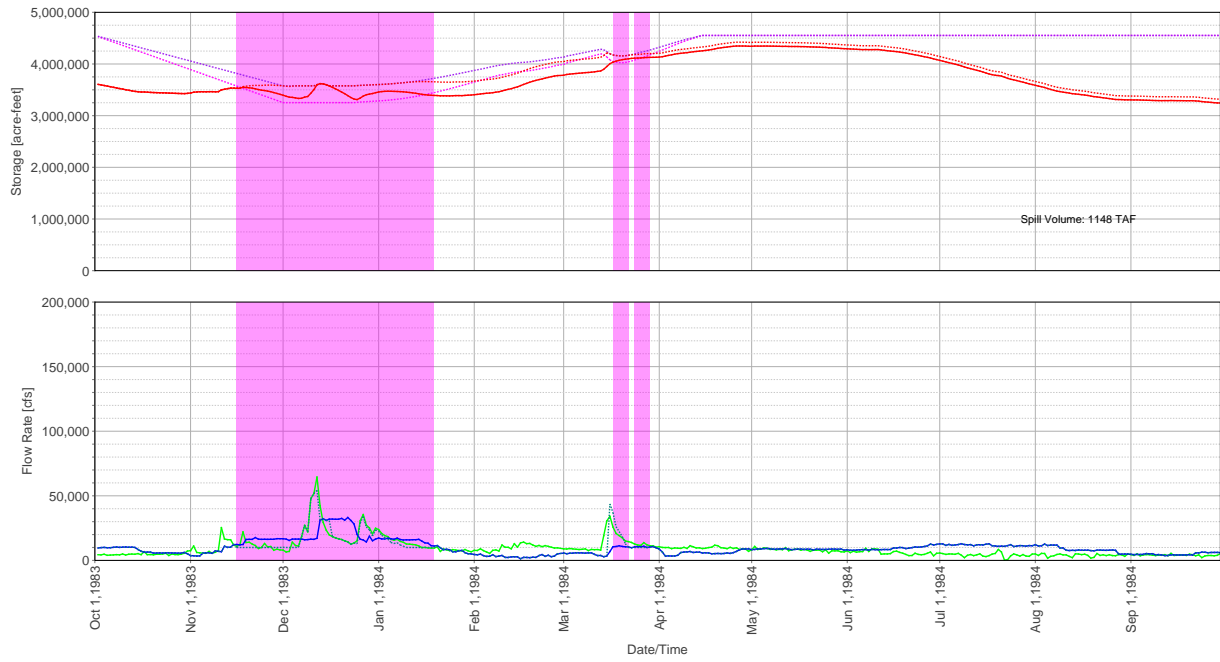
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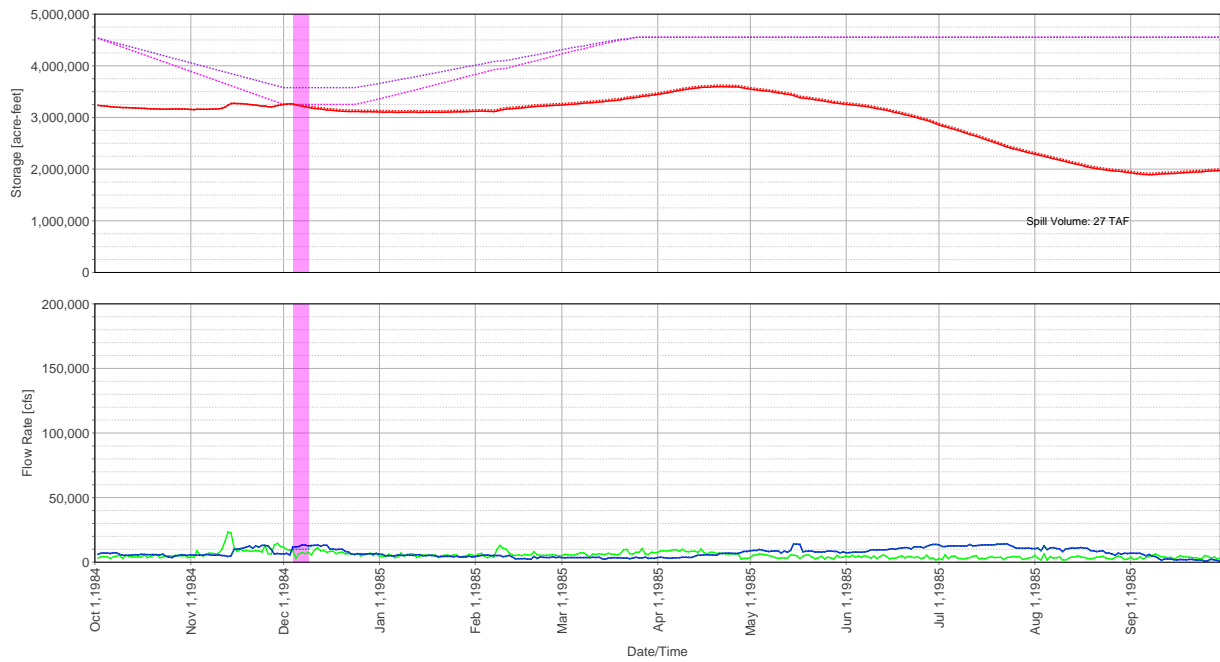
Shasta Operations Plot

Water Year: 1984



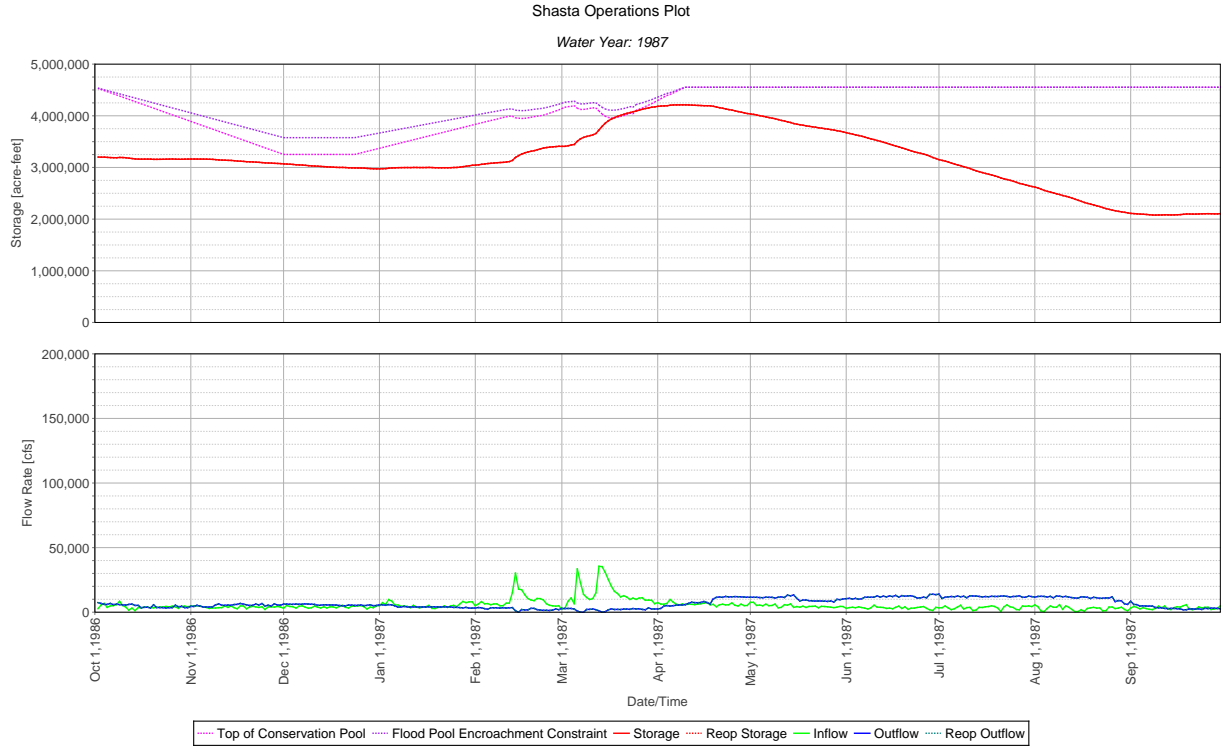
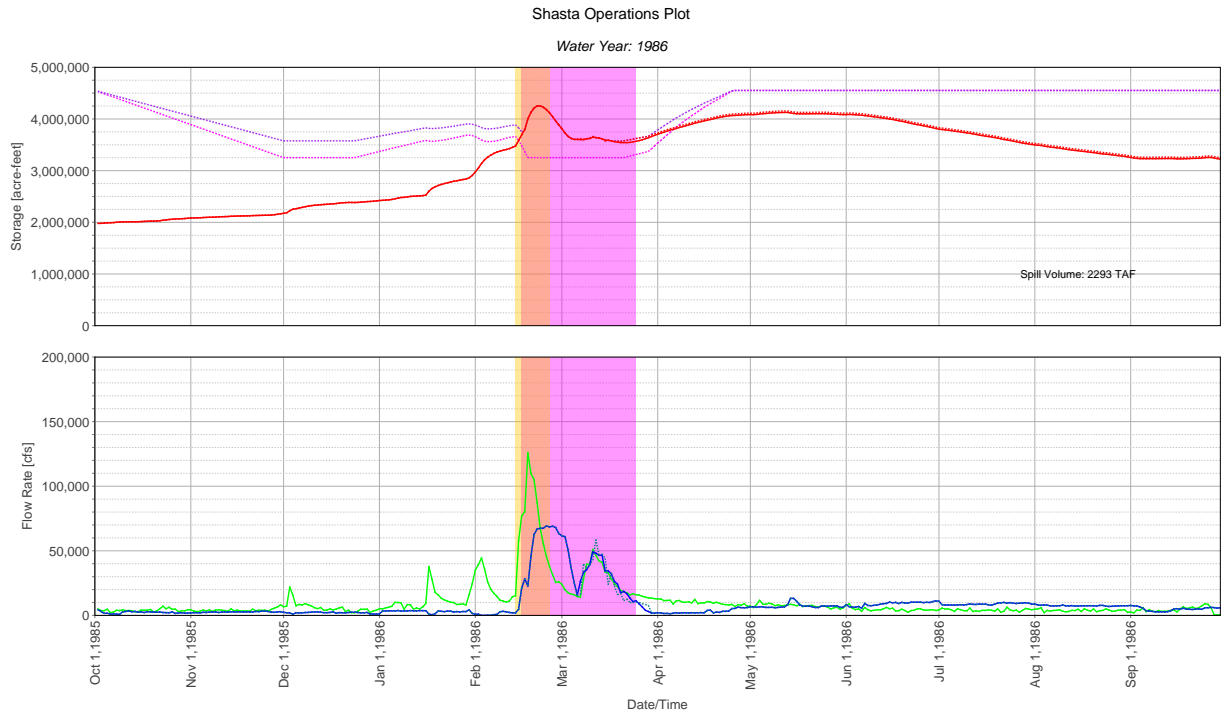
Shasta Operations Plot

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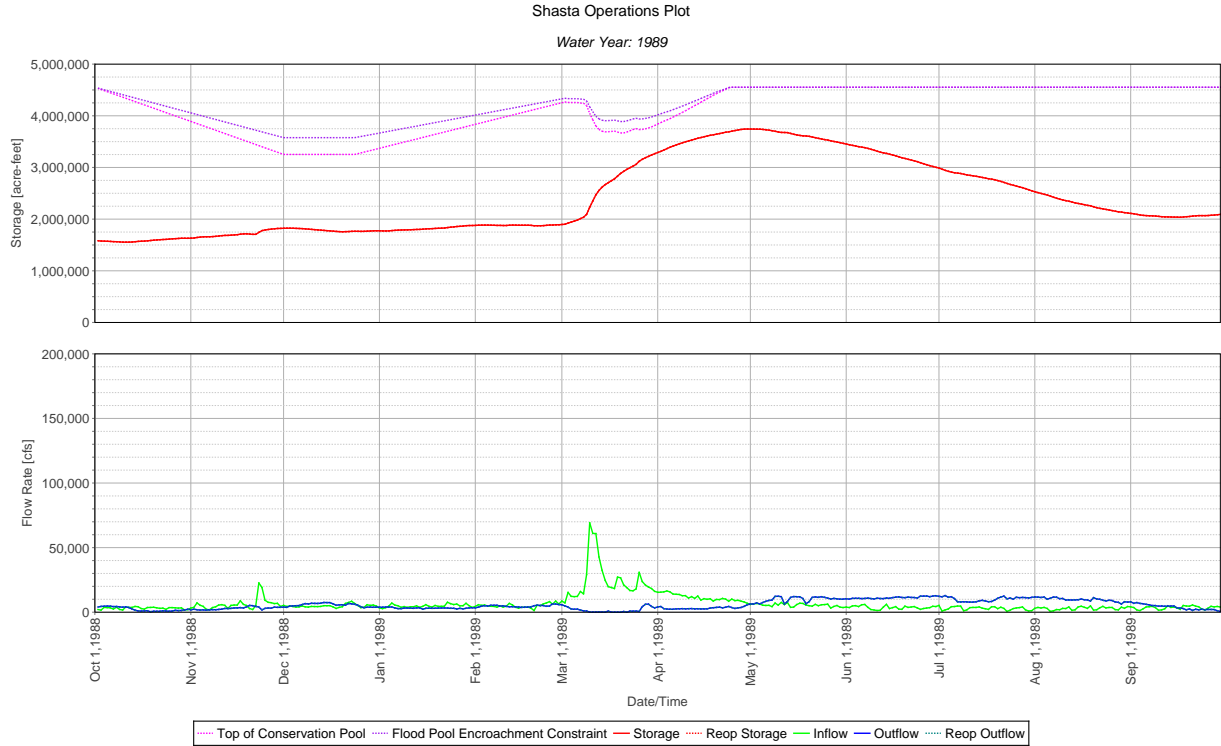
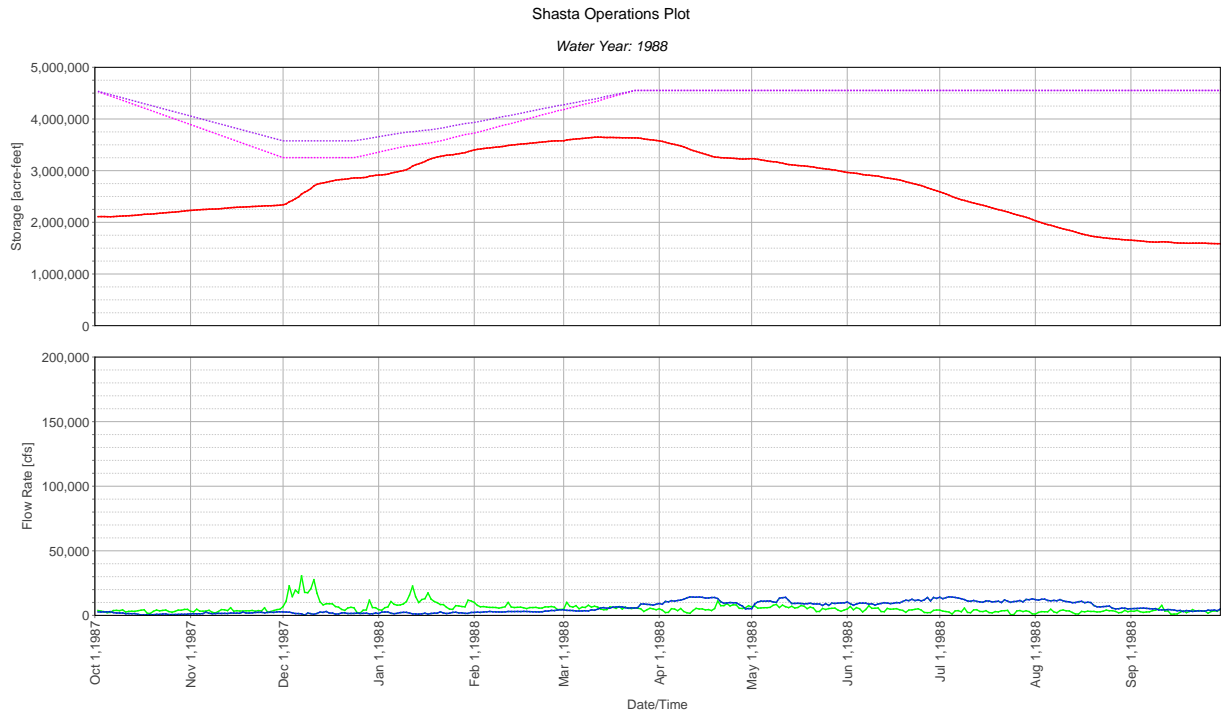


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 Flood Pool Encroachment Constraint
 —— Storage
 Reop Storage
 —— Inflow
 —— Outflow
 Reop Outflow

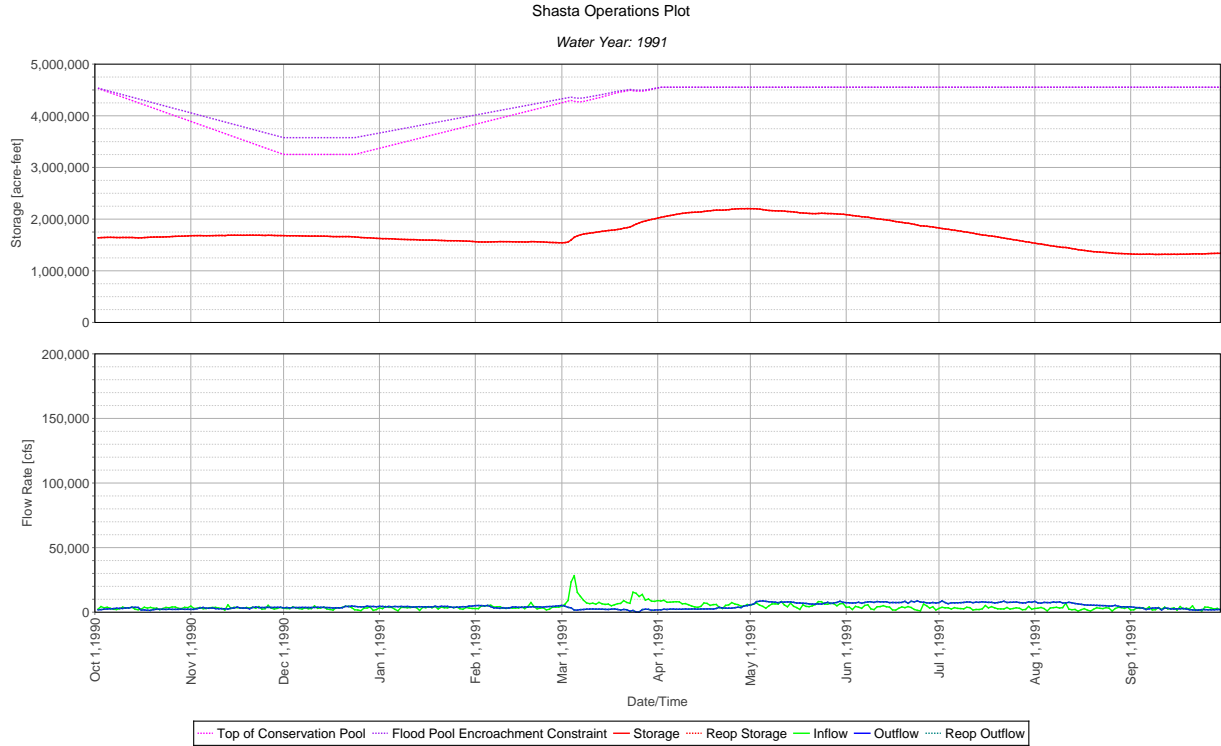
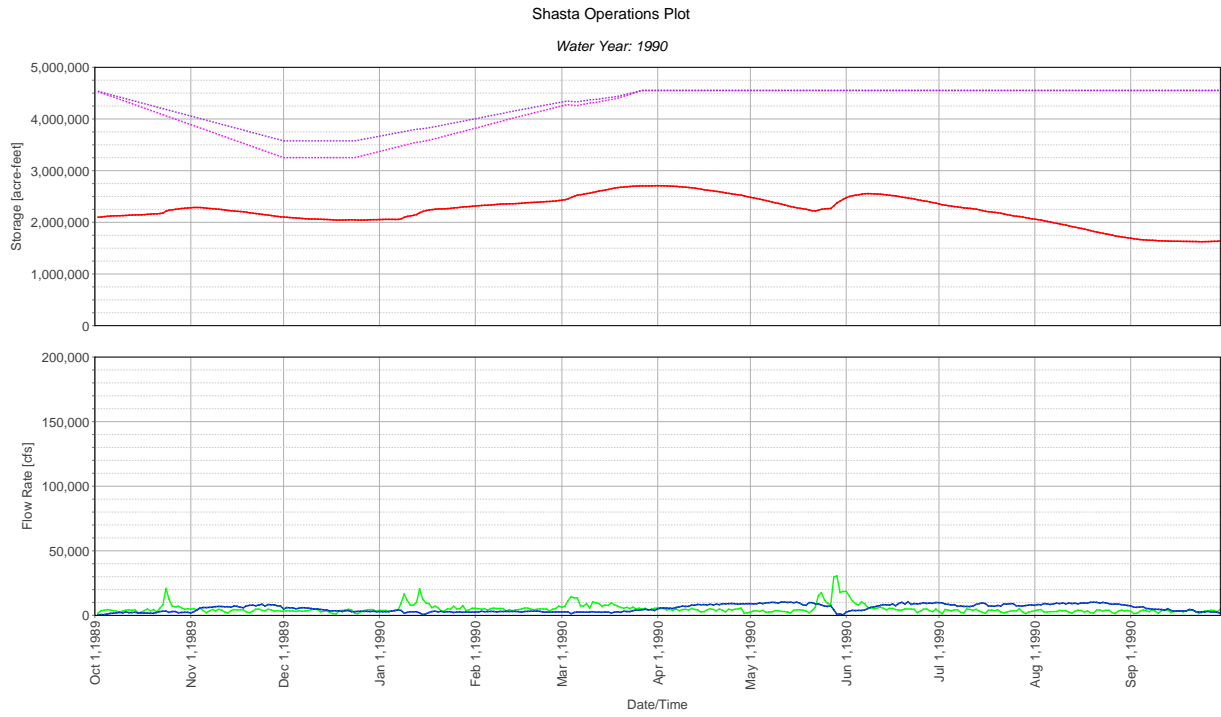
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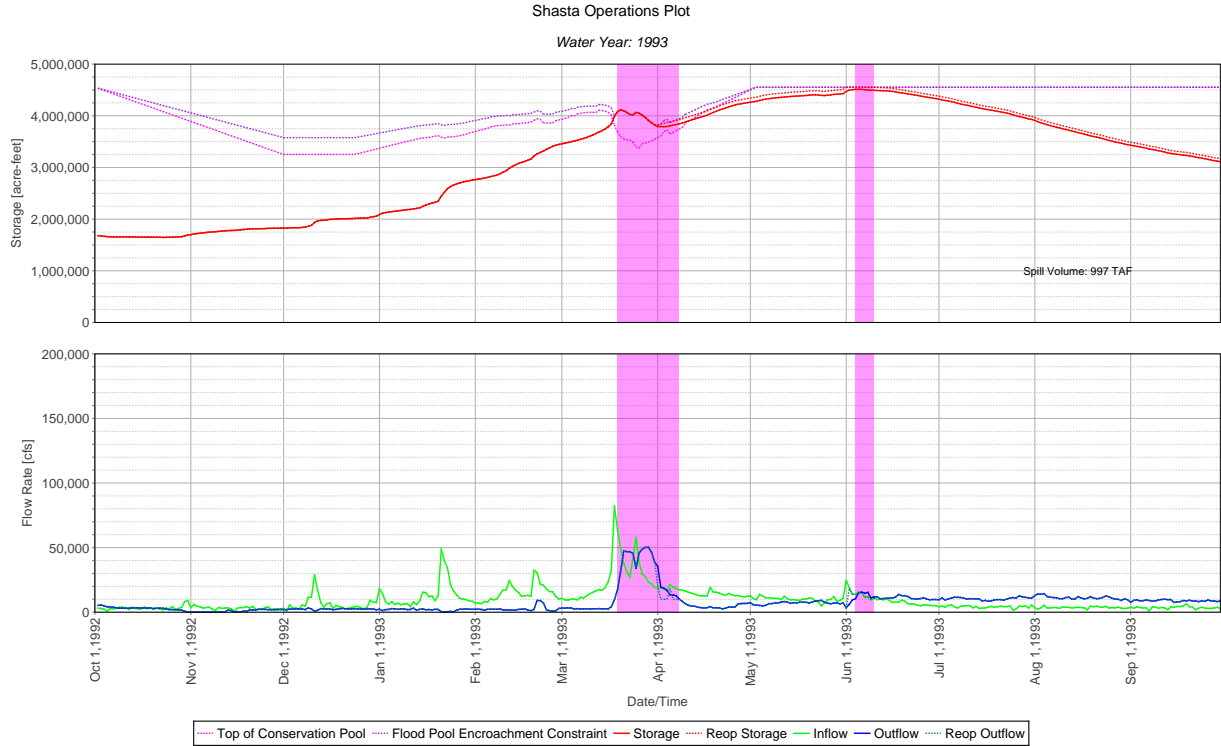
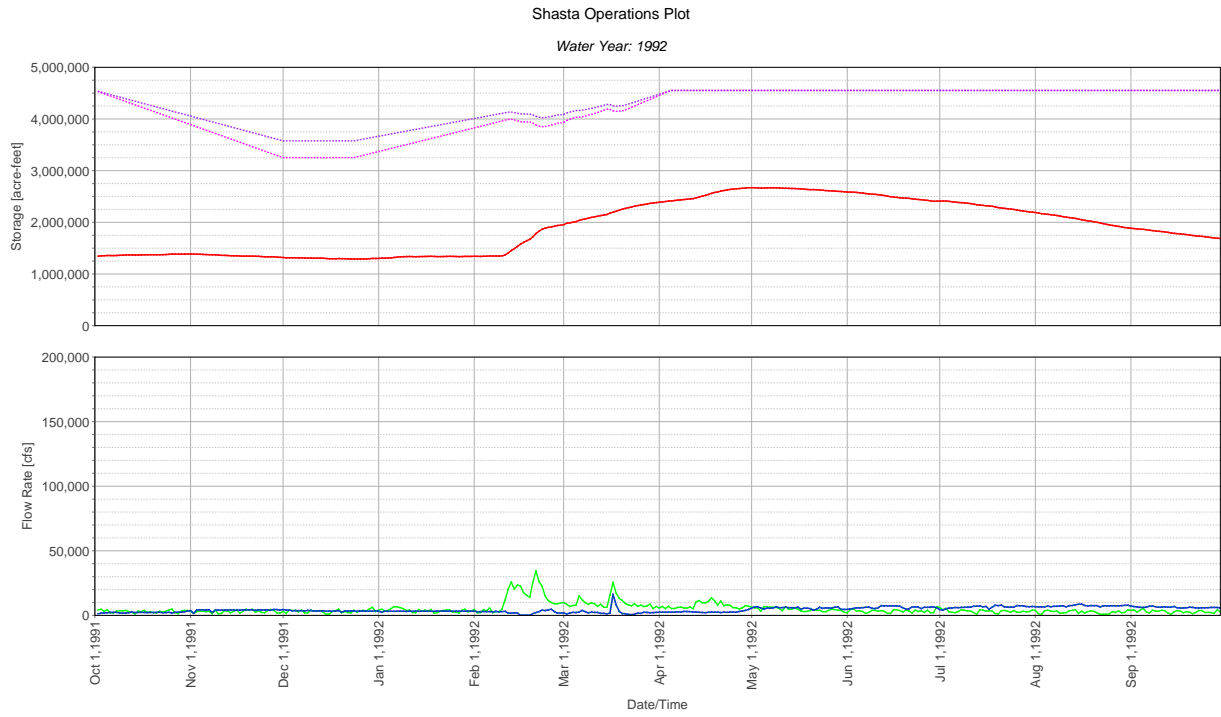
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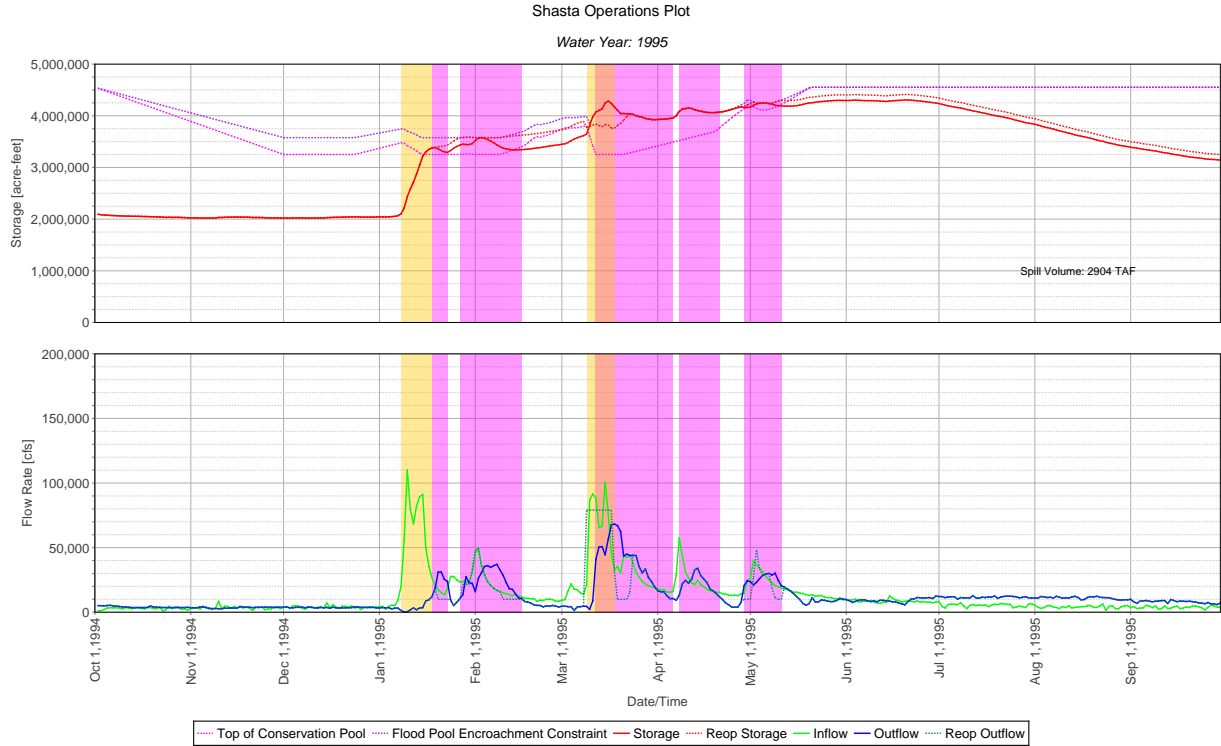
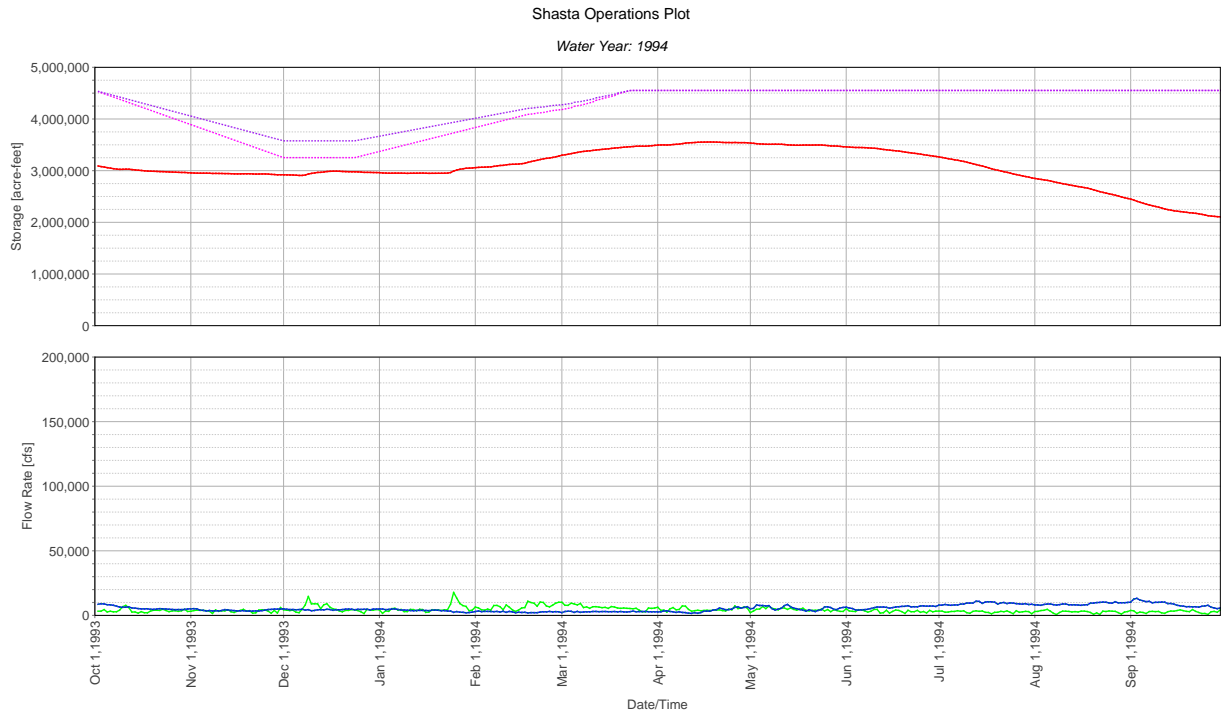
A: Reoperation Plots



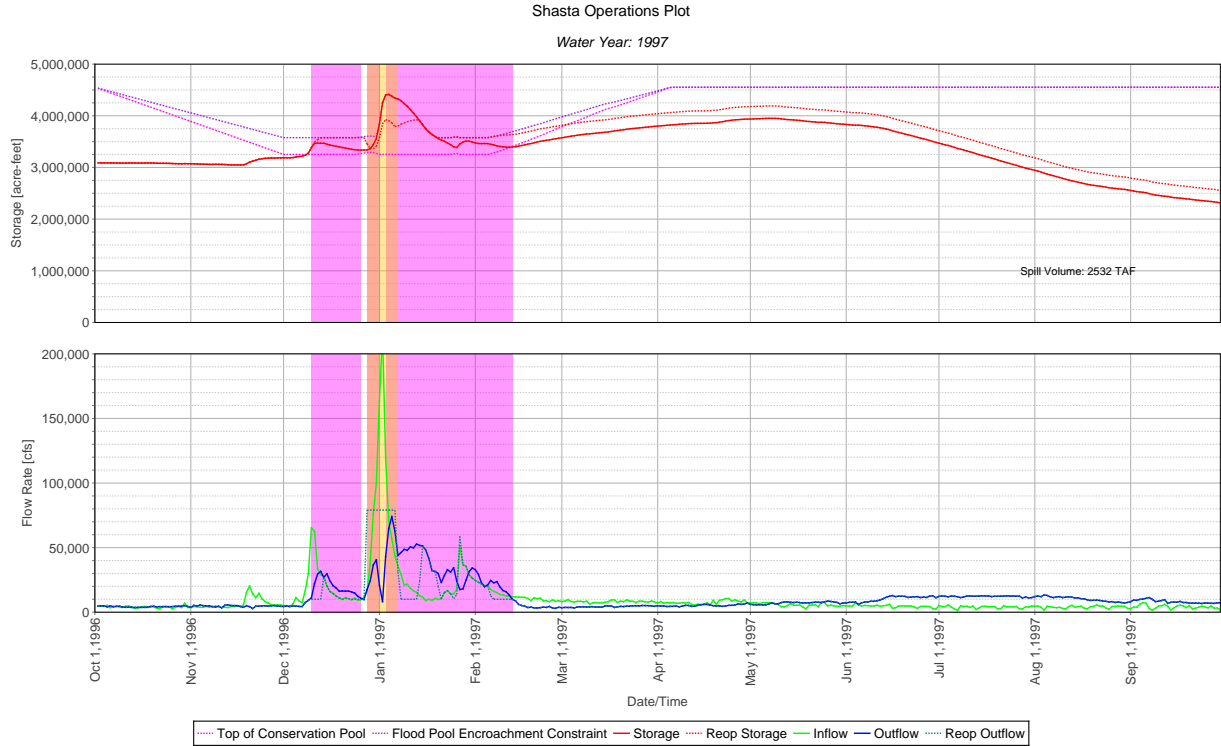
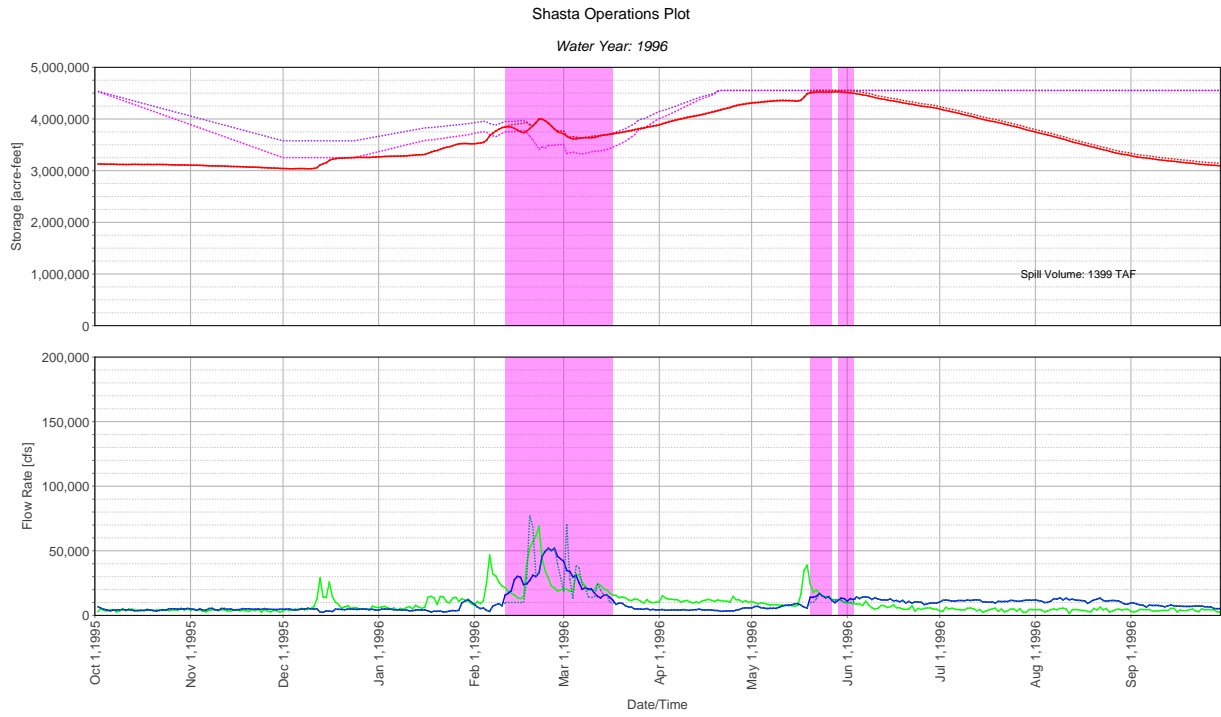
A: Reoperation Plots



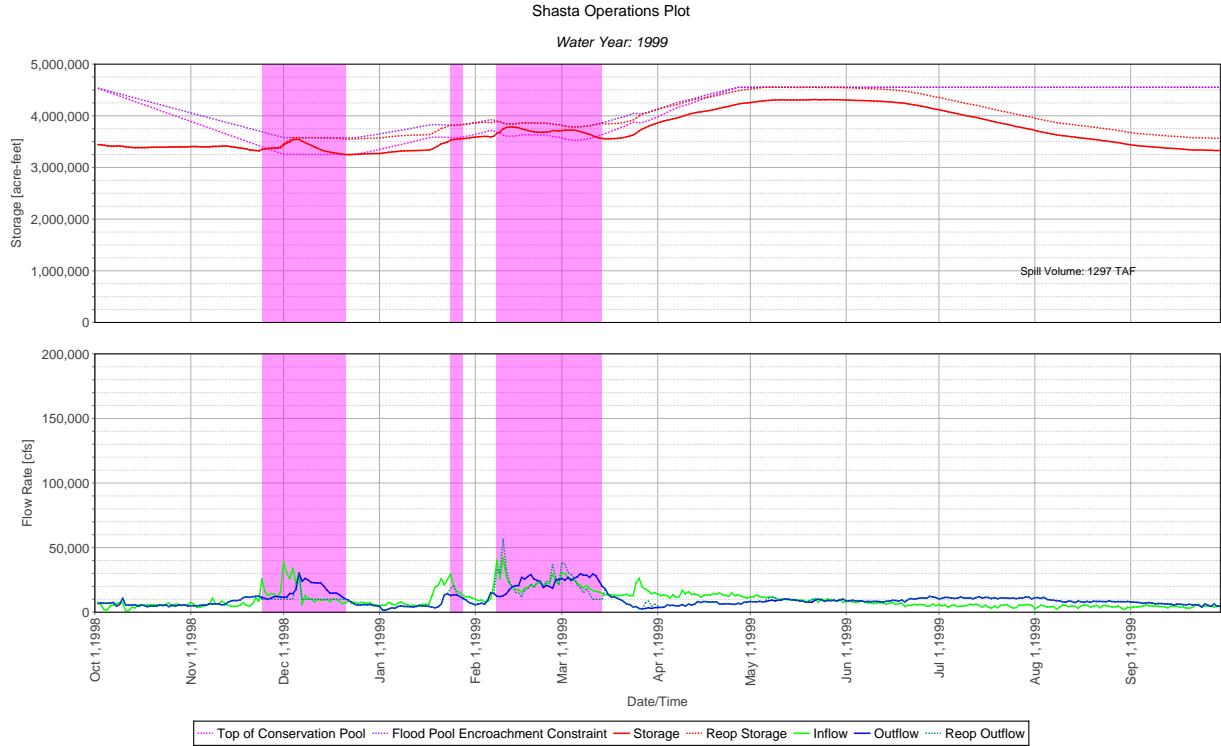
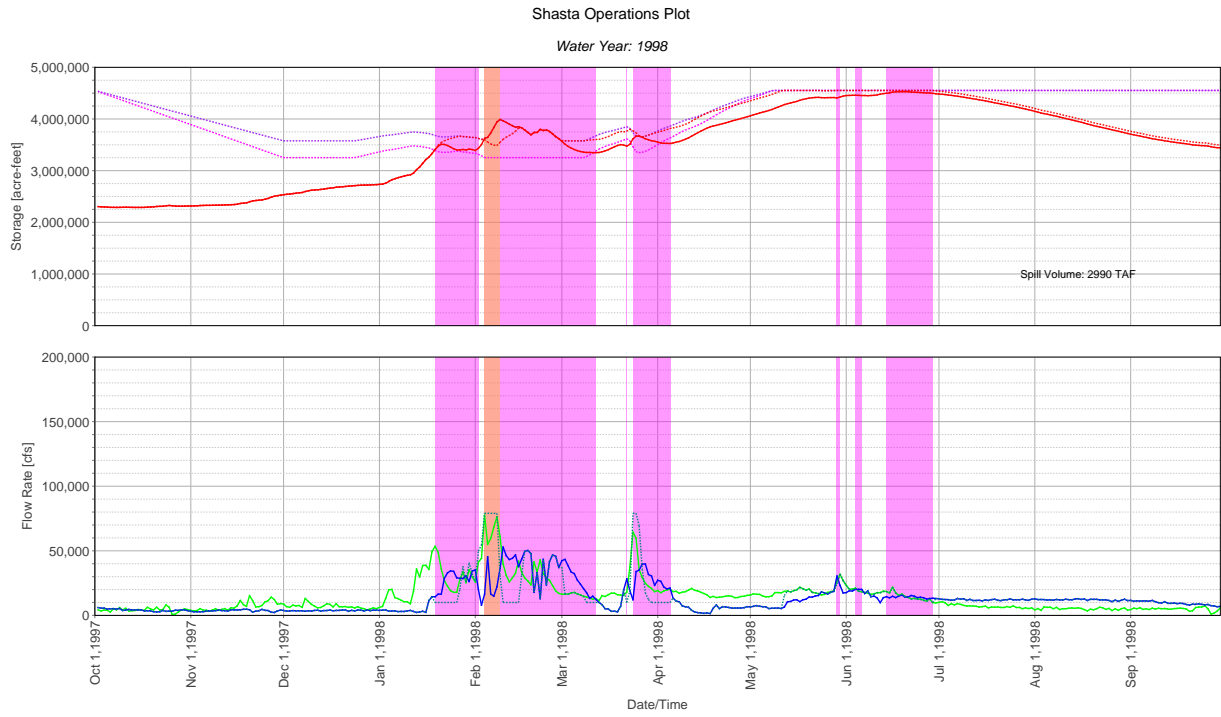
A: Reoperation Plots



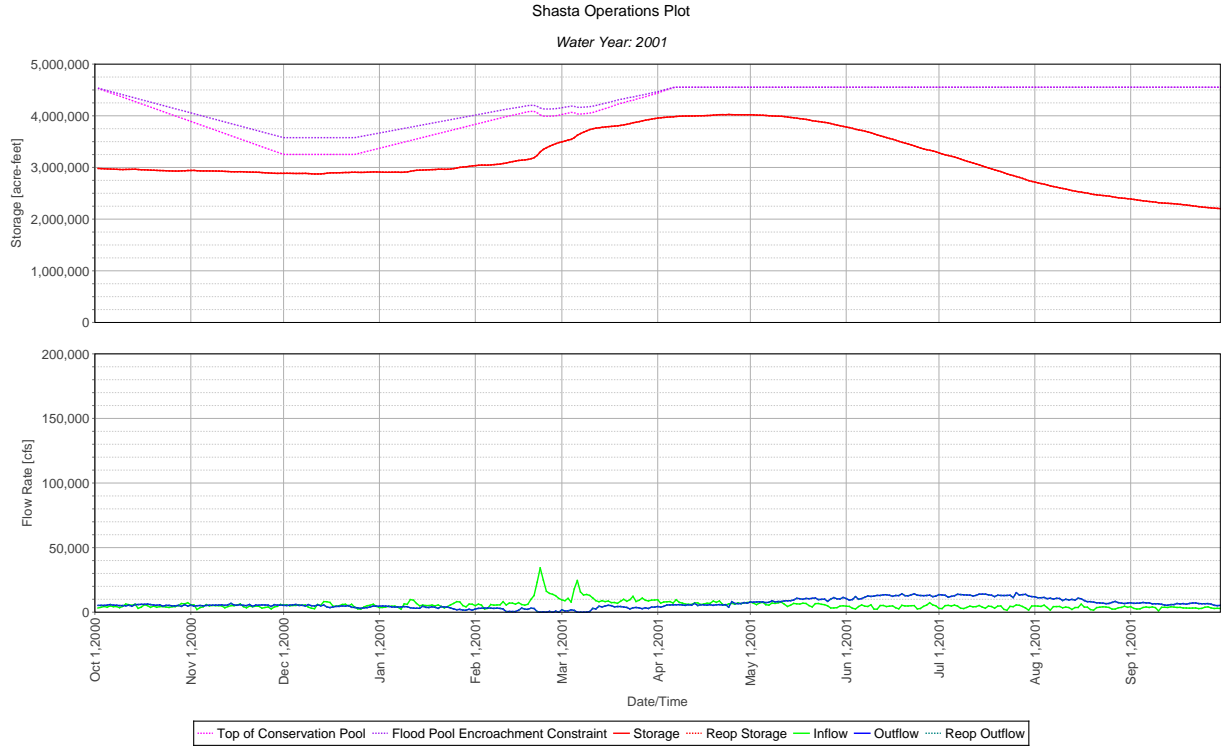
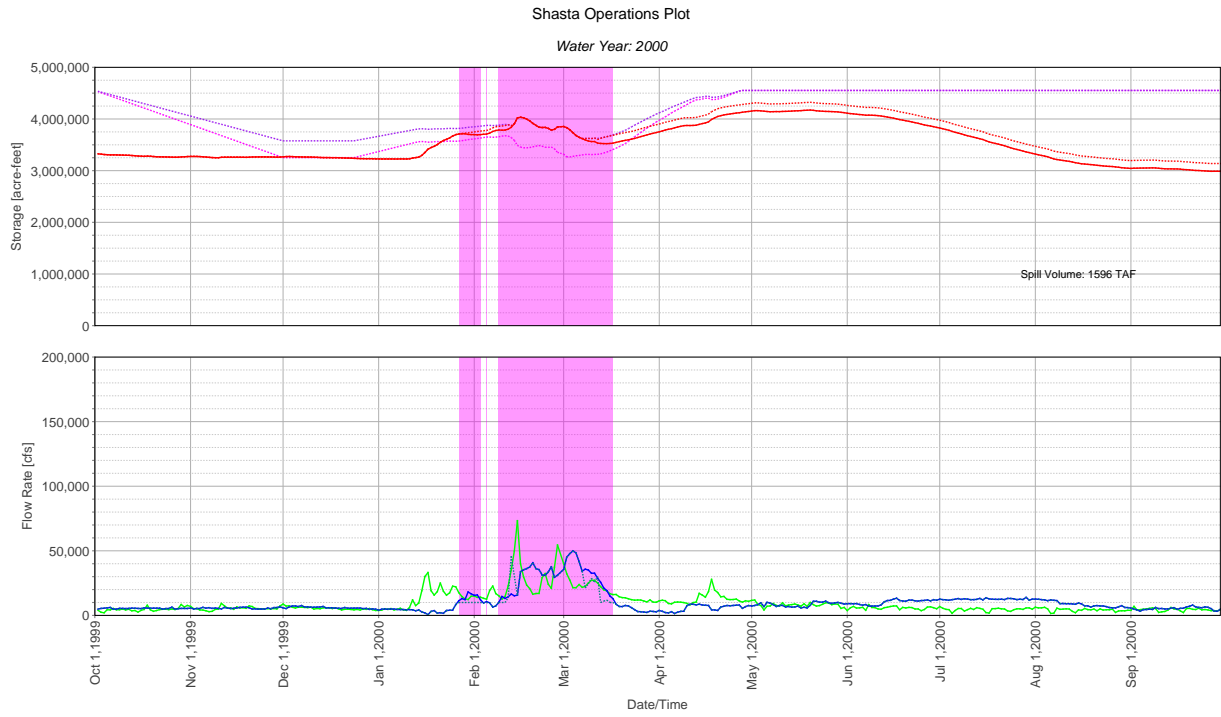
A: Reoperation Plots



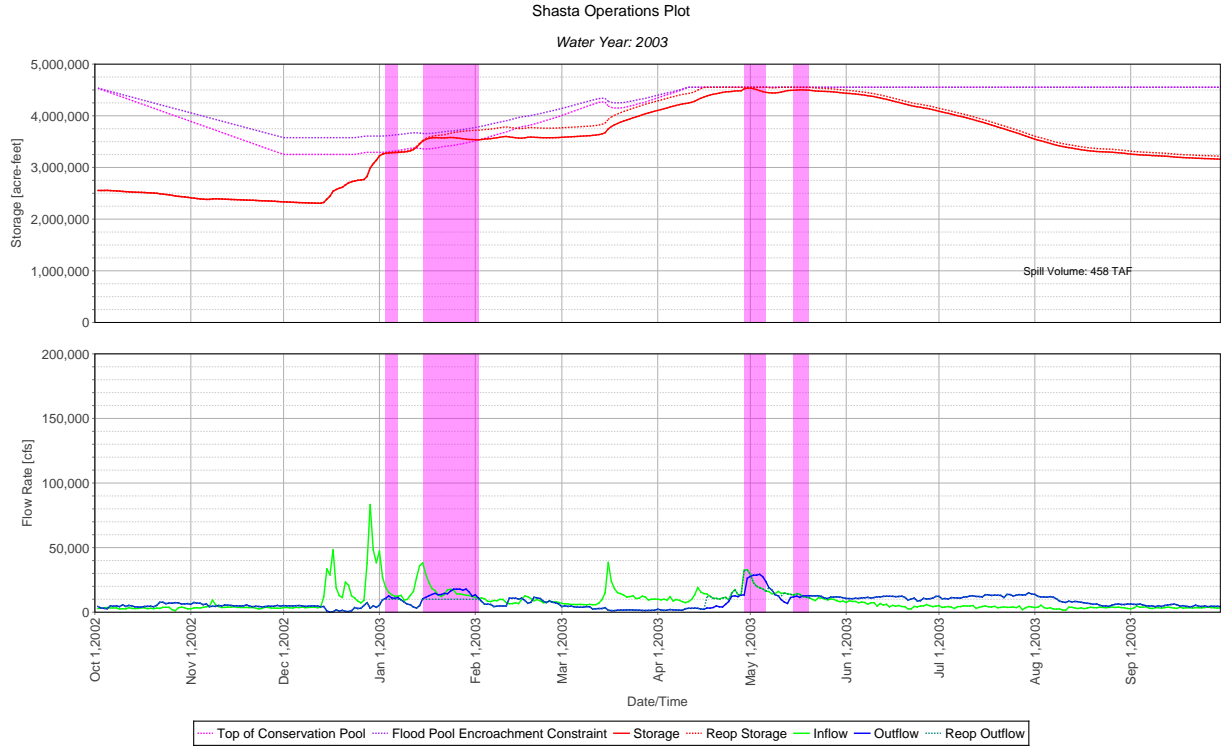
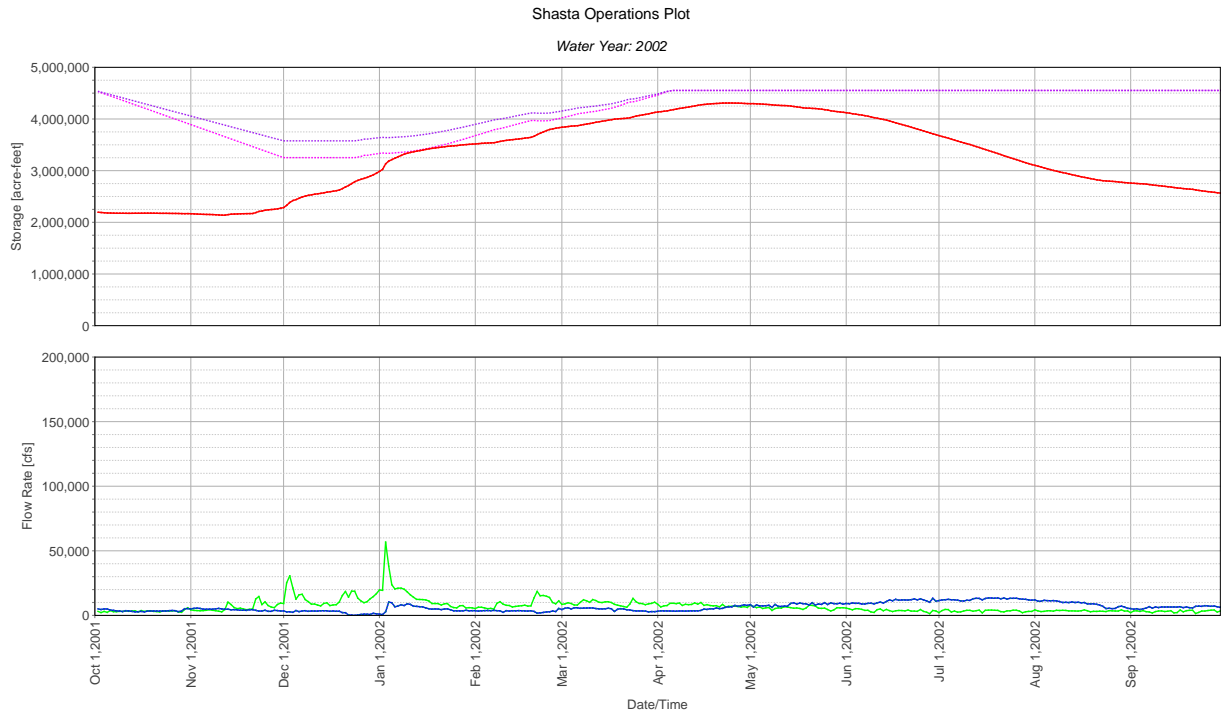
A: Reoperation Plots



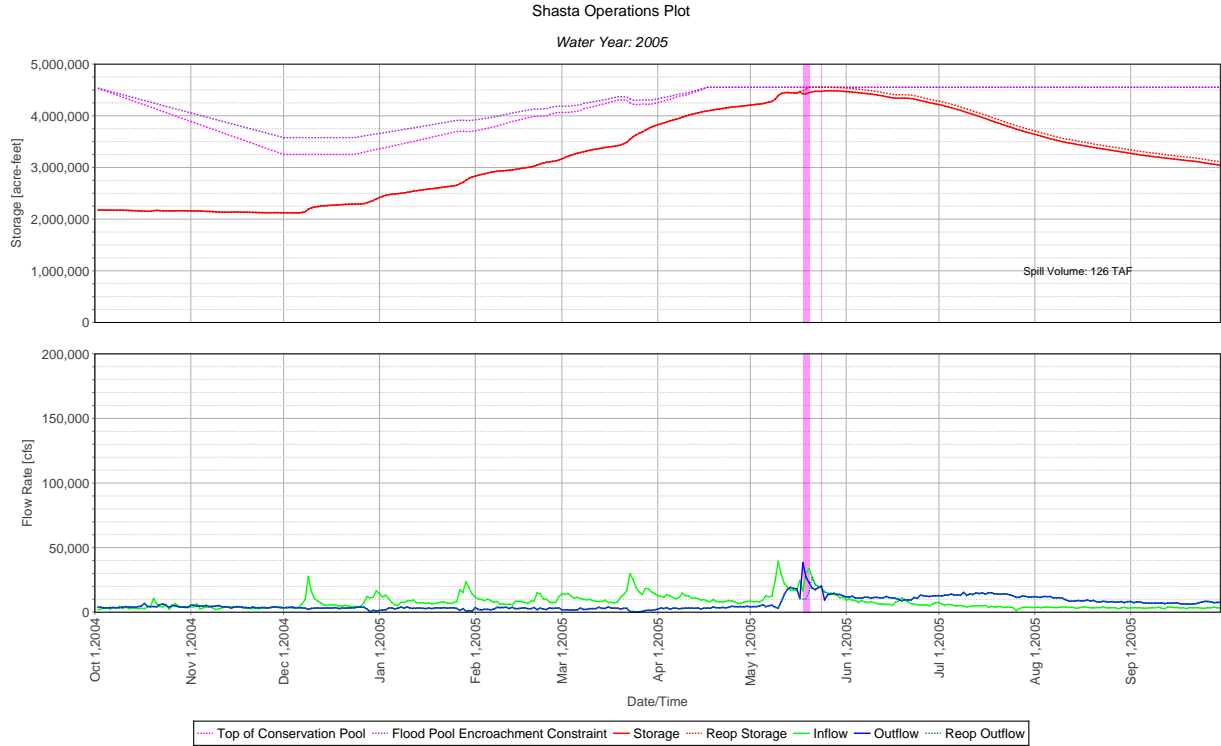
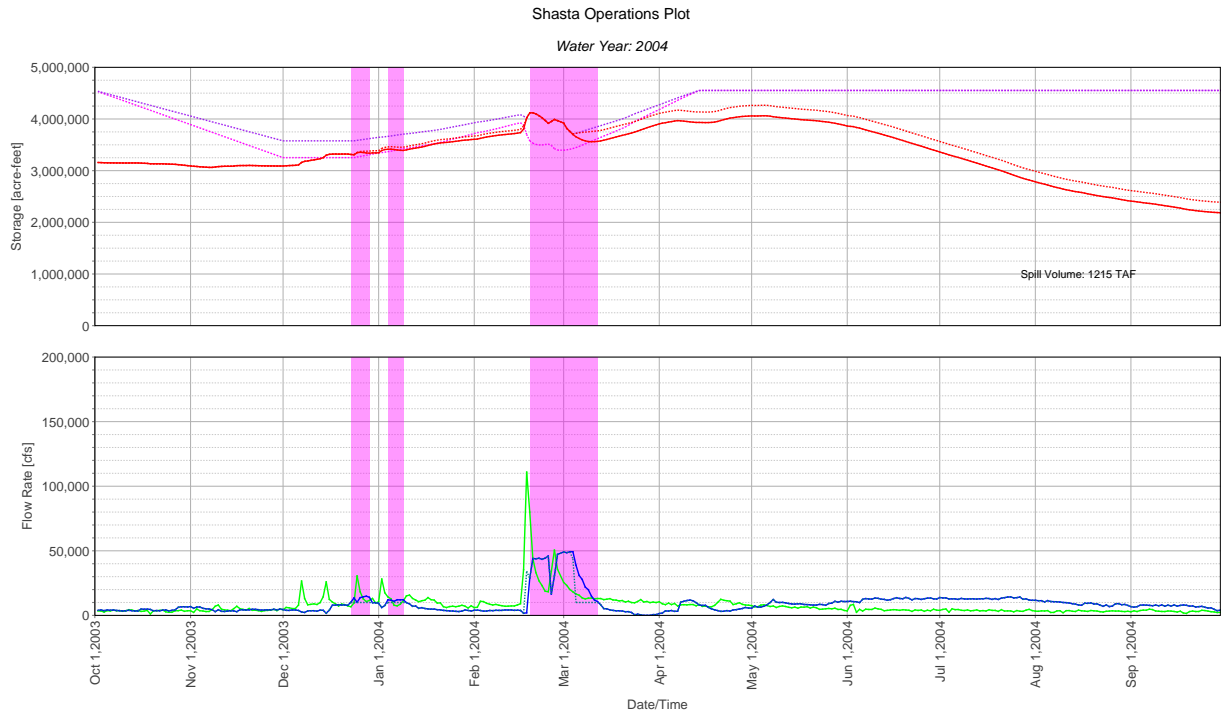
A: Reoperation Plots



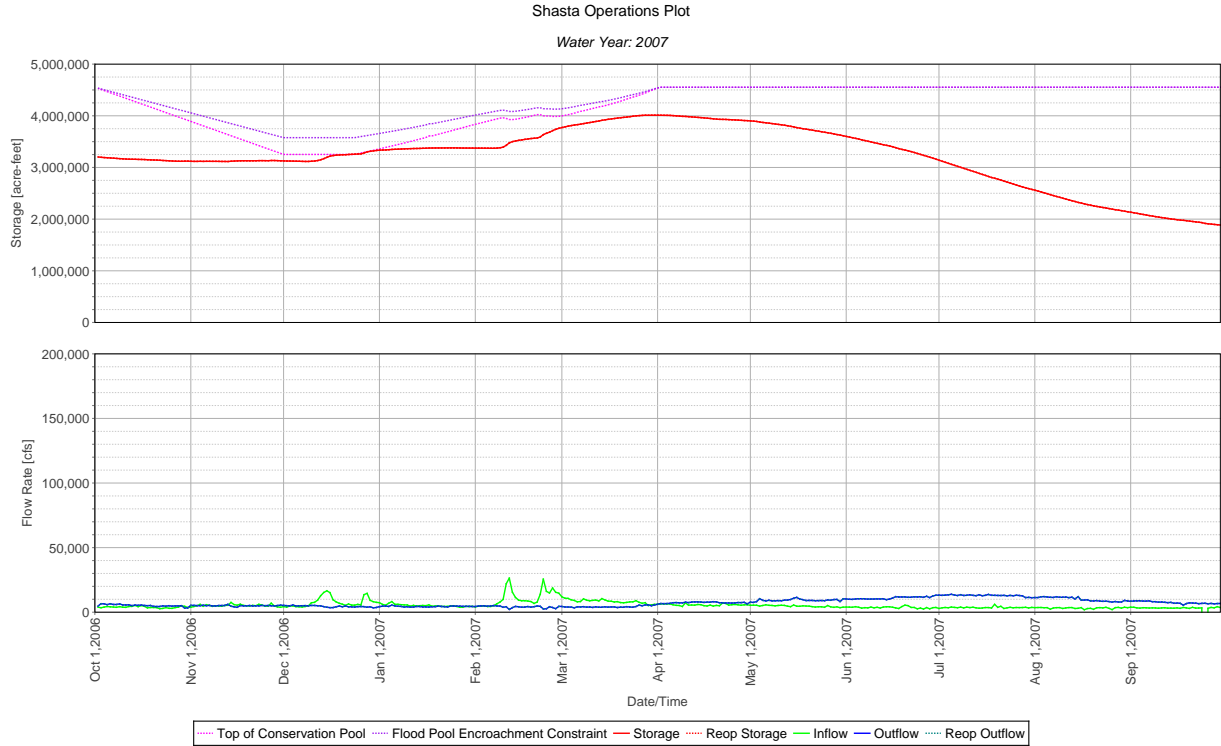
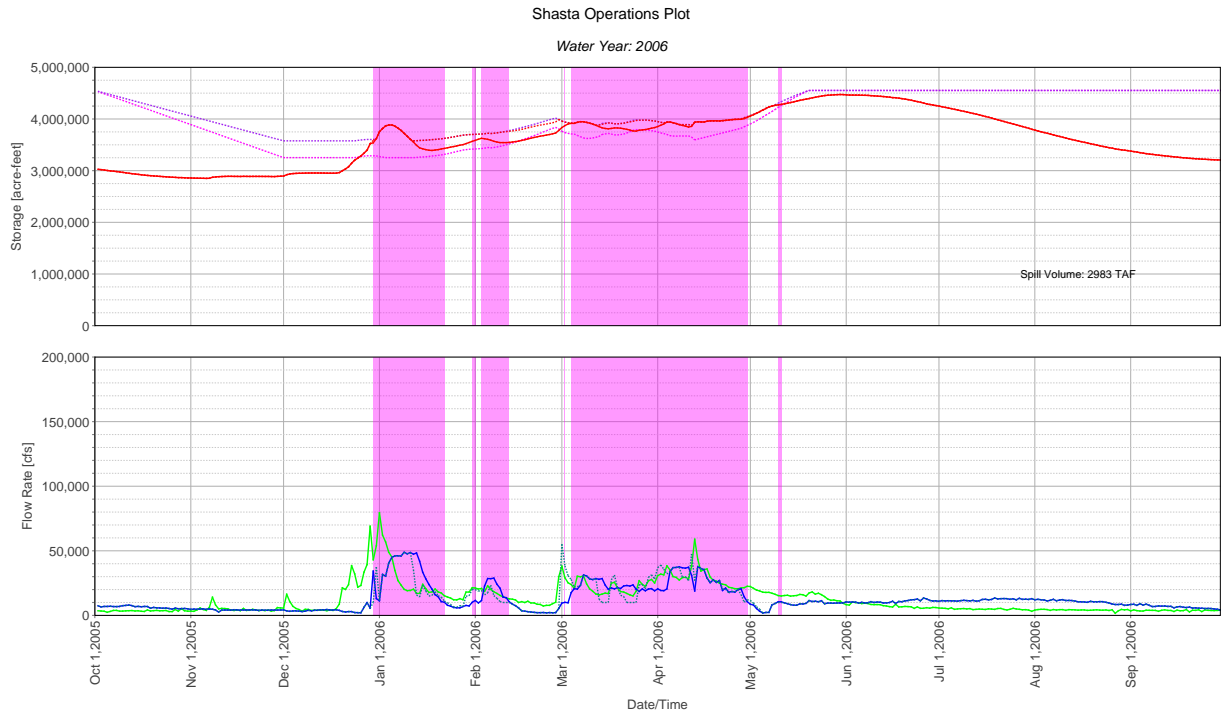
A: Reoperation Plots



A: Reoperation Plots



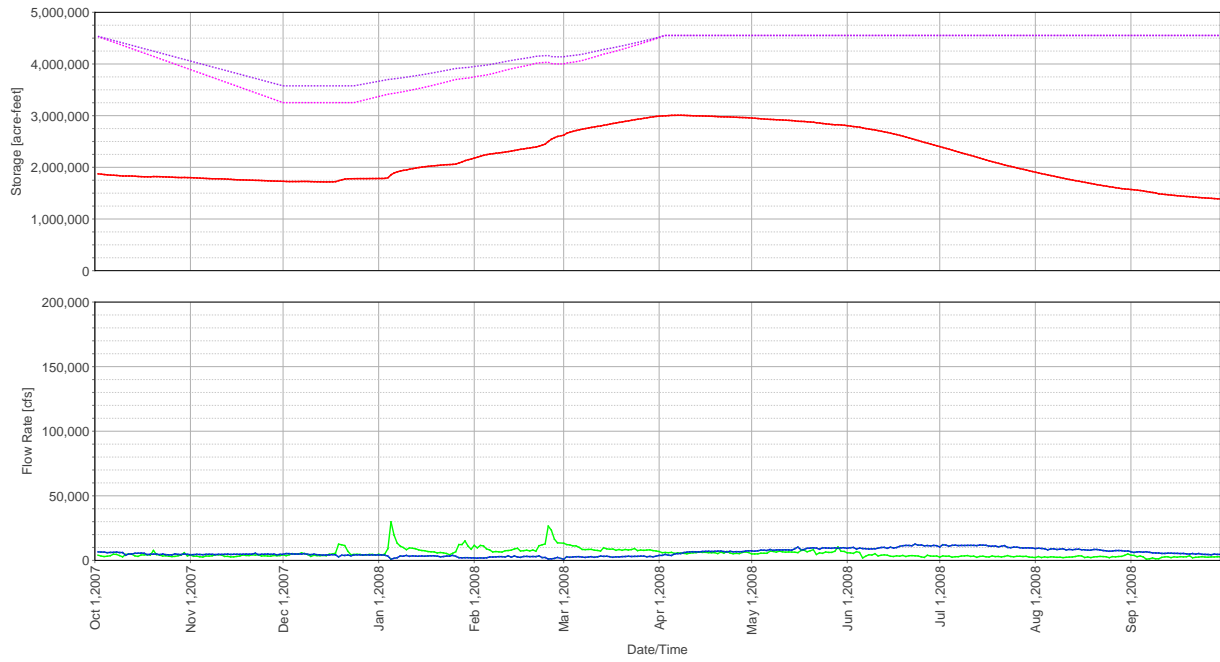
A: Reoperation Plots



A: Reoperation Plots

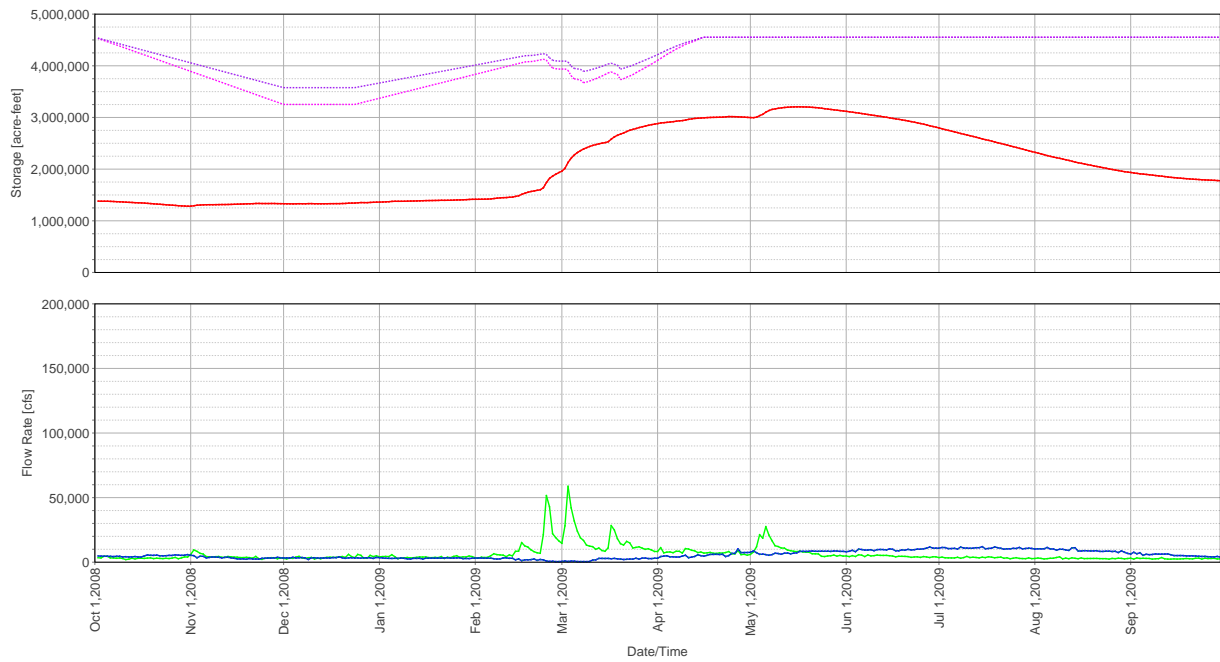
Shasta Operations Plot

Water Year: 2008



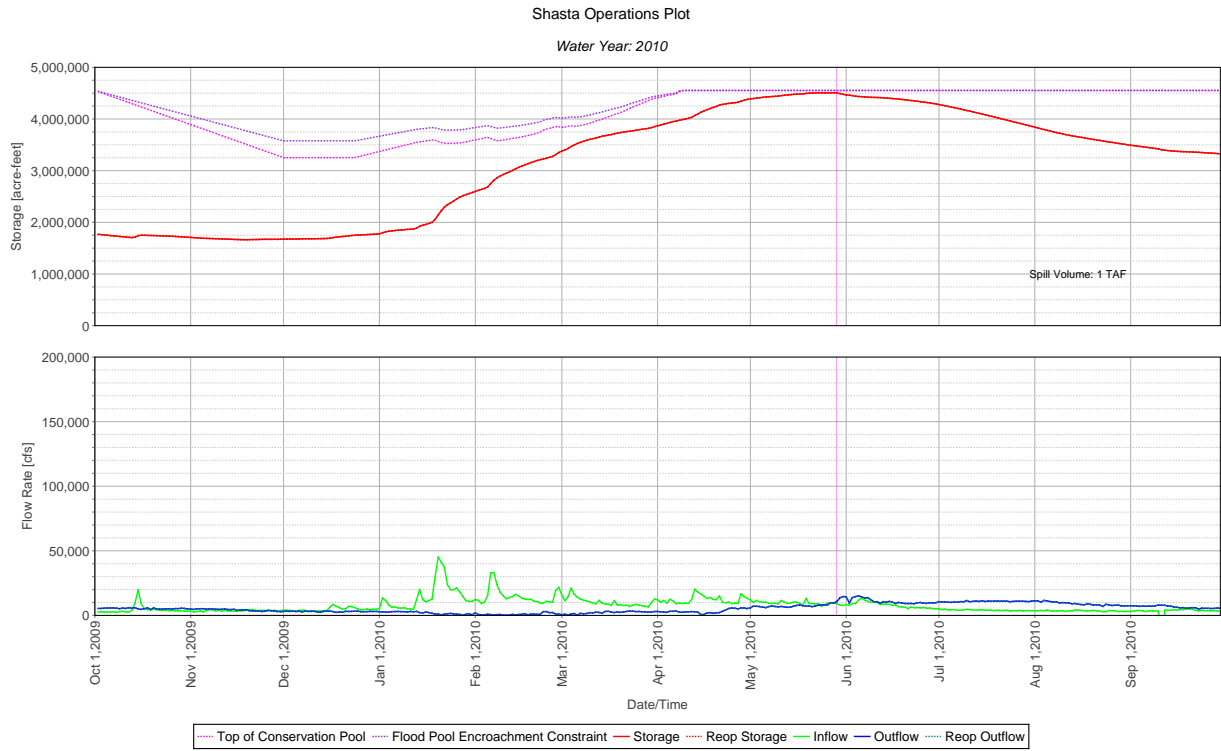
Shasta Operations Plot

Water Year: 2009



..... Top of Conservation Pool
 - - - - Flood Pool Encroachment Constraint
 ——— Storage
 Reop Storage
 ——— Inflow
 ——— Outflow
 Reop Outflow

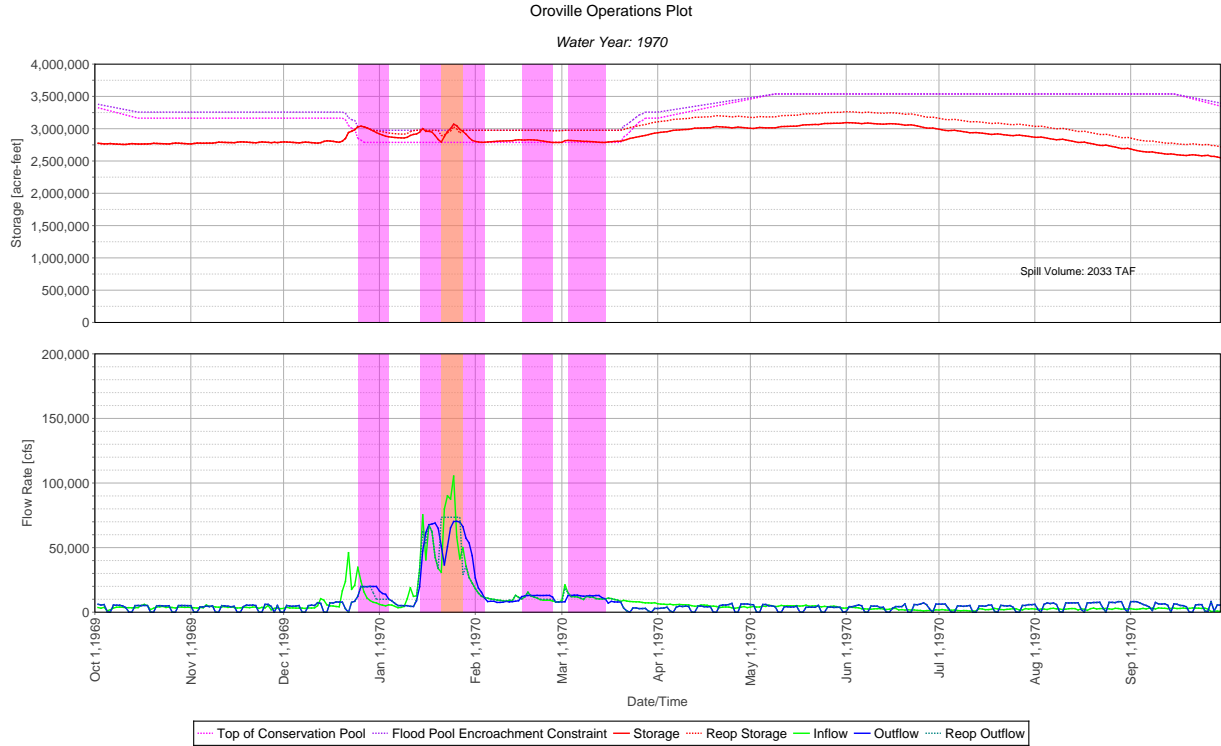
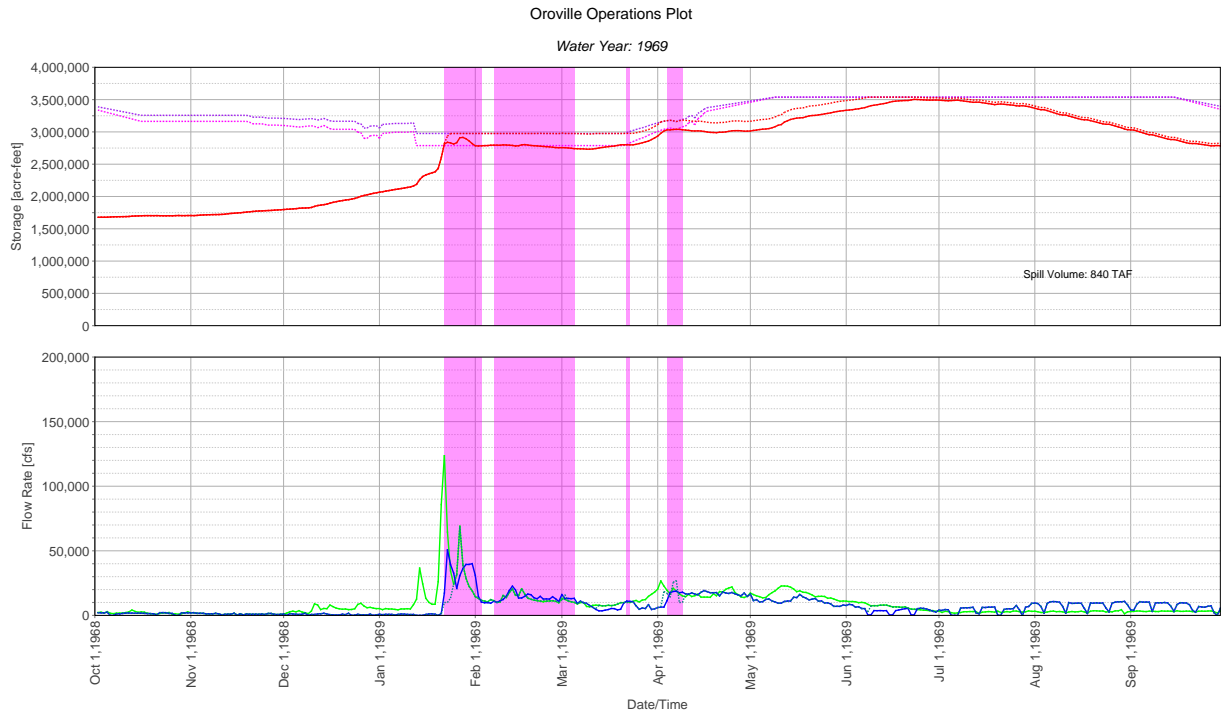
A: Reoperation Plots



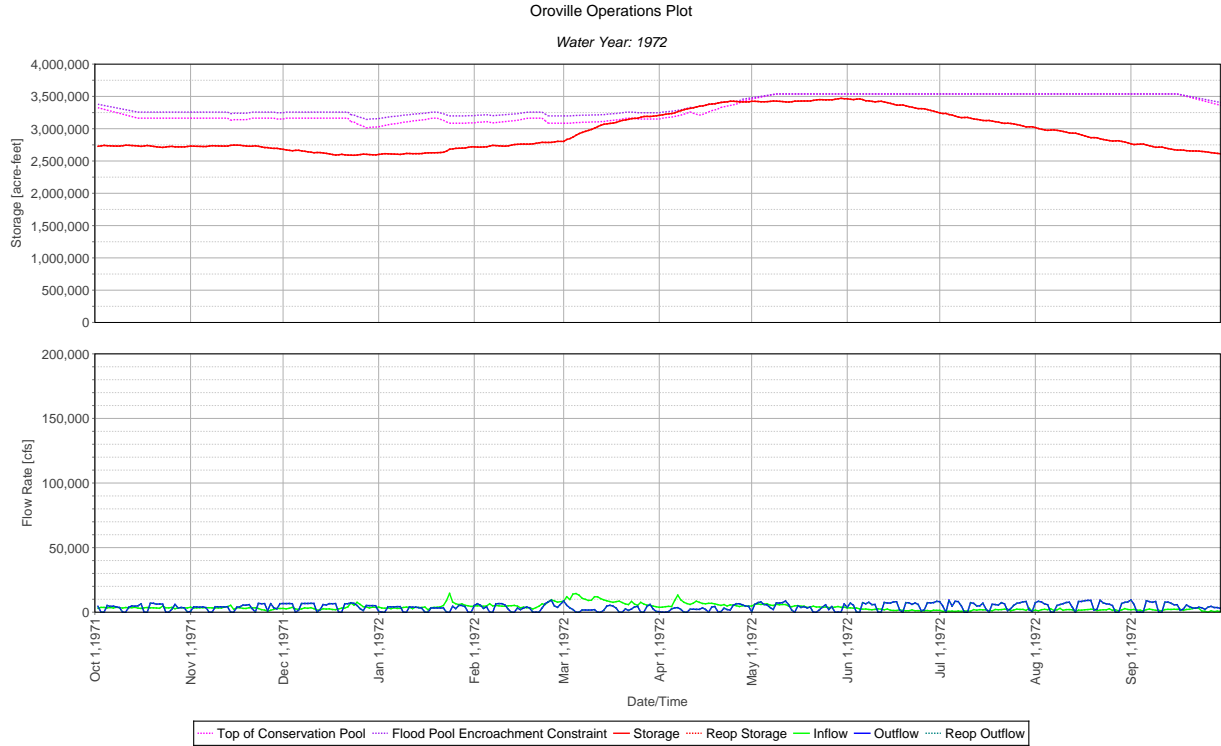
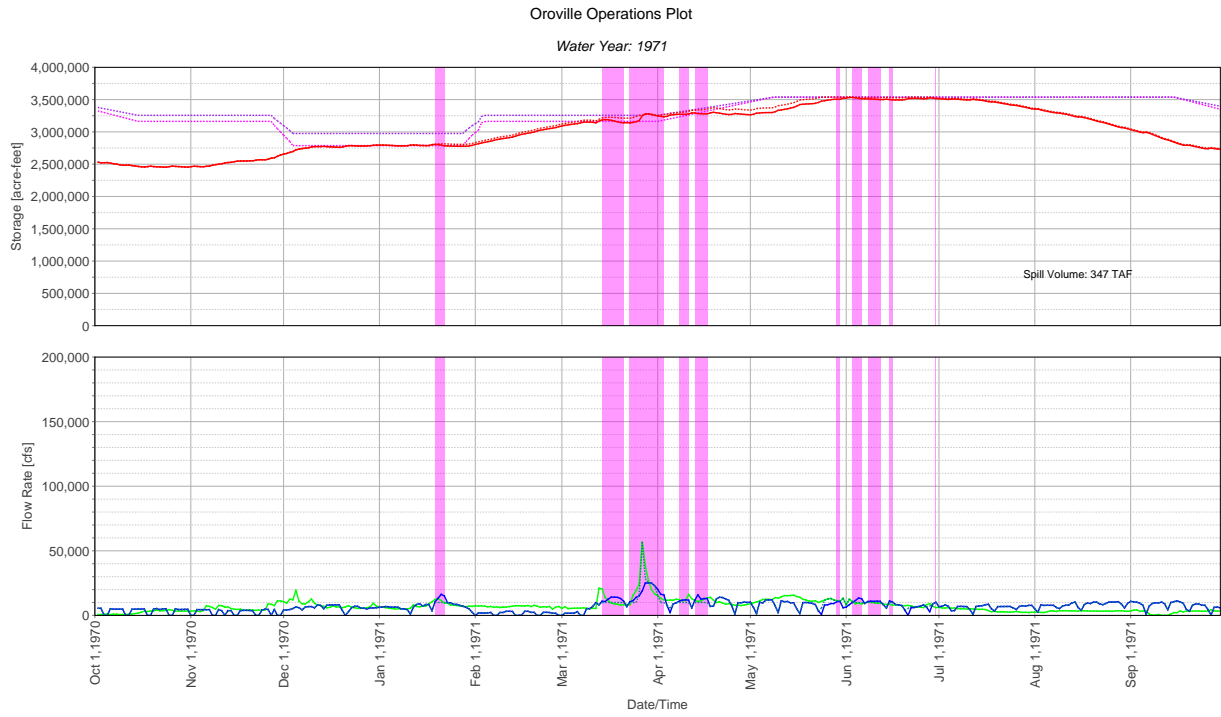
A.2 Oroville

The following section includes the reoperation plots for Oroville Dam.

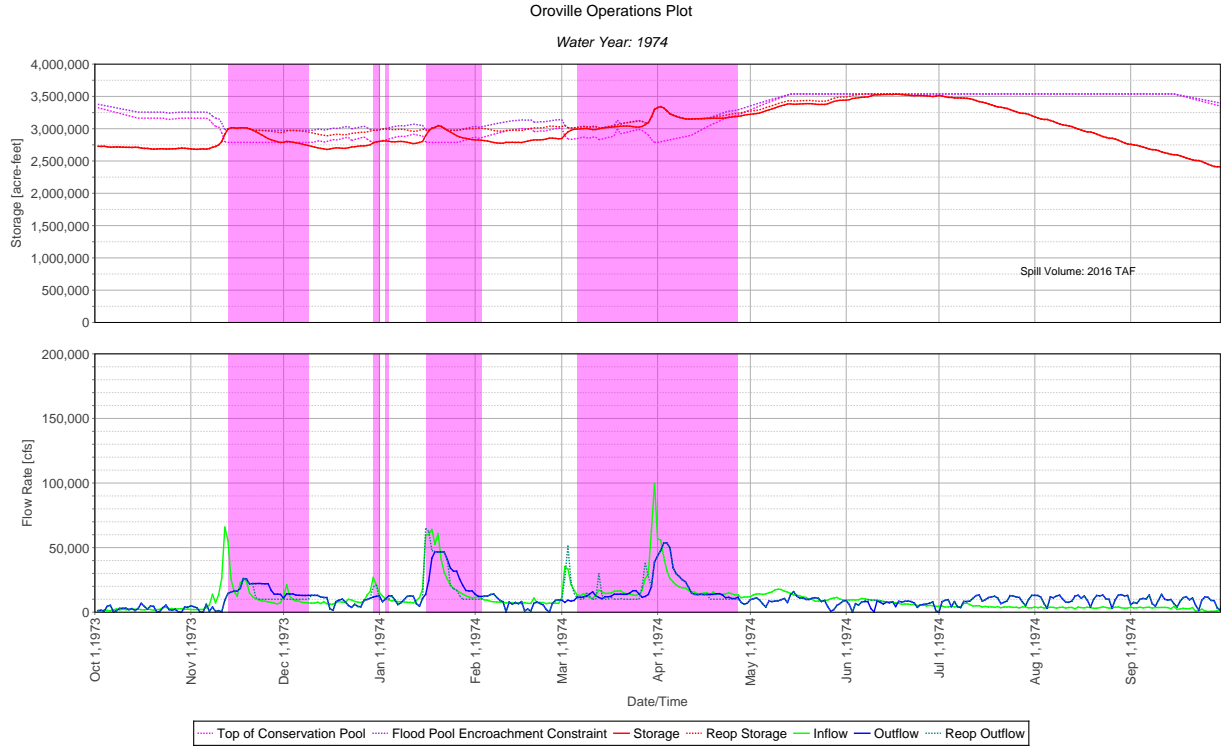
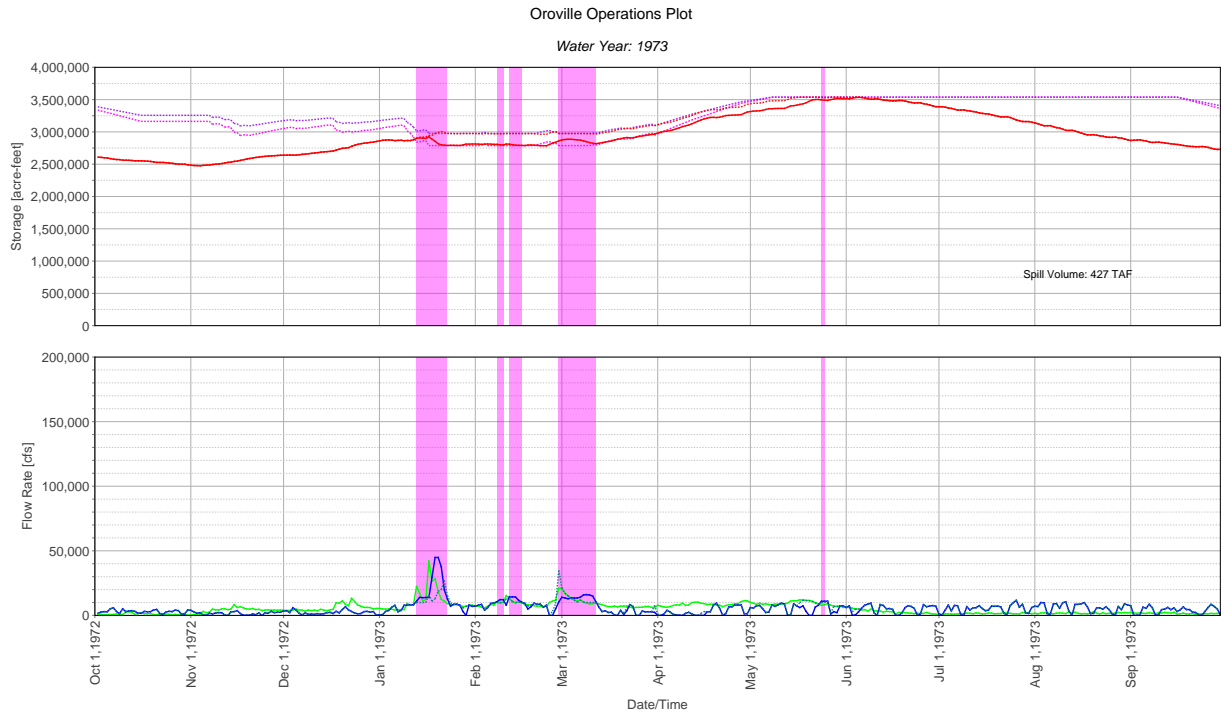
A: Reoperation Plots



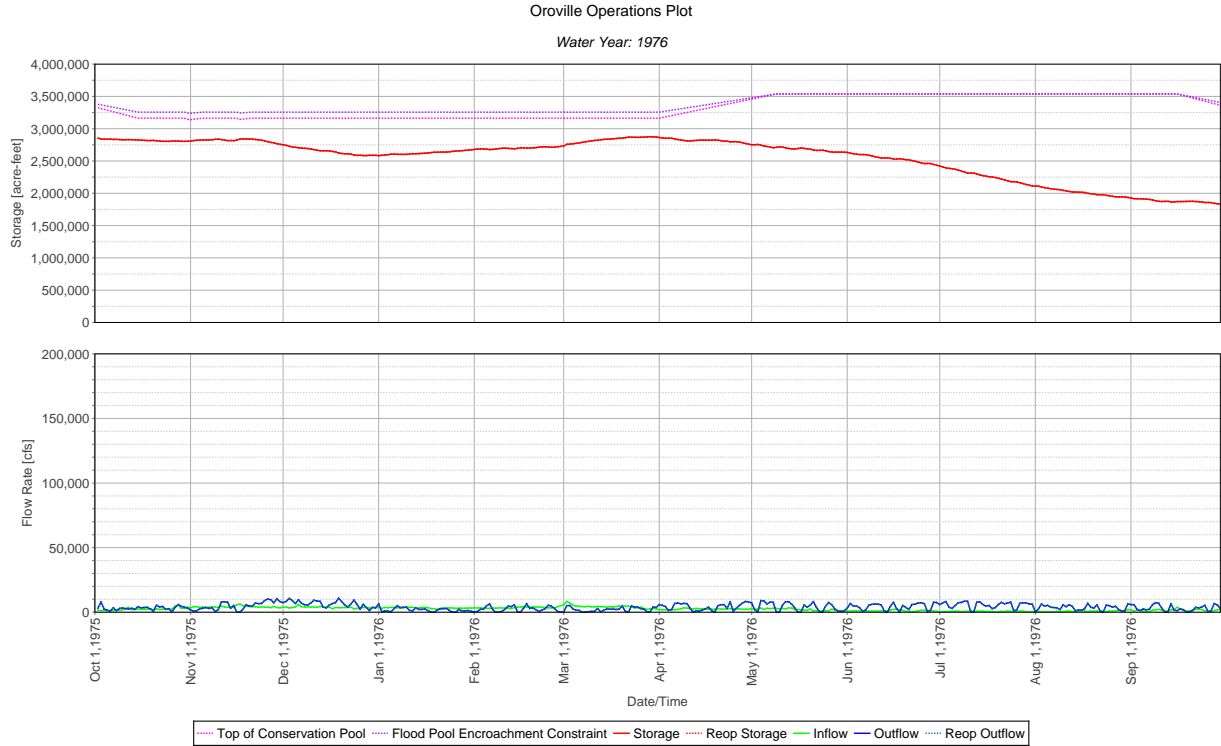
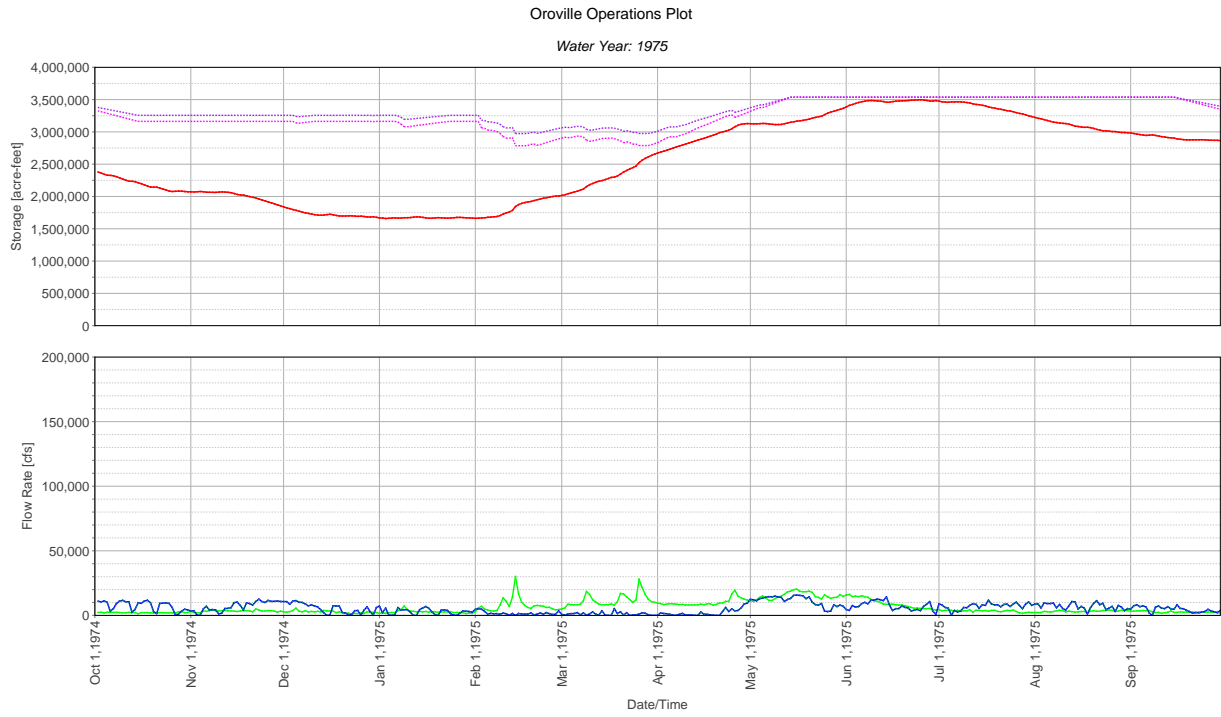
A: Reoperation Plots



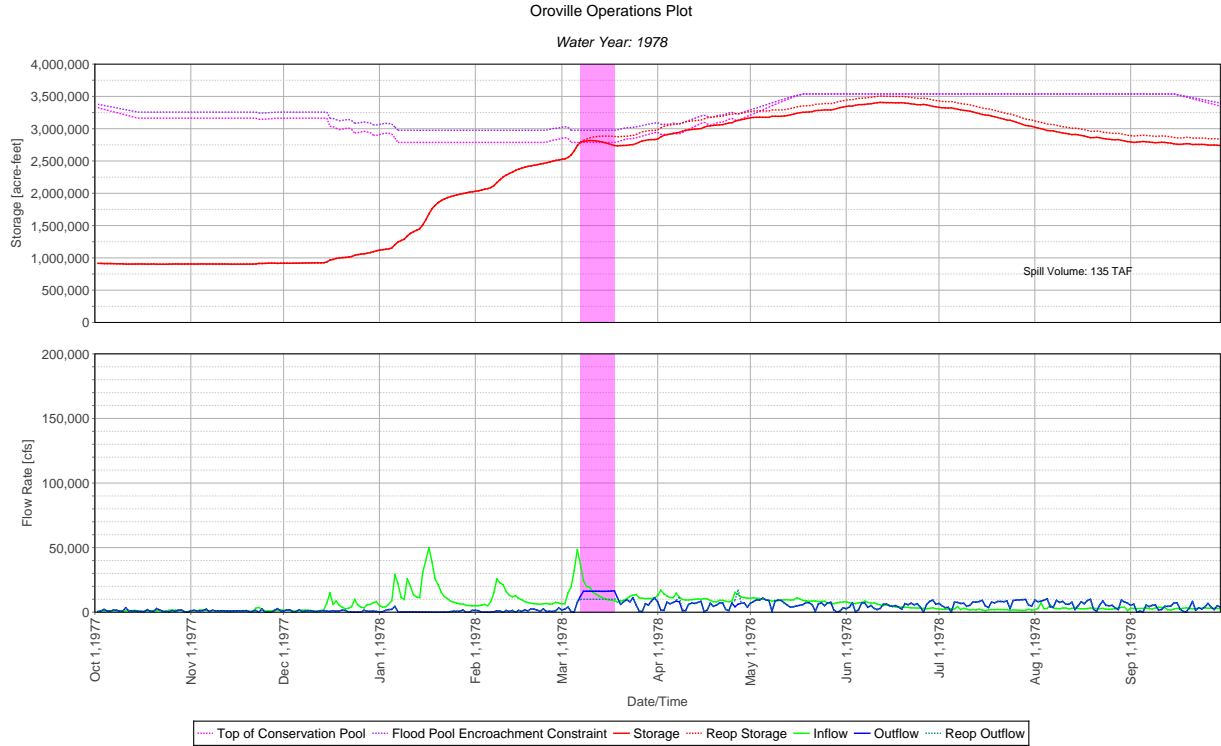
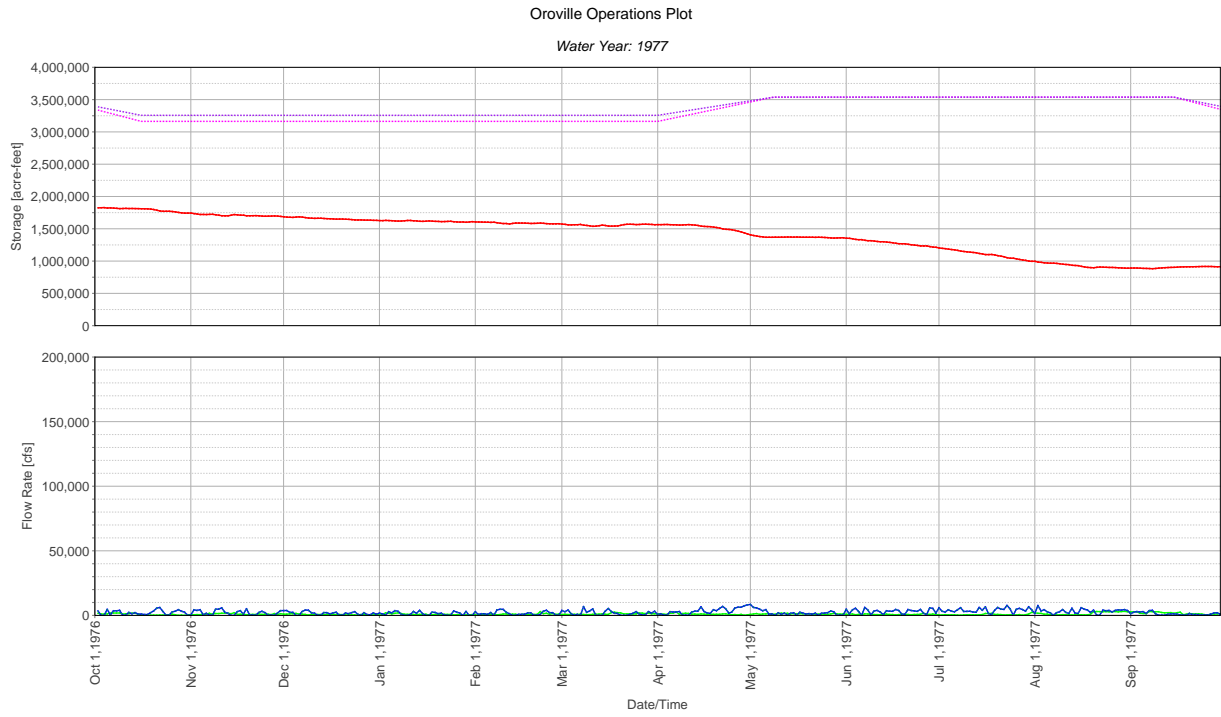
A: Reoperation Plots



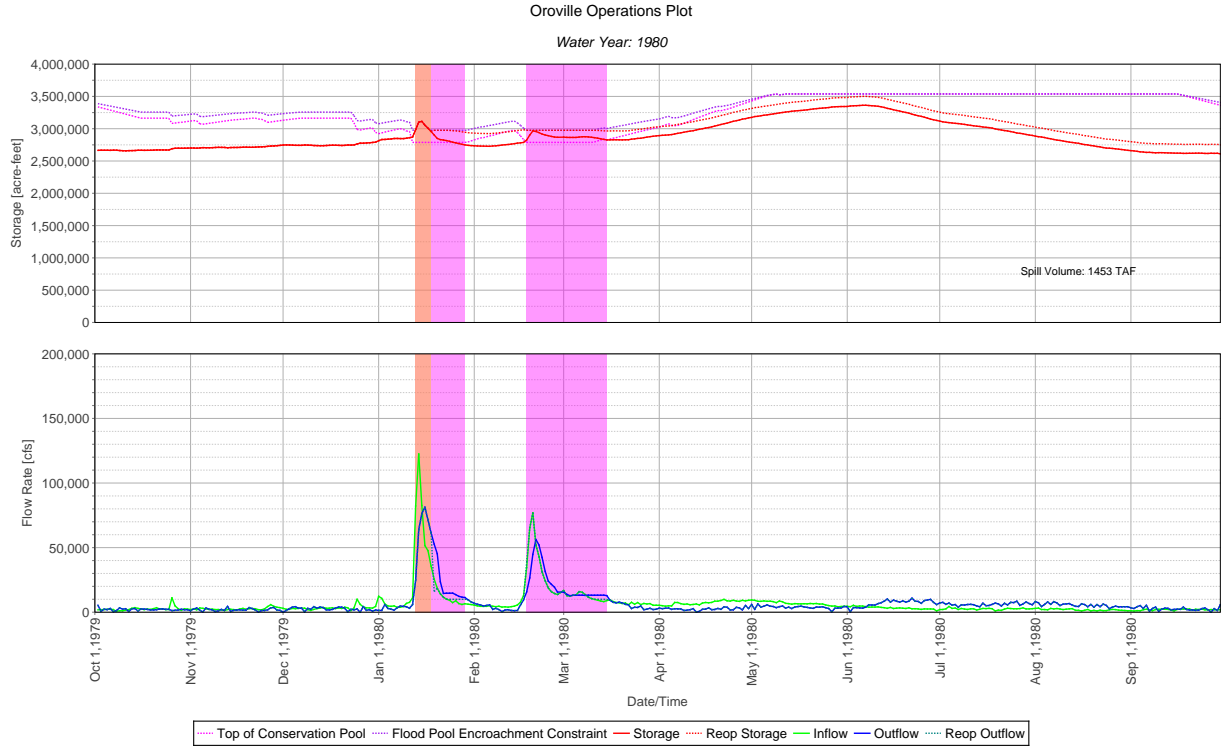
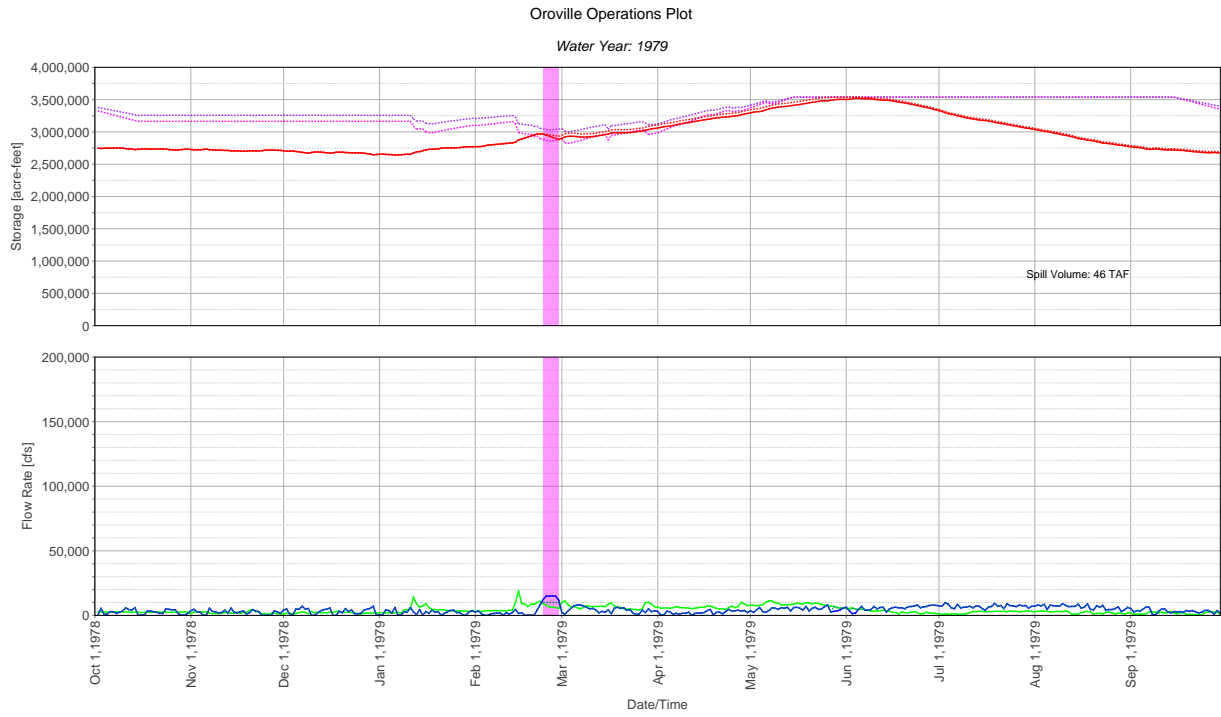
A: Reoperation Plots



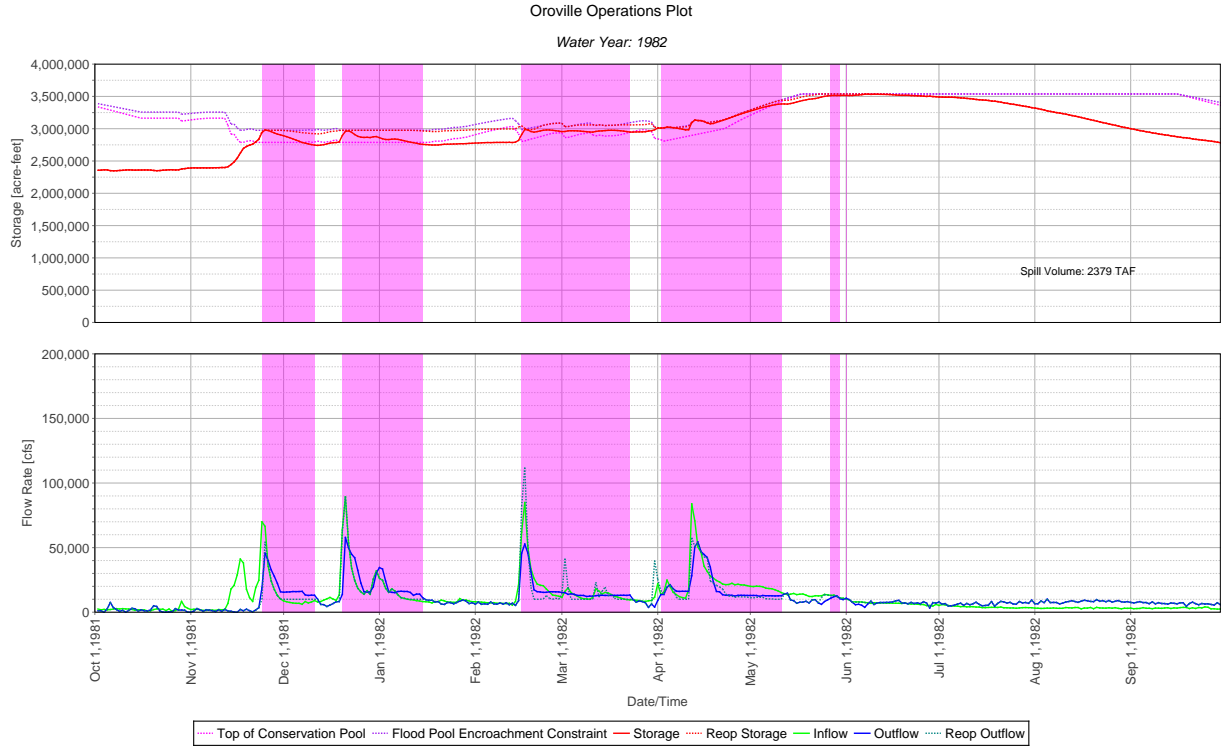
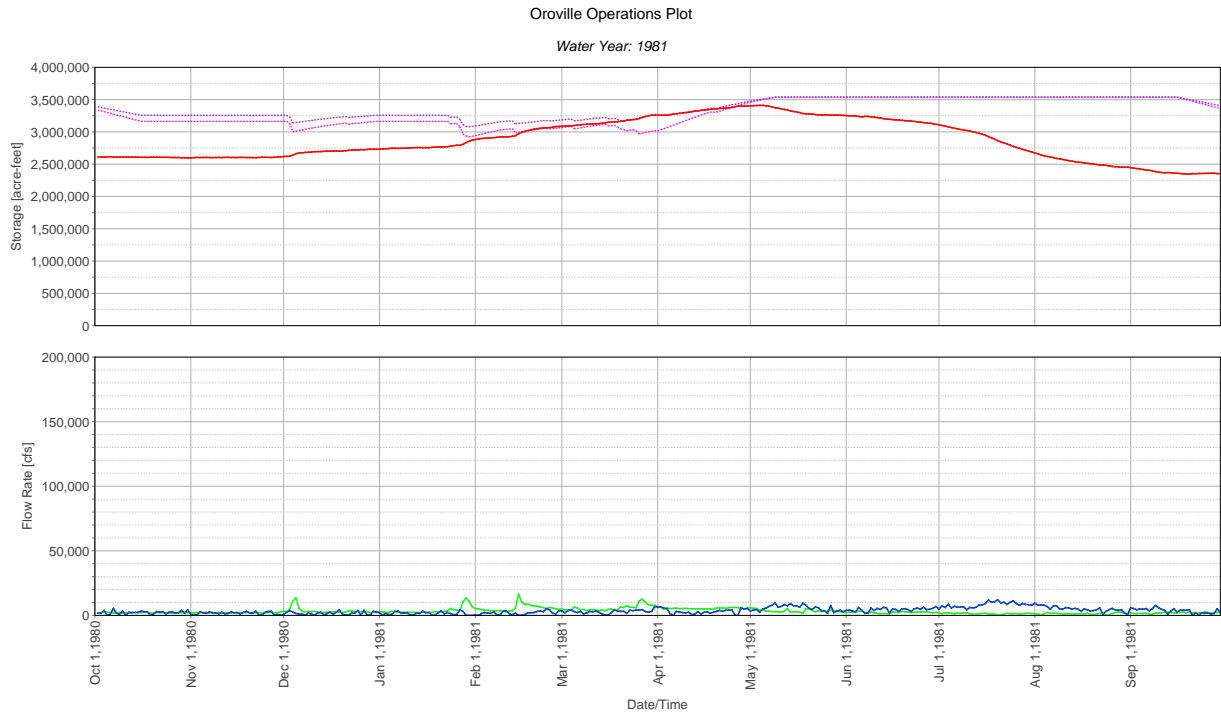
A: Reoperation Plots



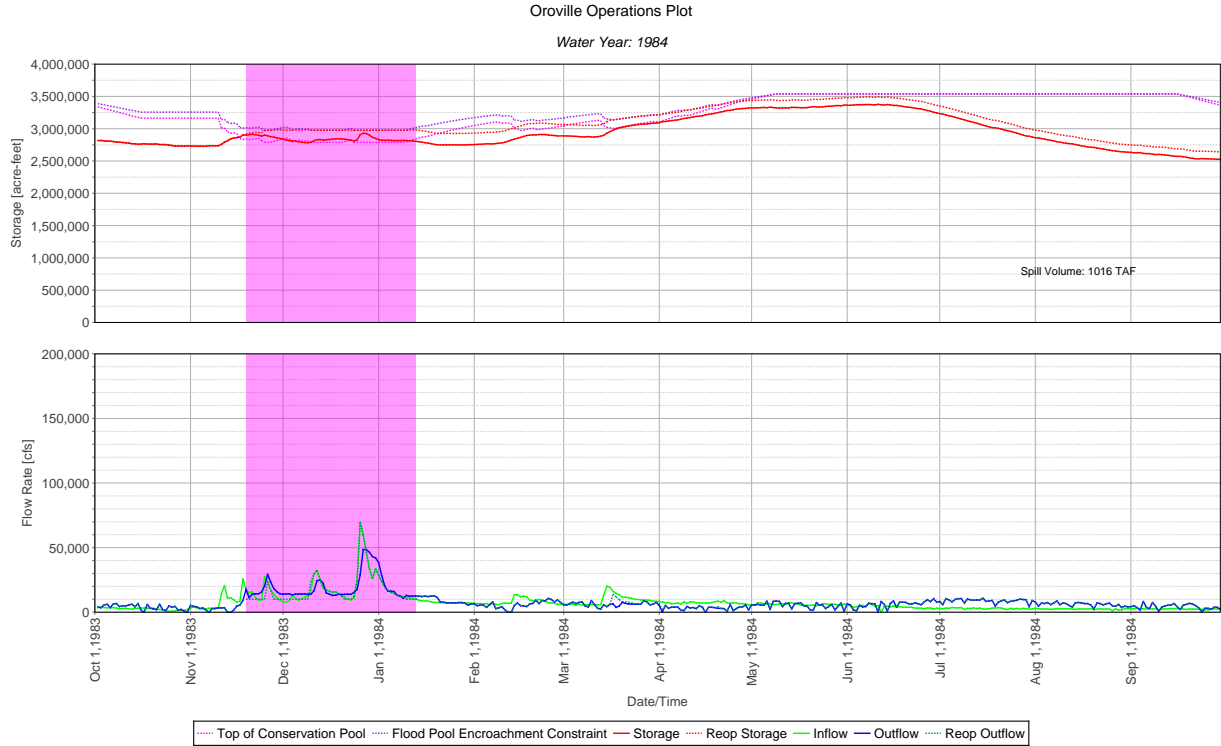
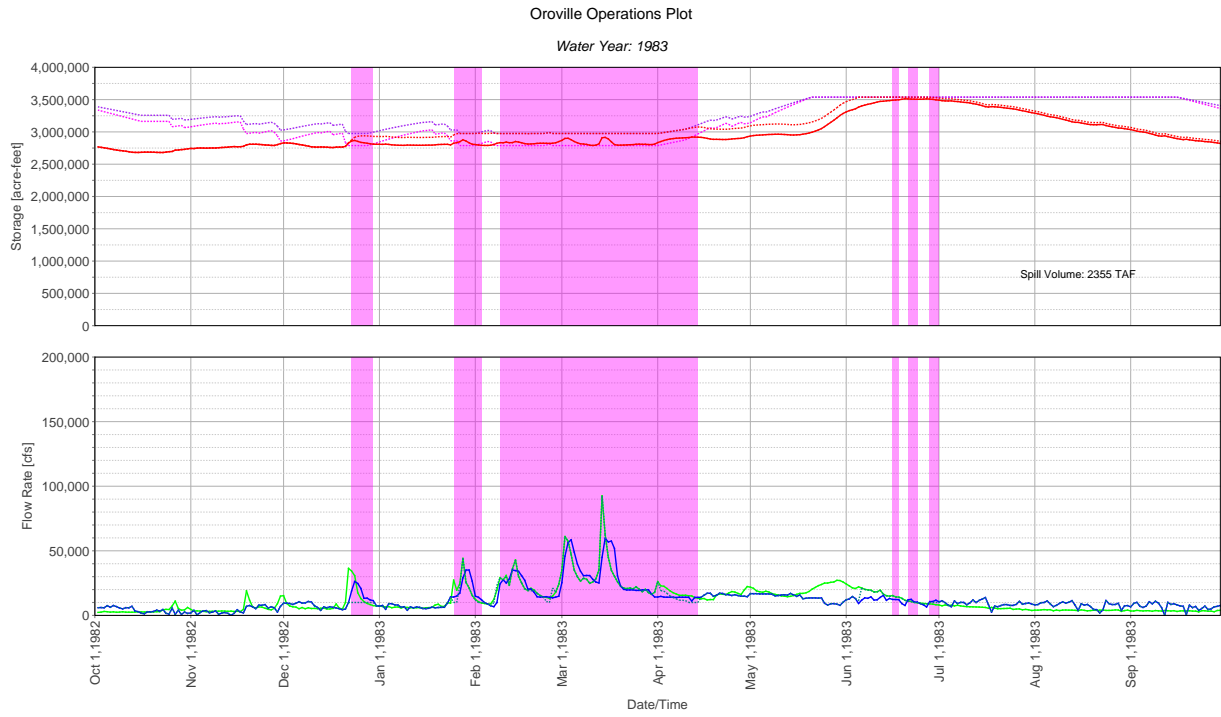
A: Reoperation Plots



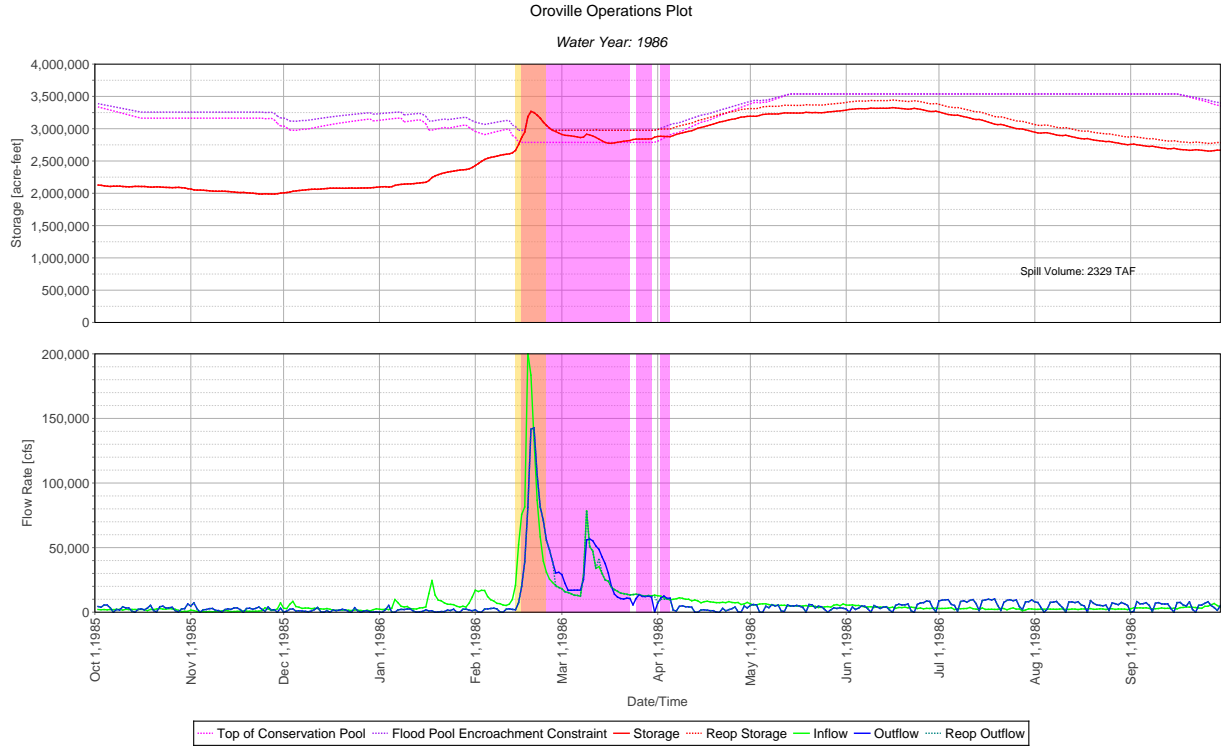
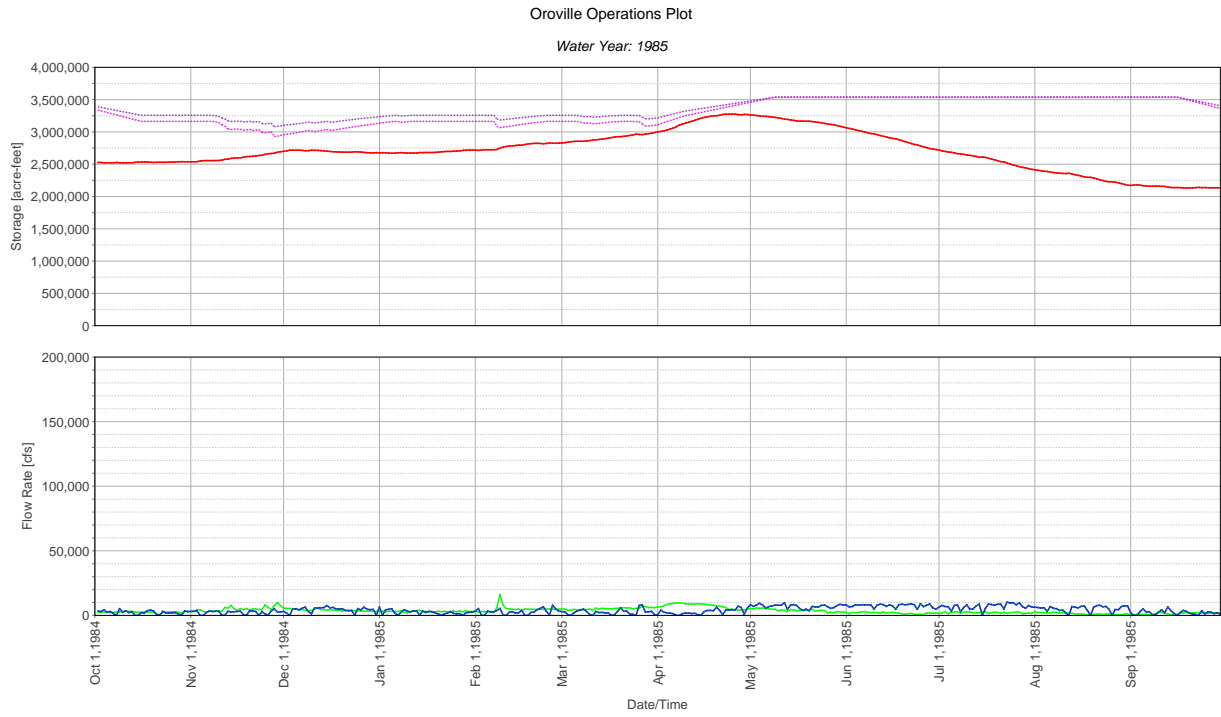
A: Reoperation Plots



A: Reoperation Plots



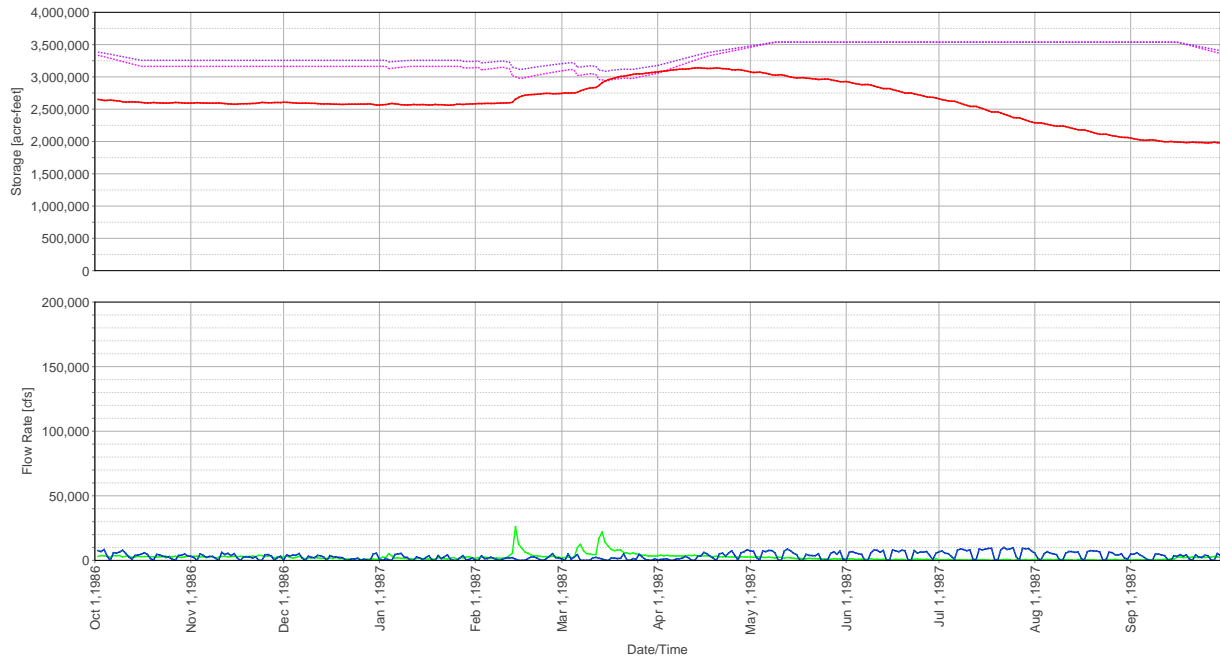
A: Reoperation Plots



A: Reoperation Plots

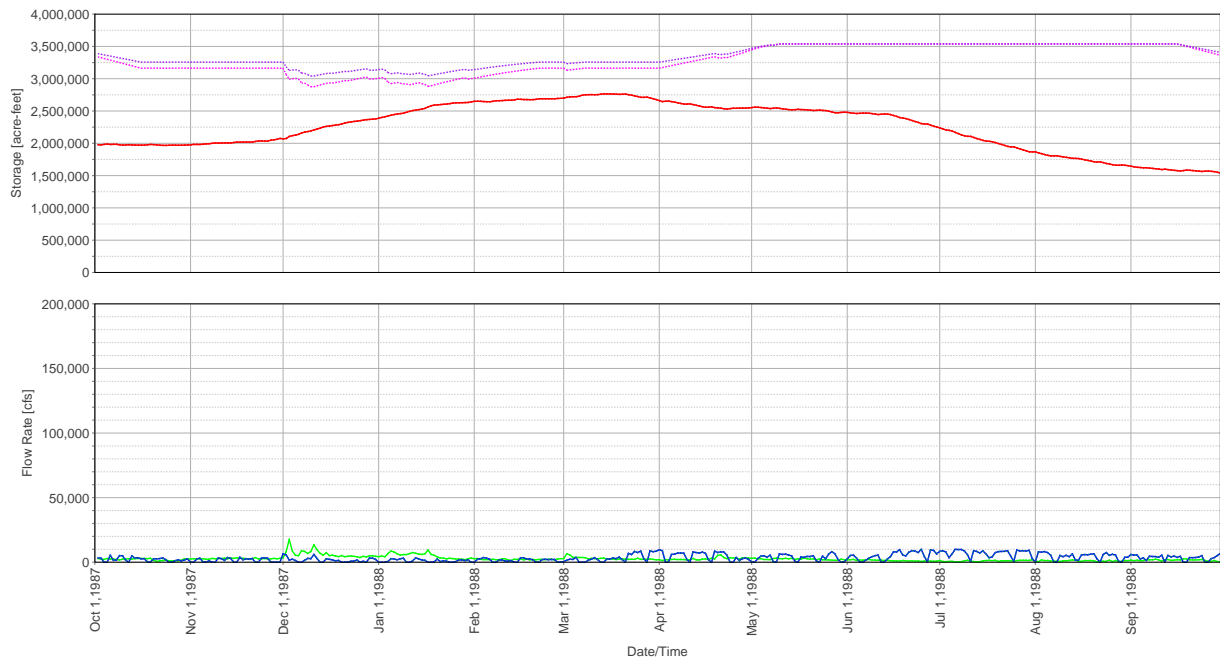
Oroville Operations Plot

Water Year: 1987



Oroville Operations Plot

Water Year: 1988

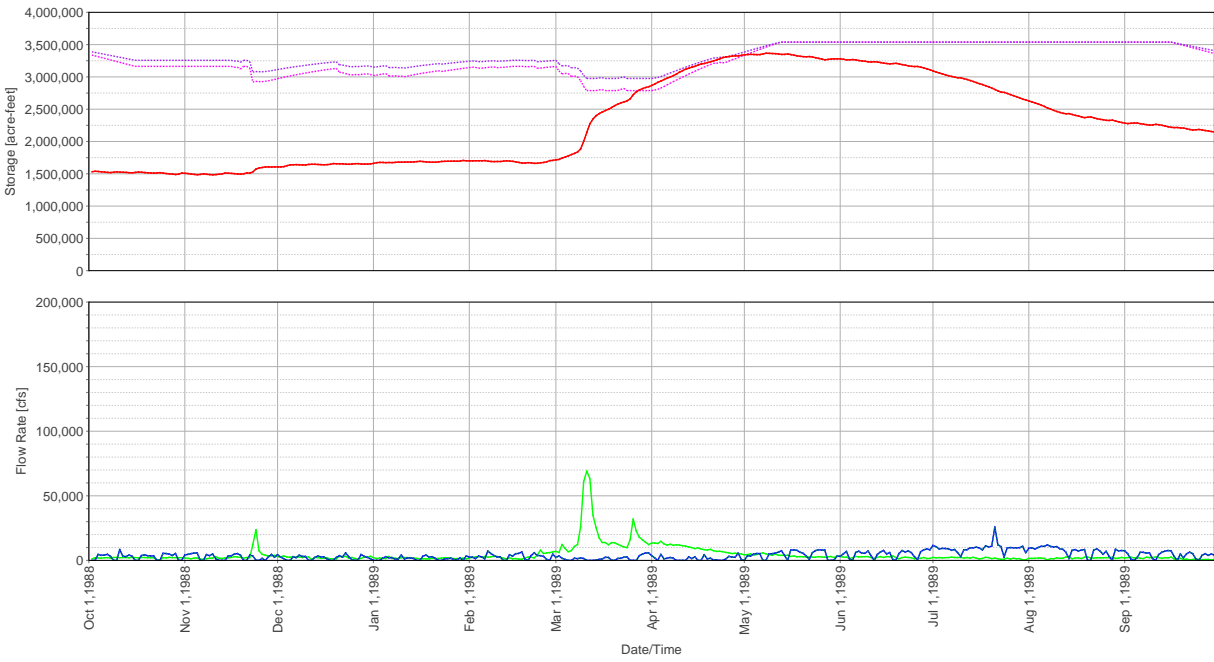


..... Top of Conservation Pool
 - - - - Flood Pool Encroachment Constraint
 ——— Storage
 Reop Storage
 ——— Inflow
 ——— Outflow
 Reop Outflow

A: Reoperation Plots

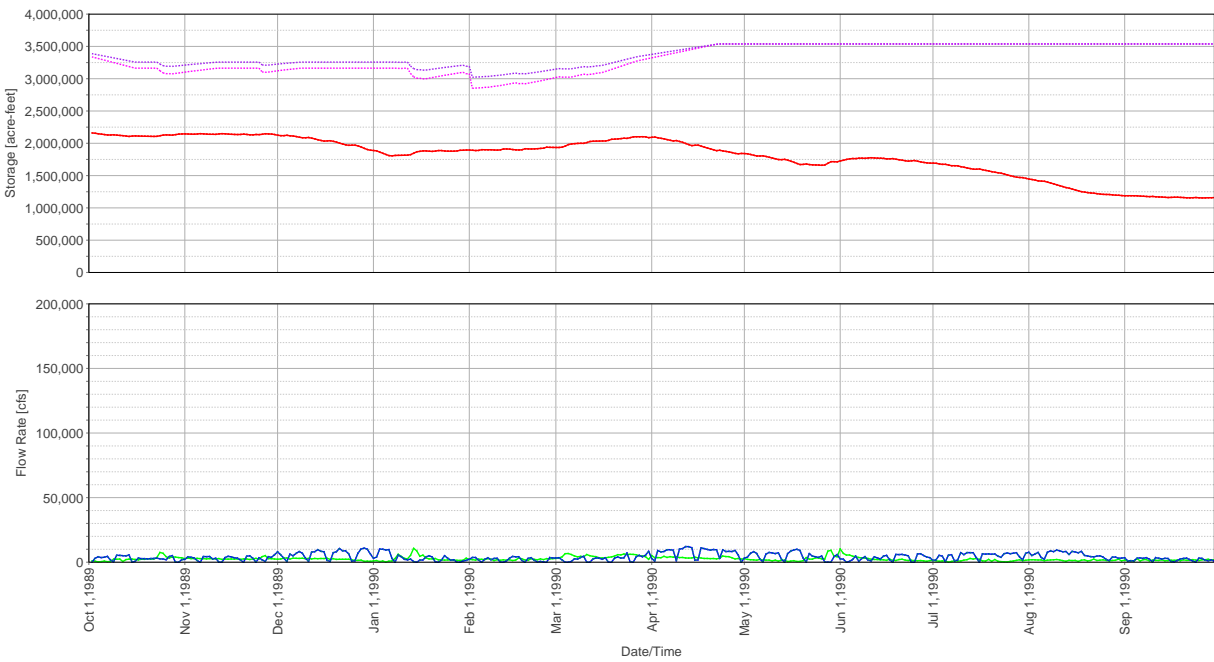
Oroville Operations Plot

Water Year: 1989



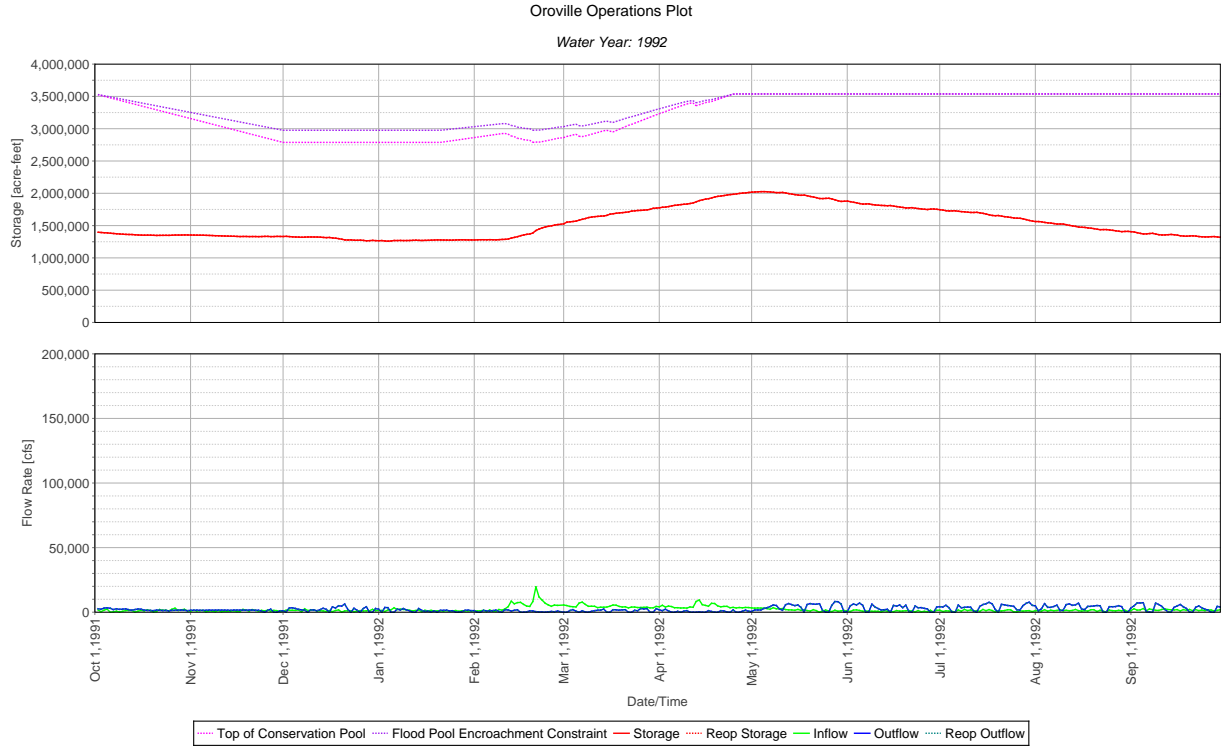
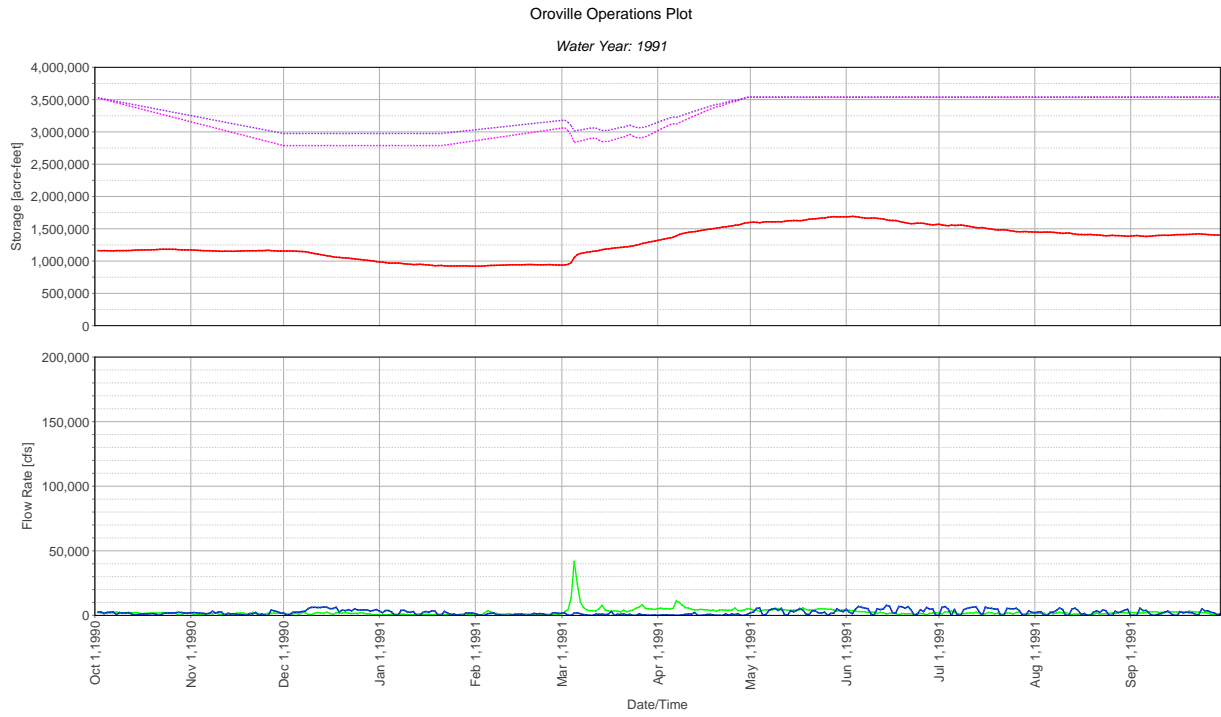
Oroville Operations Plot

Water Year: 1990

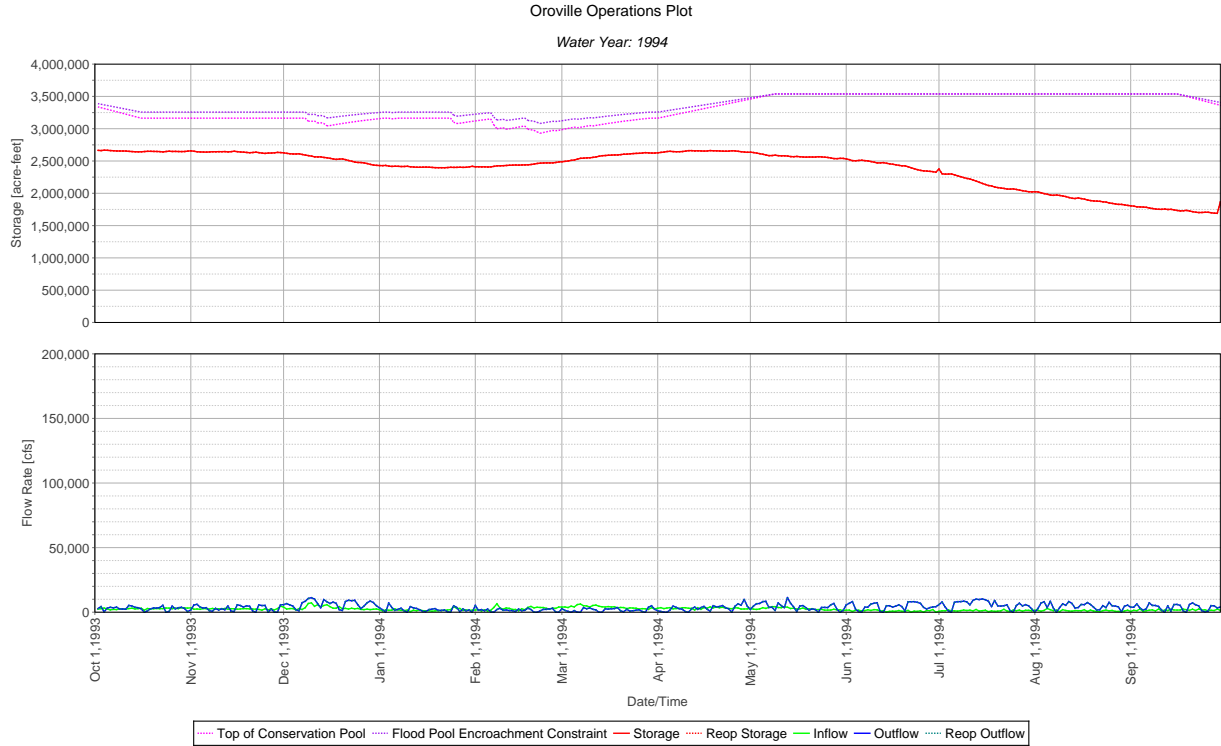
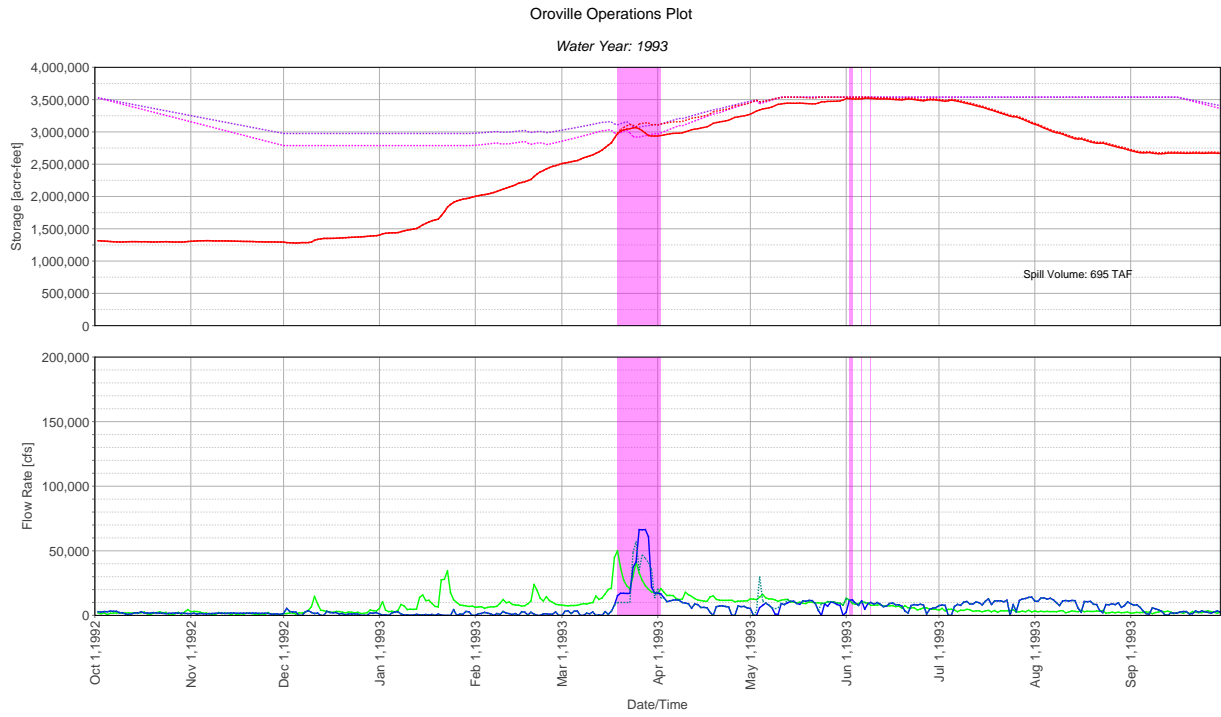


..... Top of Conservation Pool
 Flood Pool Encroachment Constraint
 —— Storage
 Reop Storage
 —— Inflow
 —— Outflow
 Reop Outflow

A: Reoperation Plots



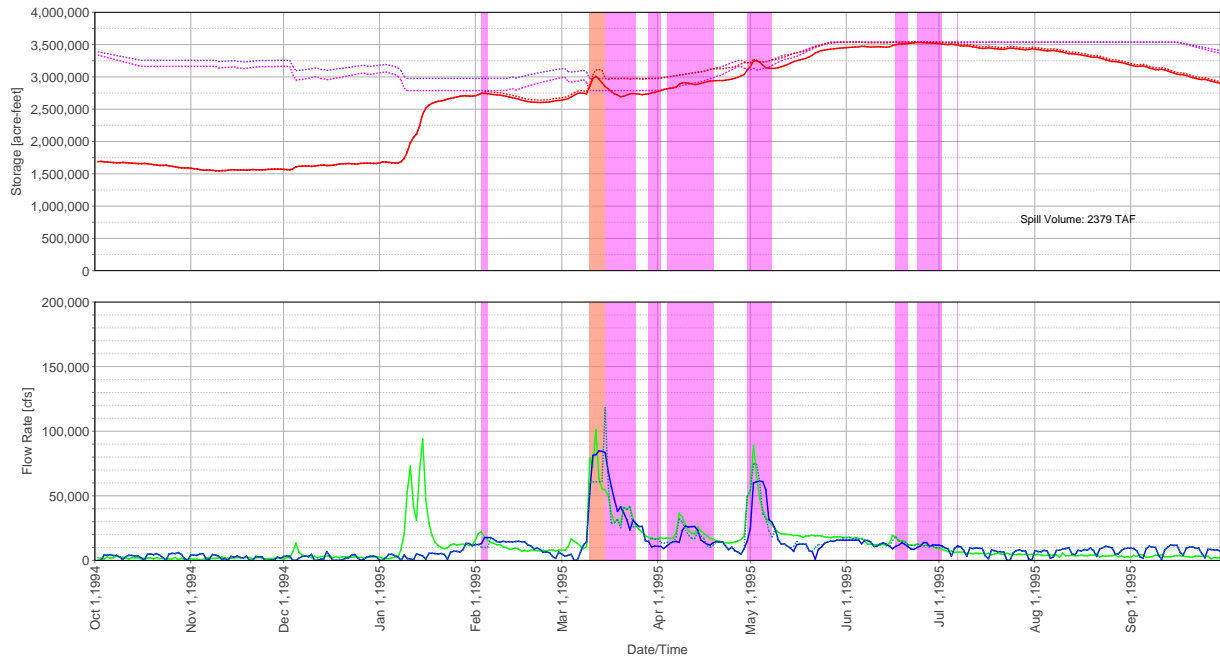
A: Reoperation Plots



A: Reoperation Plots

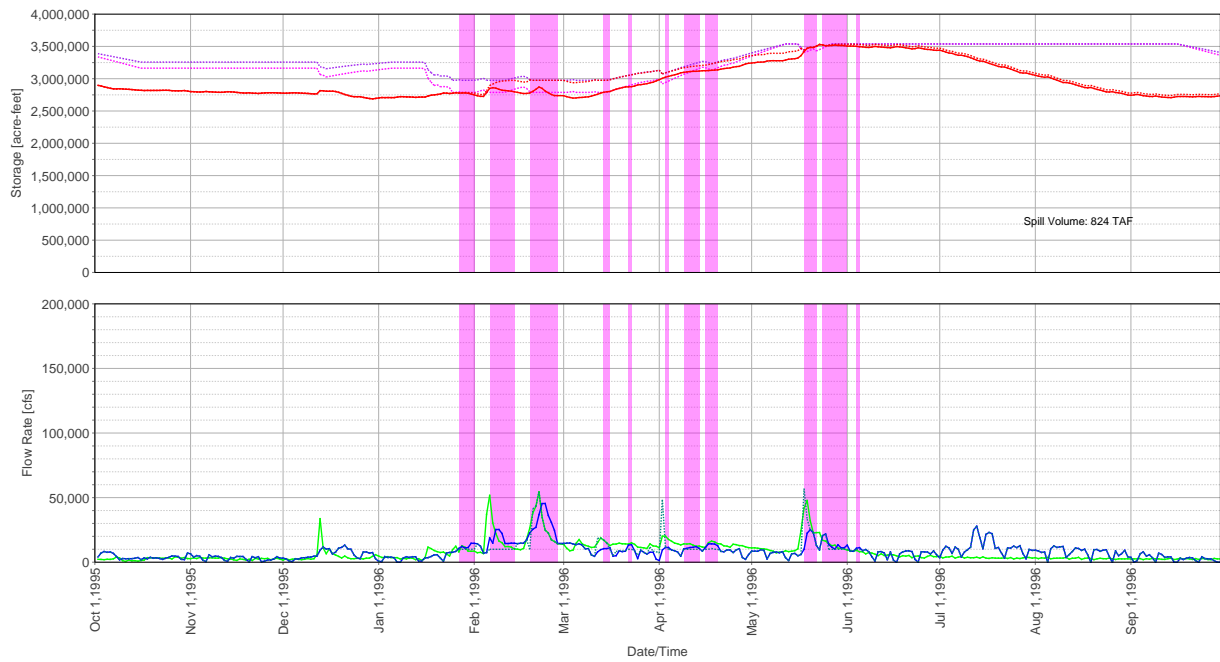
Oroville Operations Plot

Water Year: 1995



Oroville Operations Plot

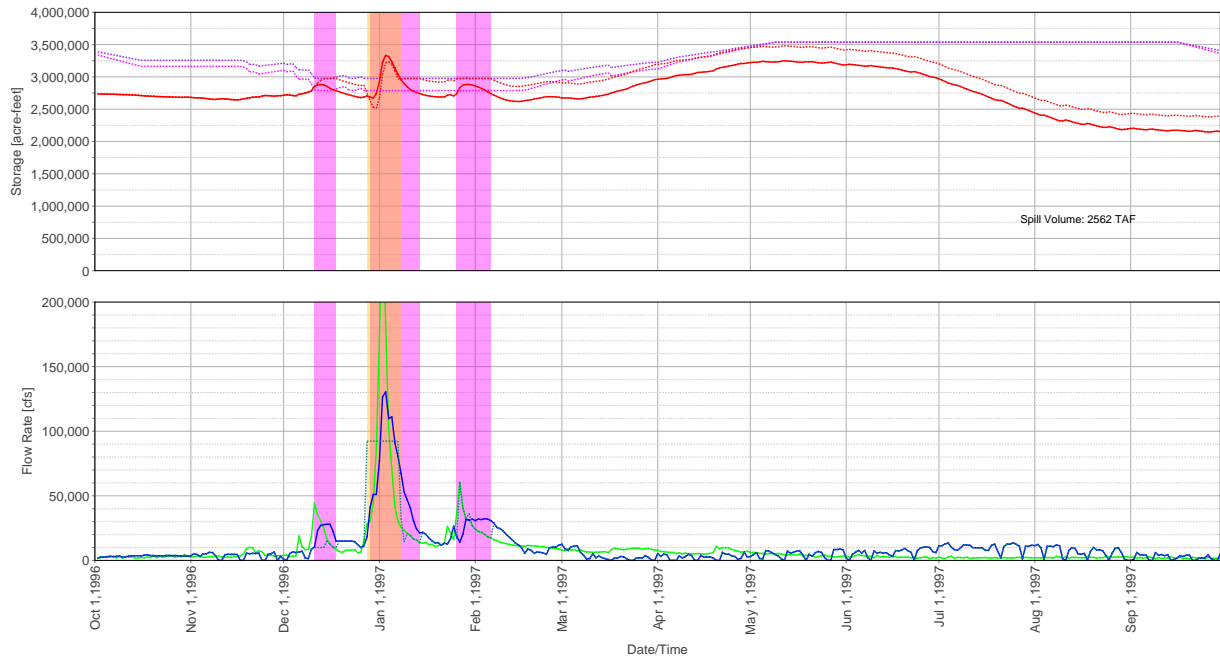
Water Year: 1996



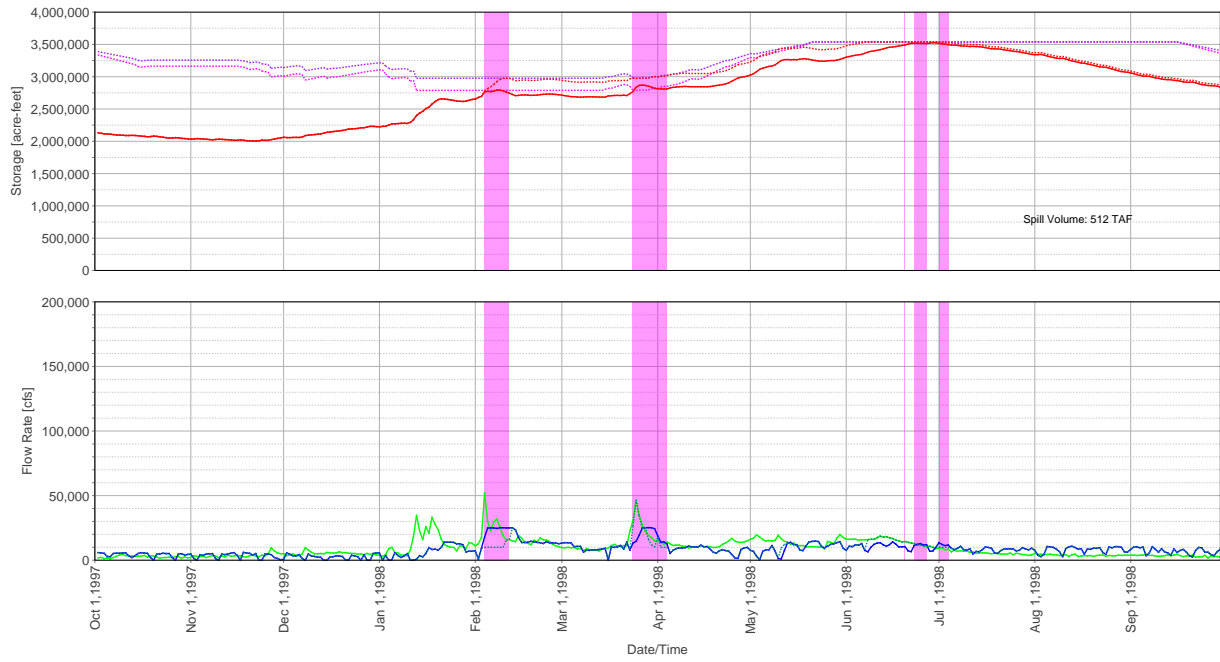
..... Top of Conservation Pool
 Flood Pool Encroachment Constraint
 —— Storage
 Reop Storage
 —— Inflow
 —— Outflow
 Reop Outflow

A: Reoperation Plots

Oroville Operations Plot
Water Year: 1997

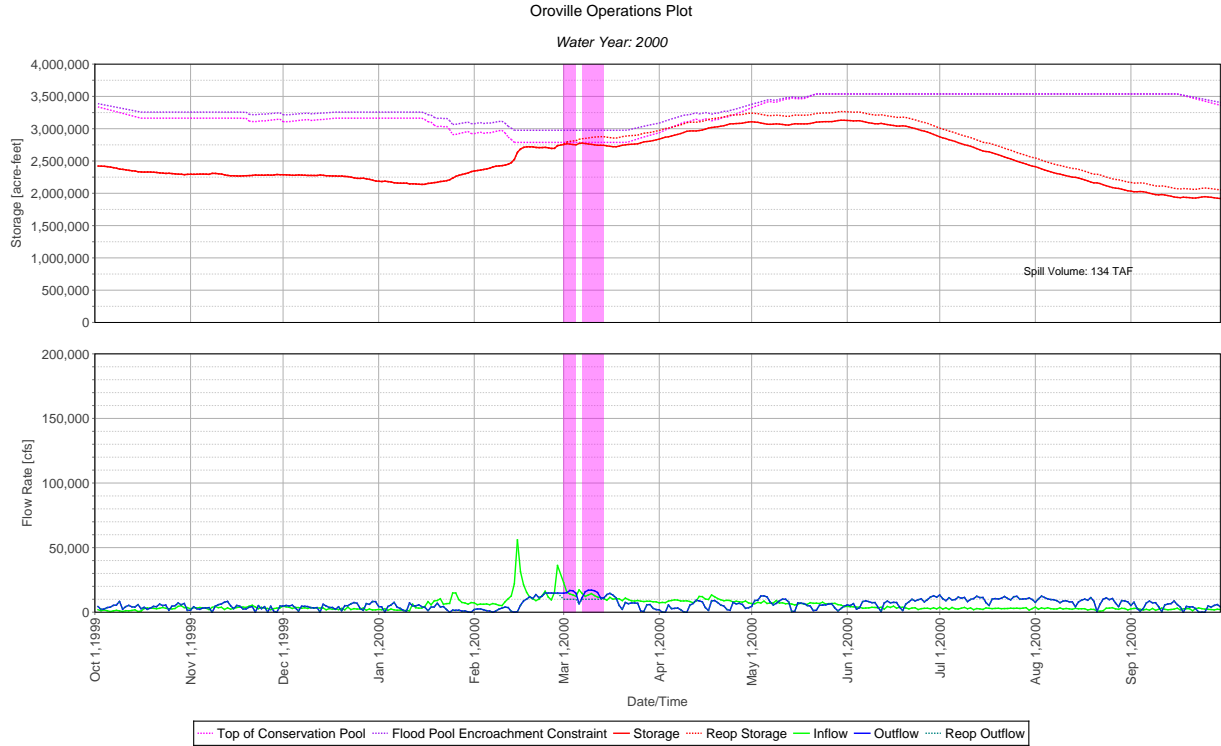
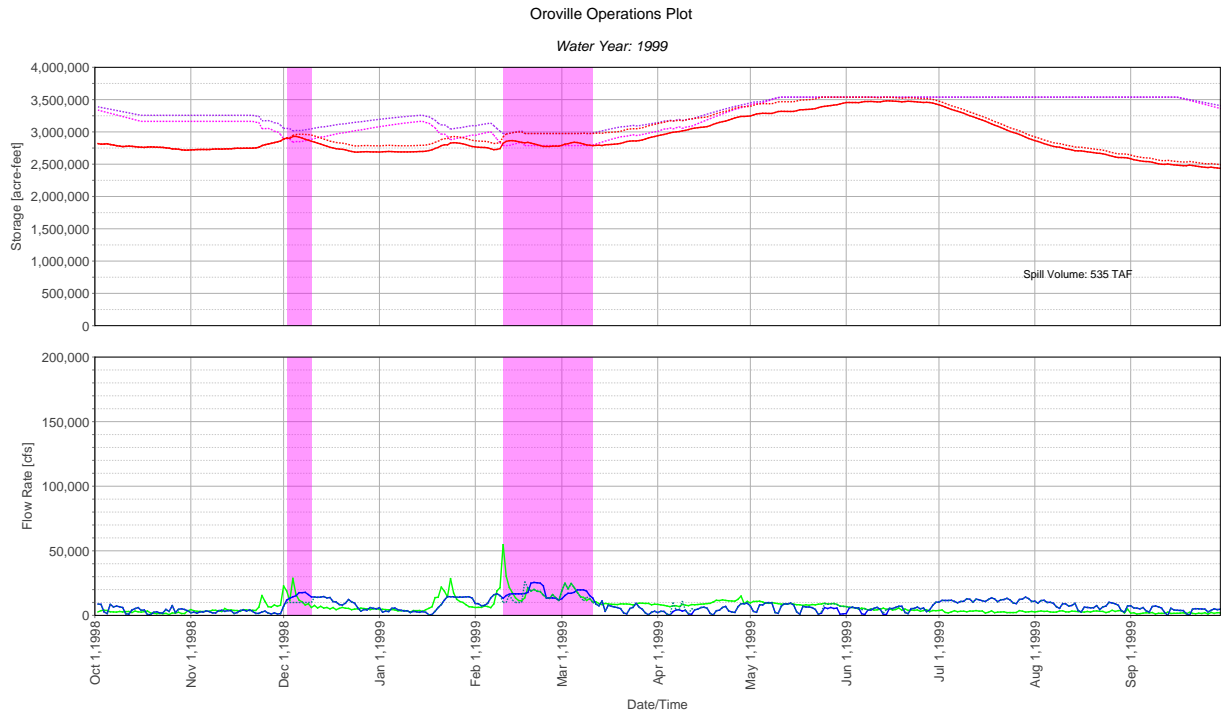


Oroville Operations Plot
Water Year: 1998

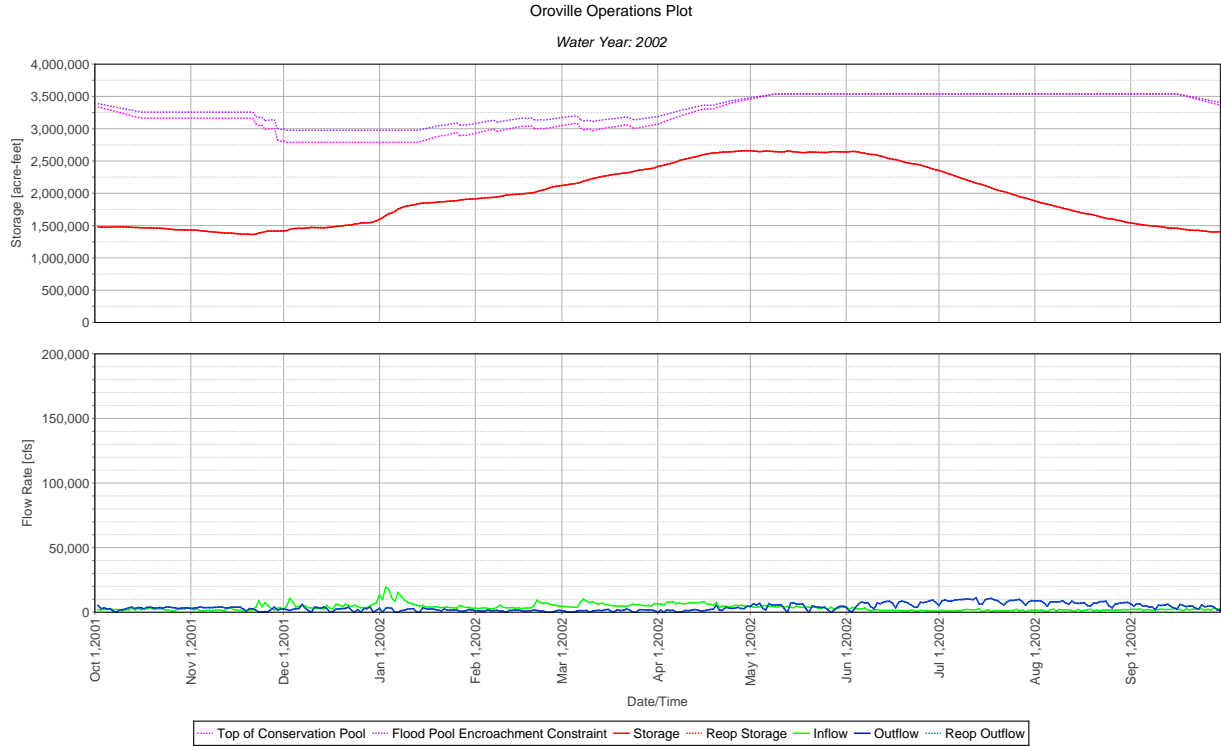
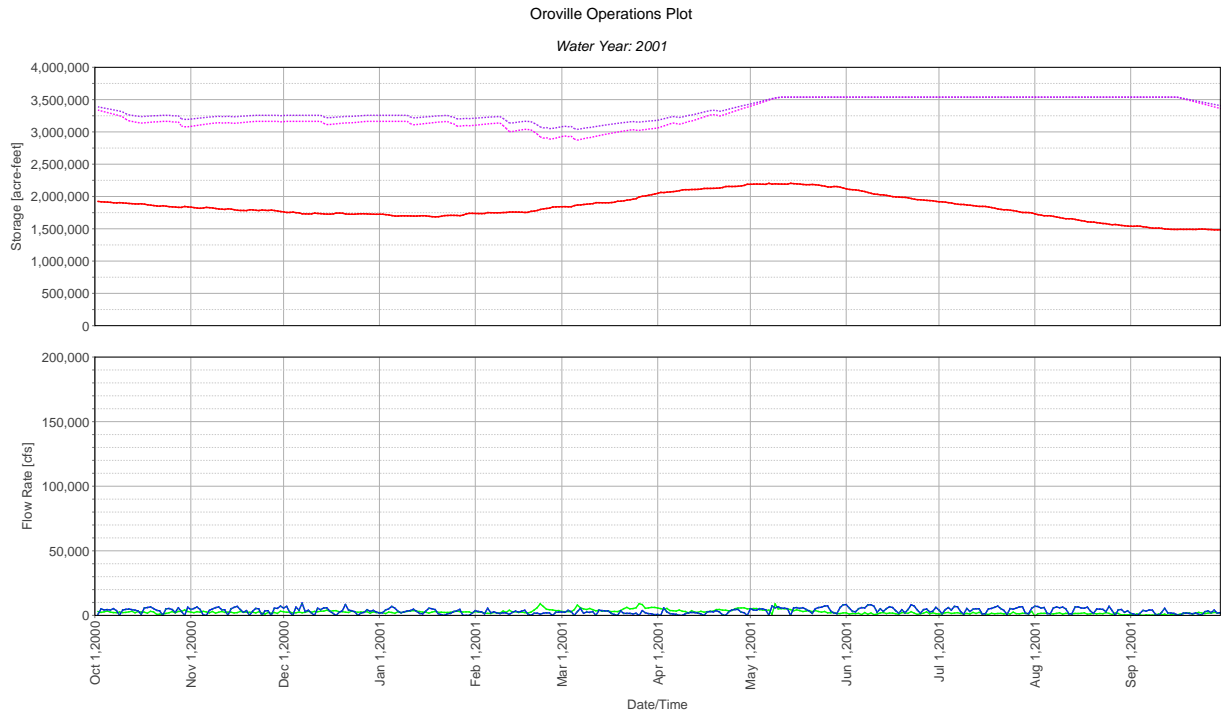


..... Top of Conservation Pool
 Flood Pool Encroachment Constraint
 —— Storage
 Reop Storage
 —— Inflow
 —— Outflow
 Reop Outflow

A: Reoperation Plots



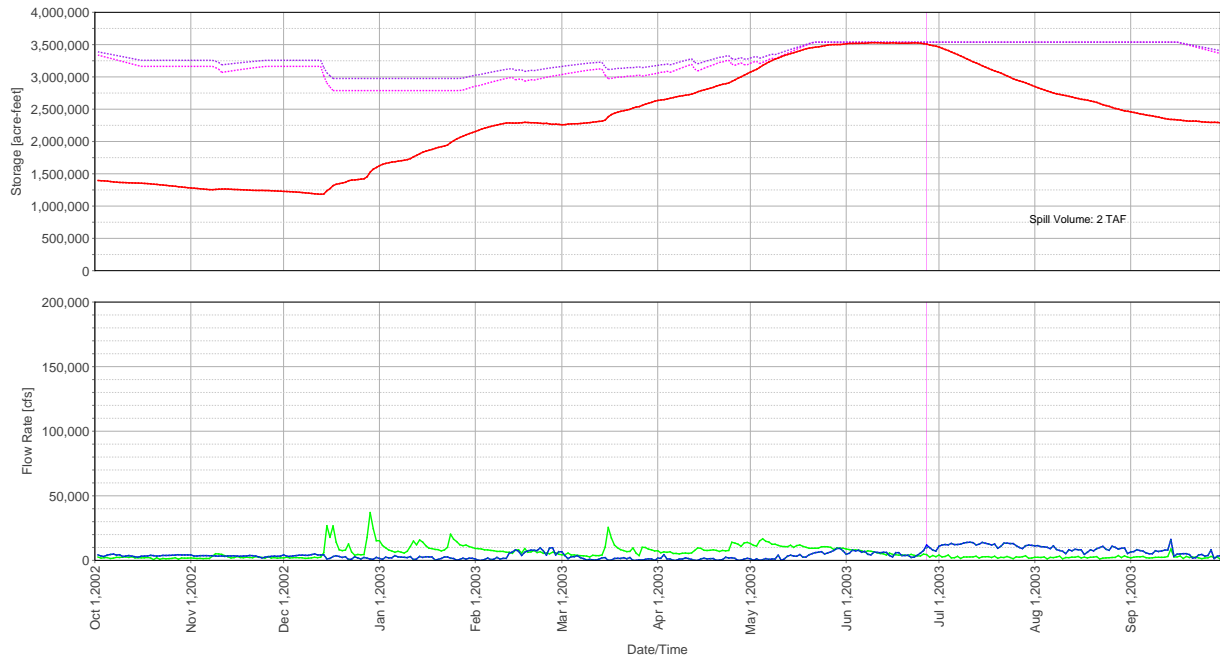
A: Reoperation Plots



A: Reoperation Plots

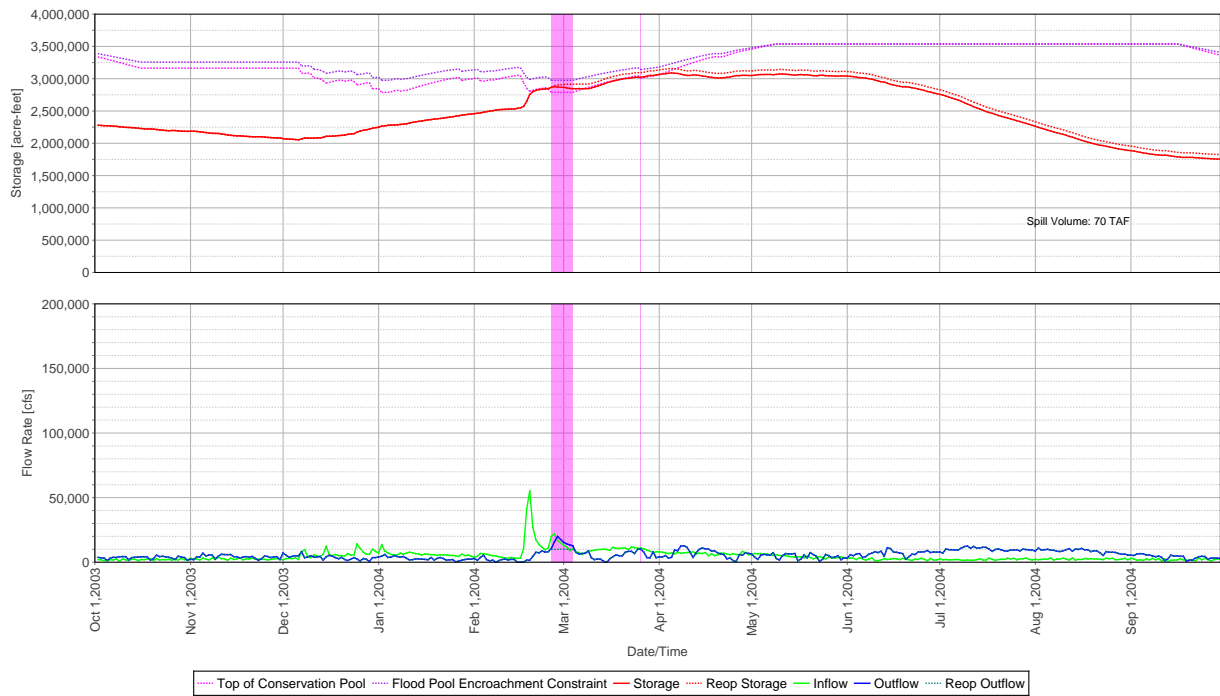
Oroville Operations Plot

Water Year: 2003

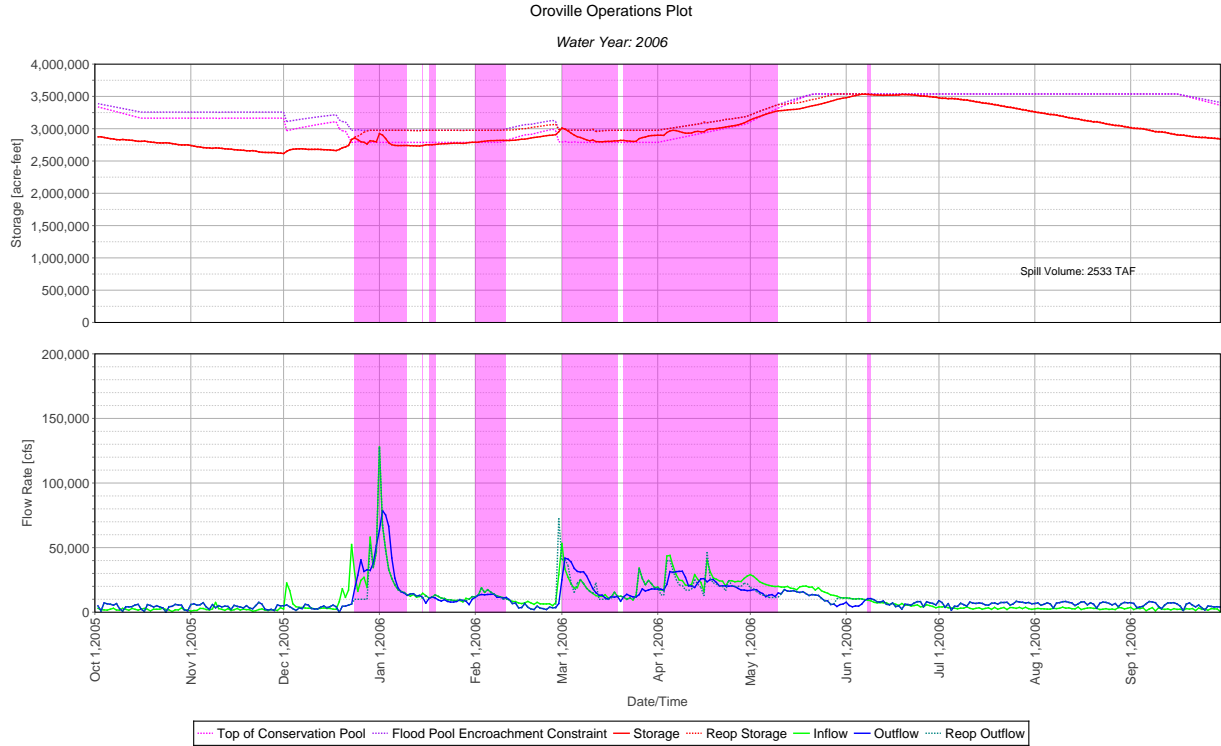
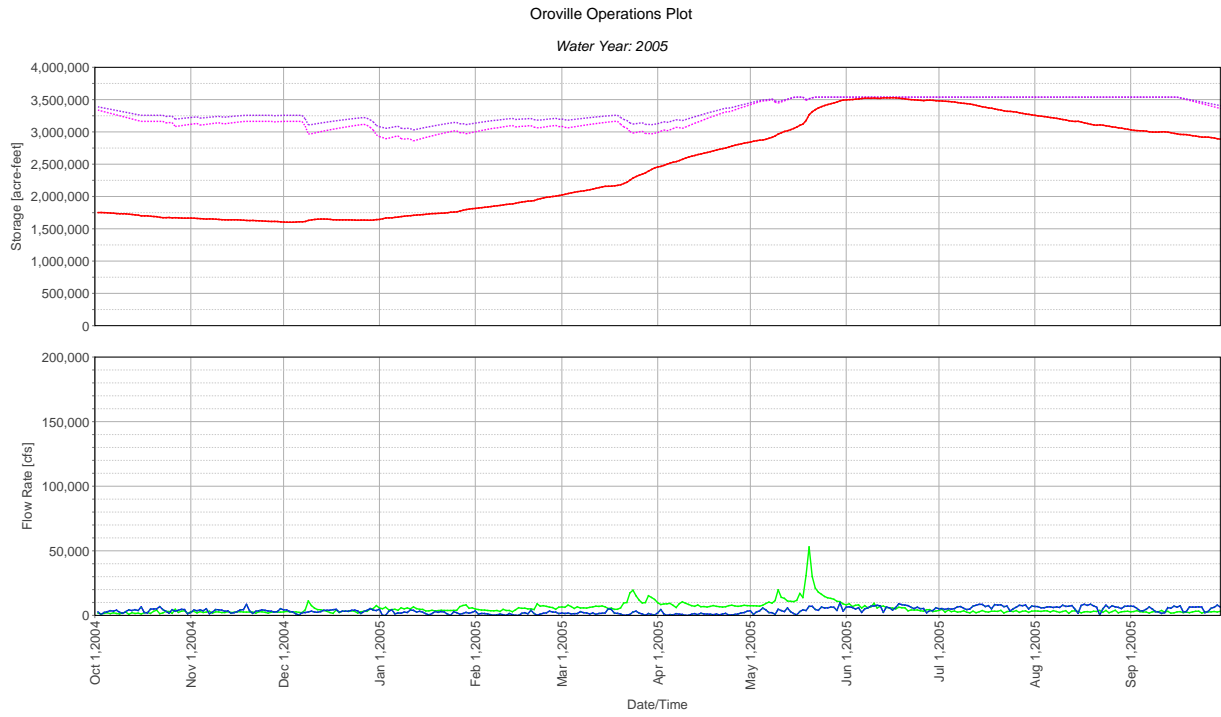


Oroville Operations Plot

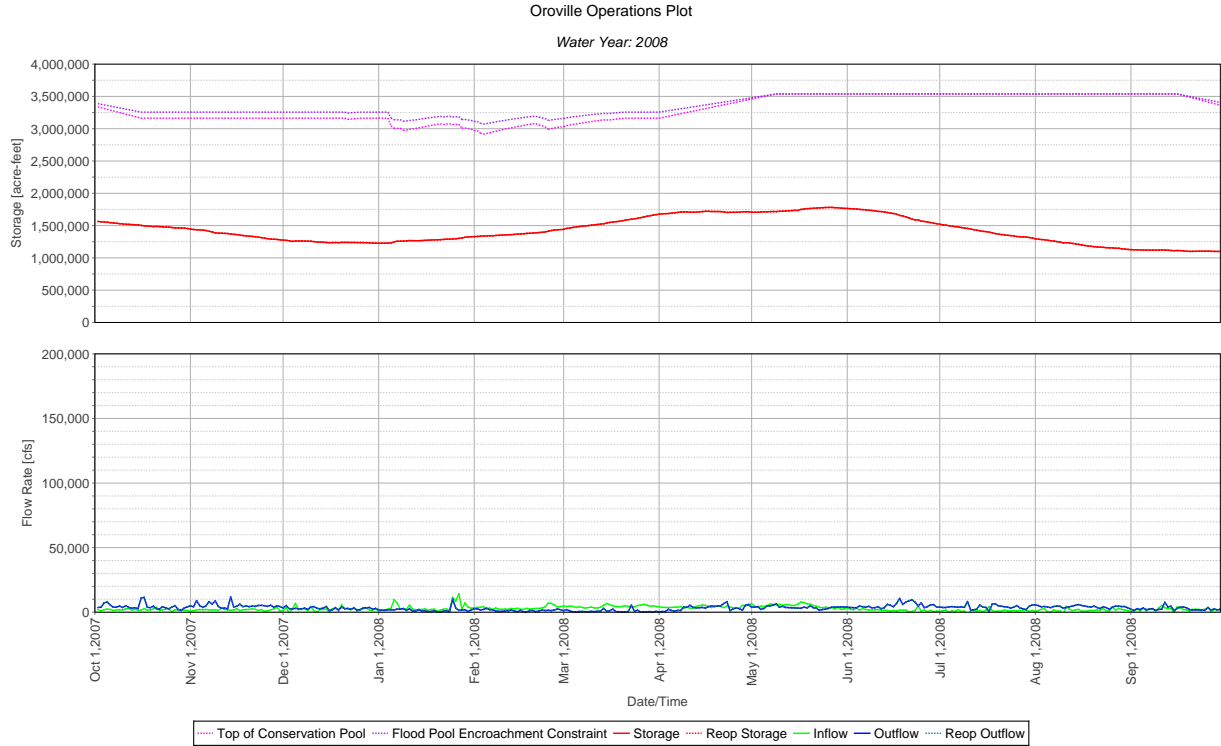
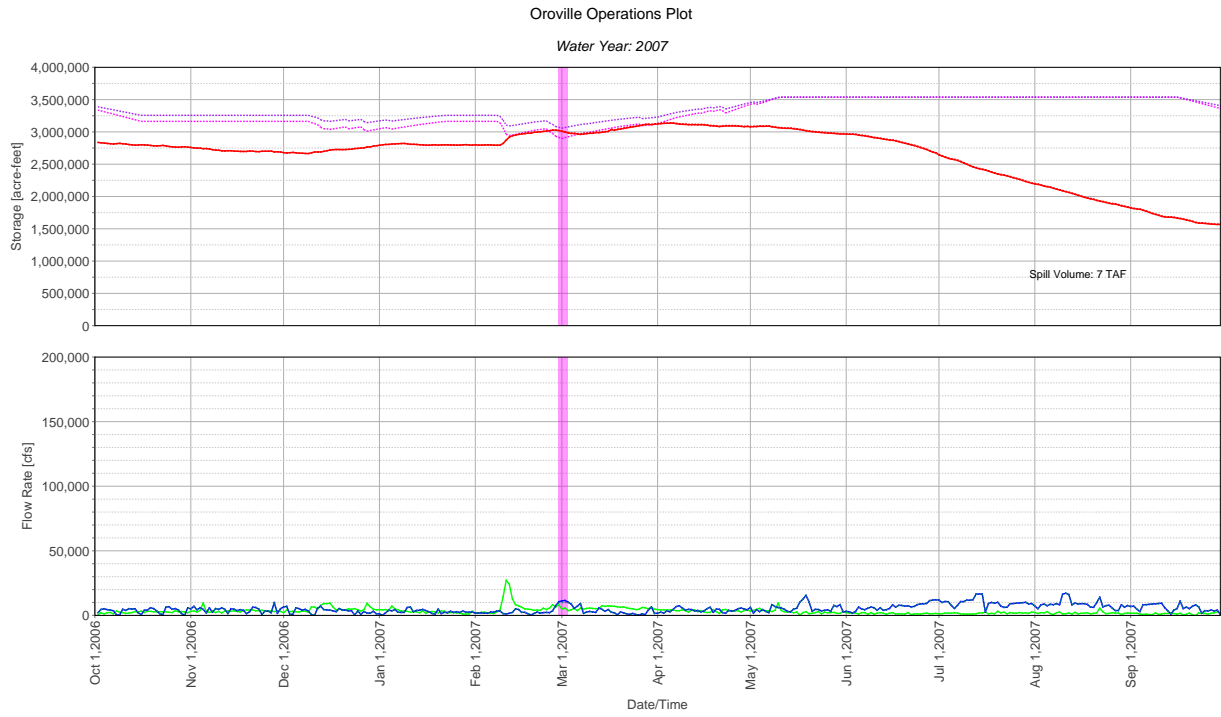
Water Year: 2004



A: Reoperation Plots



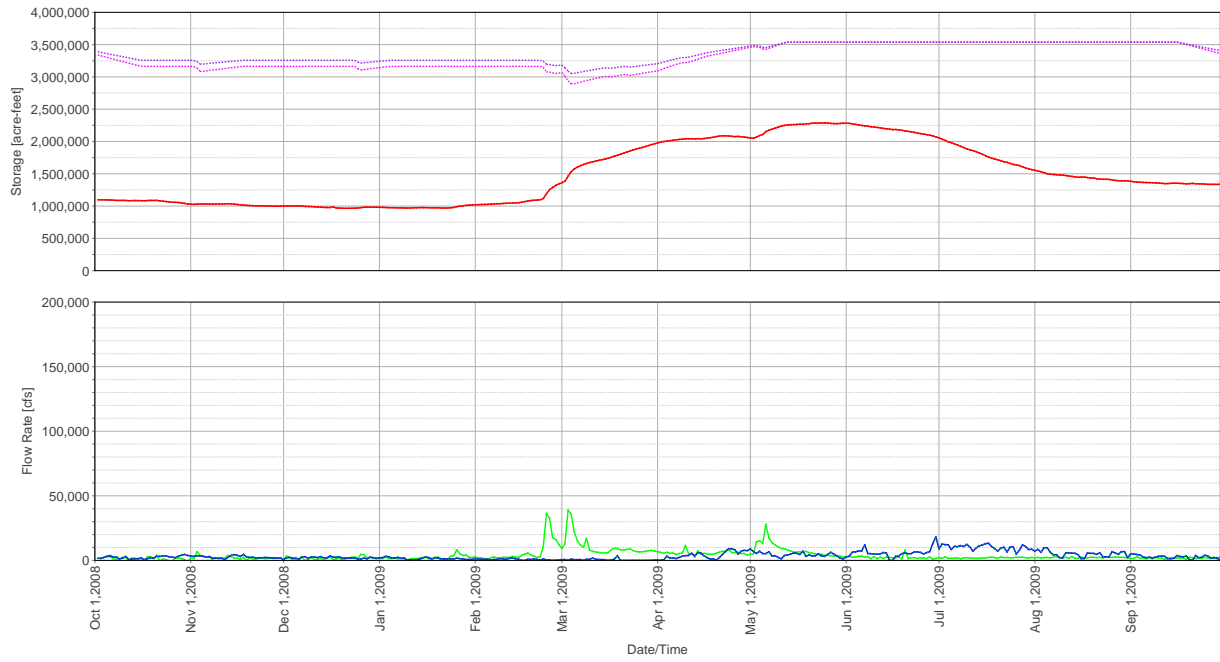
A: Reoperation Plots



A: Reoperation Plots

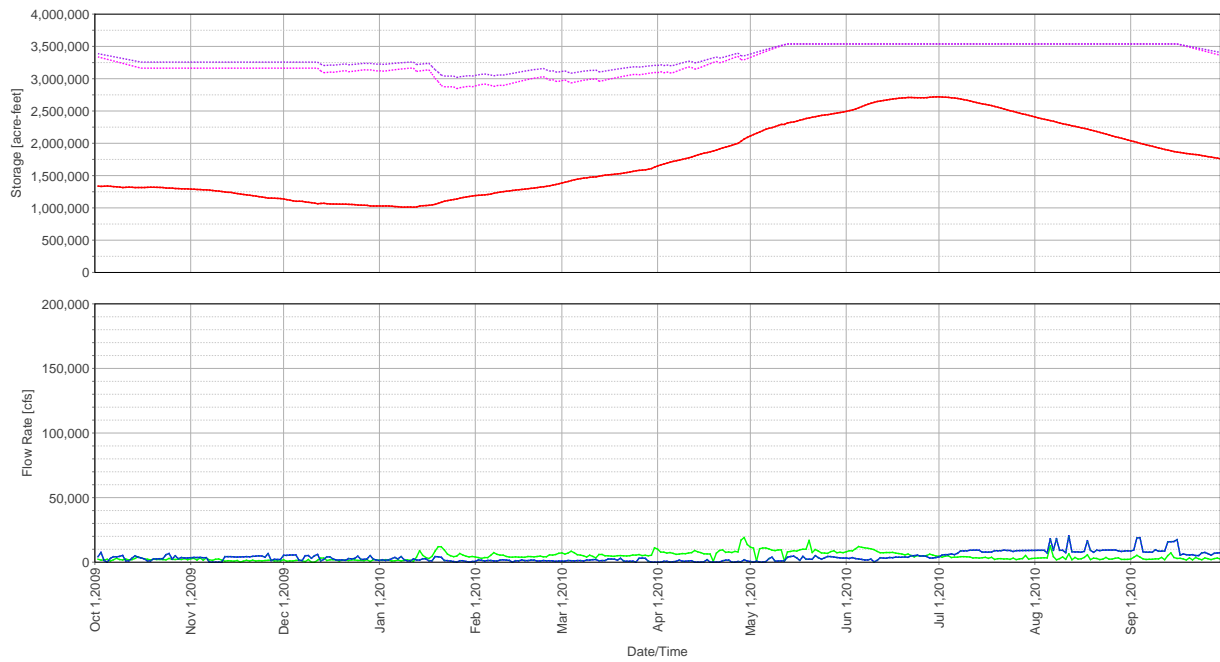
Oroville Operations Plot

Water Year: 2009



Oroville Operations Plot

Water Year: 2010

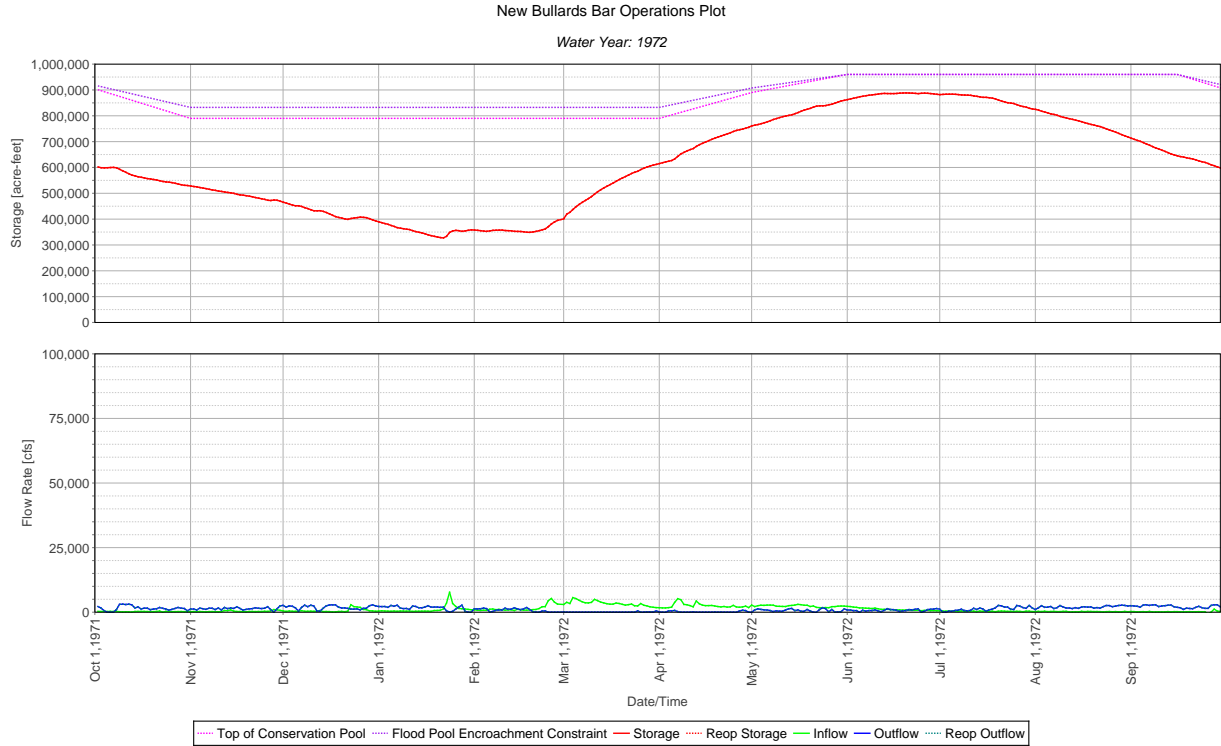
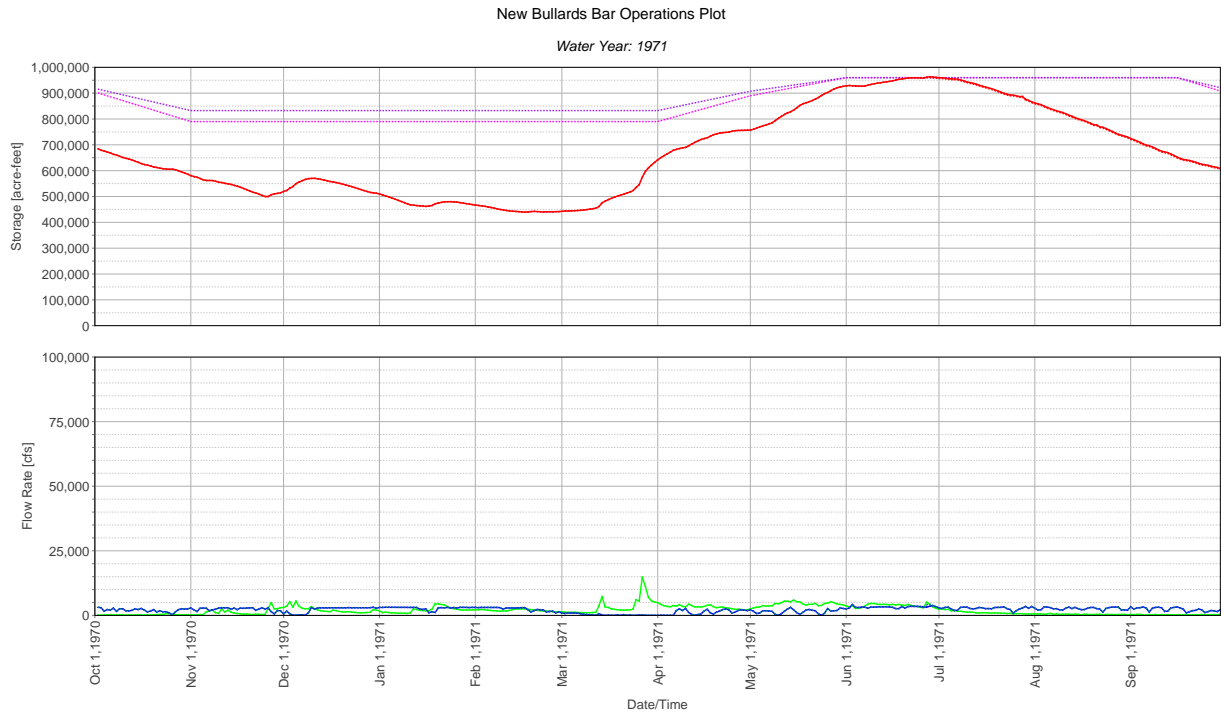


..... Top of Conservation Pool
 - - - - Flood Pool Encroachment Constraint
 ——— Storage
 - - - - Reop Storage
 ——— Inflow
 ——— Outflow
 - - - - Reop Outflow

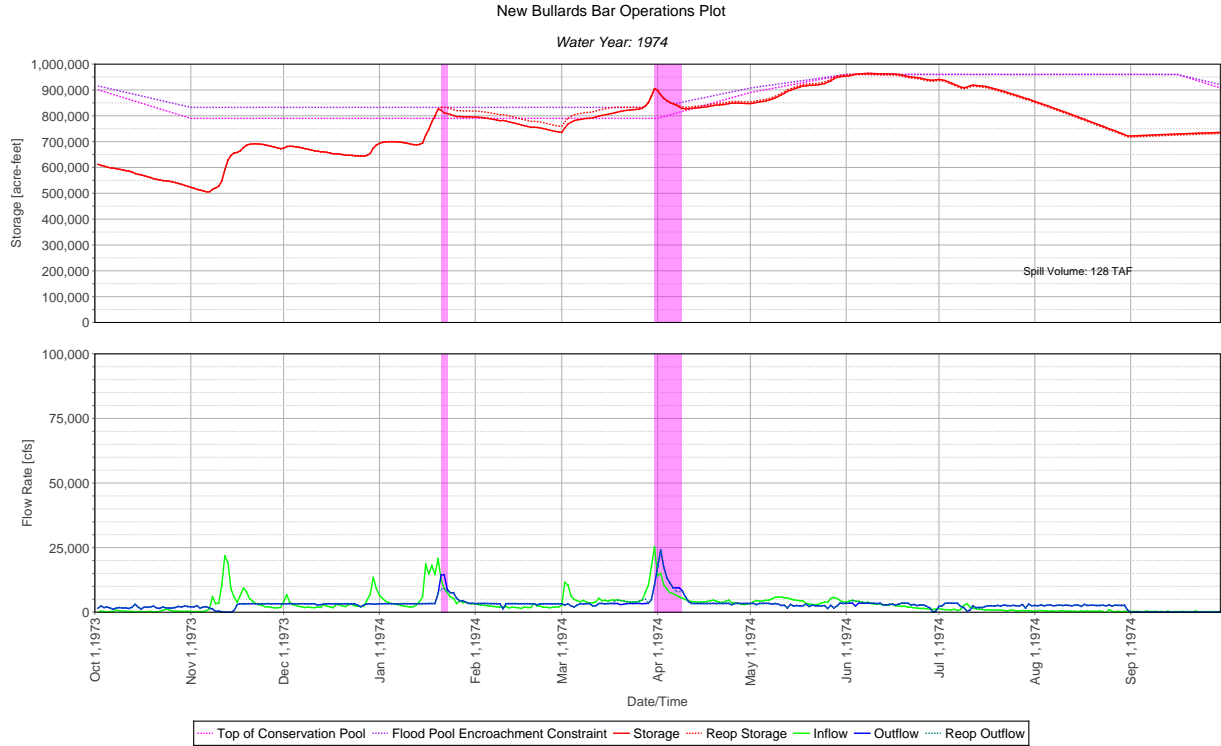
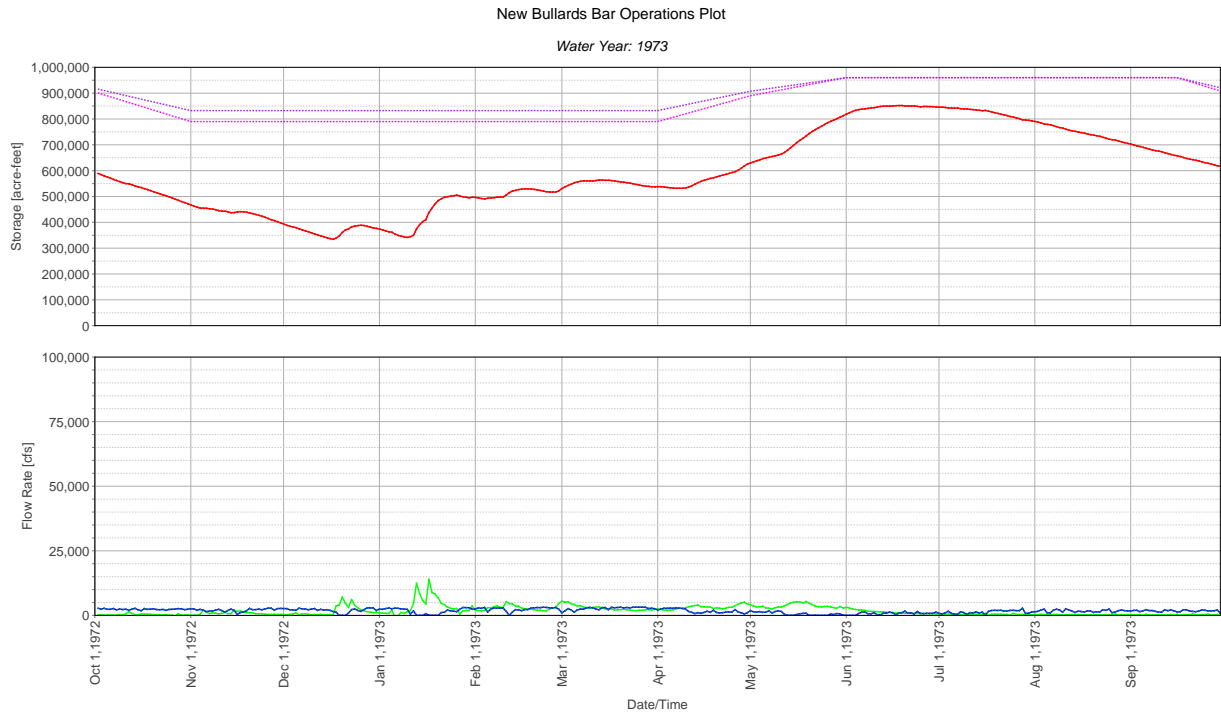
A.3 New Bullards Bar

The following section includes the reoperation plots for New Bullards Bar Dam.

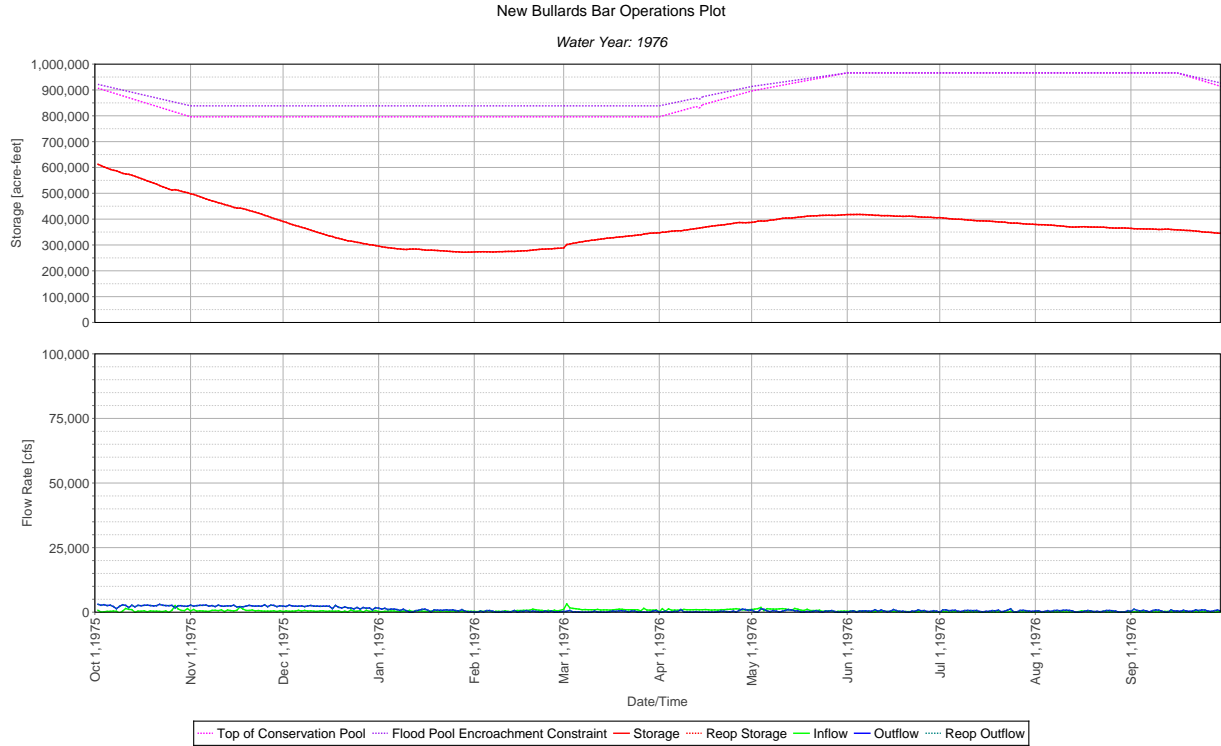
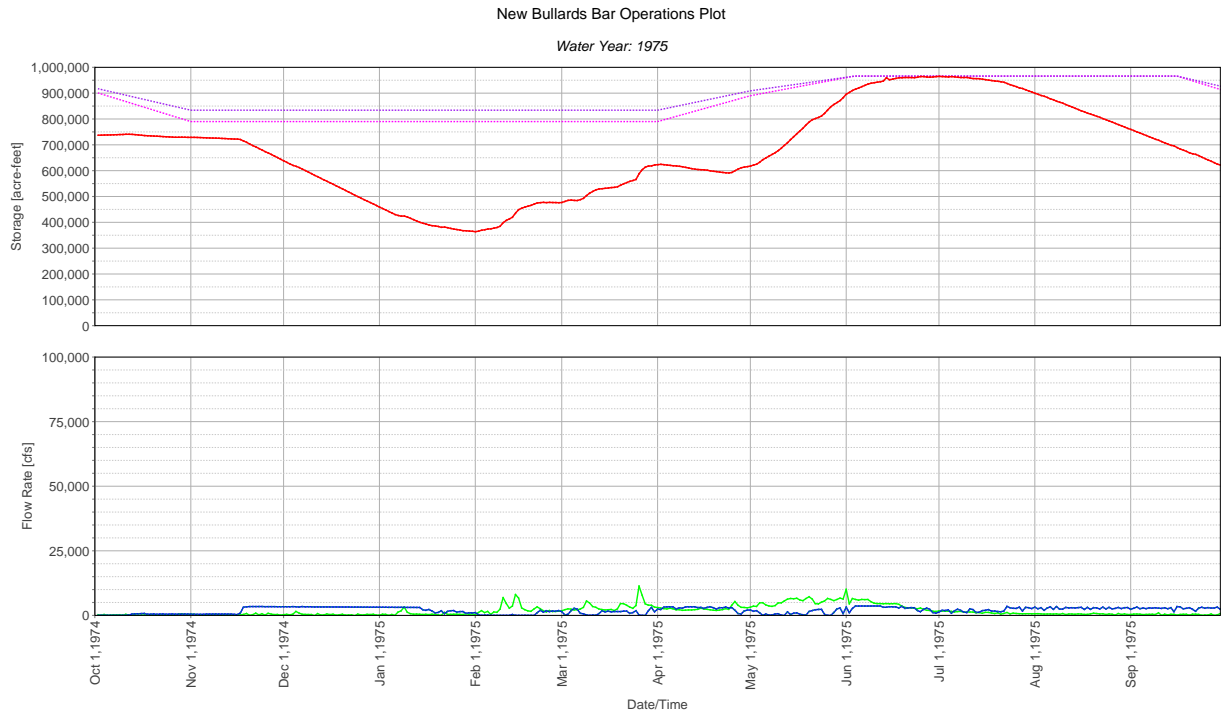
A: Reoperation Plots



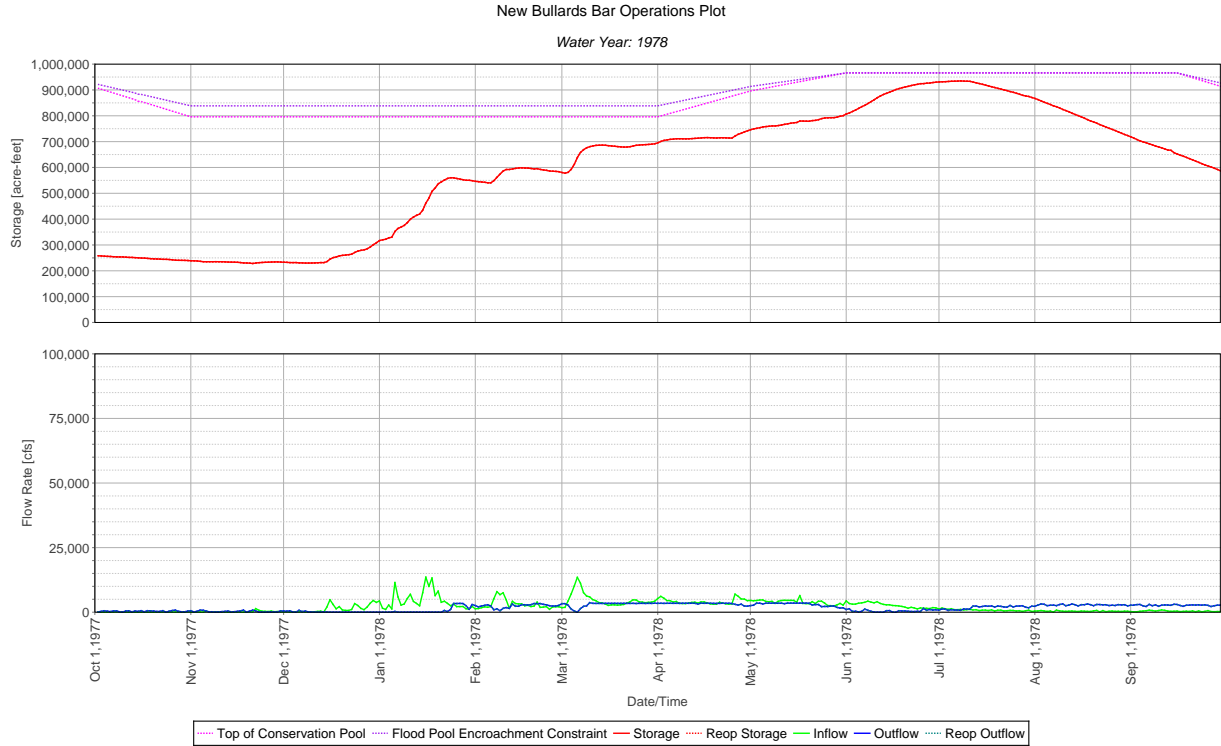
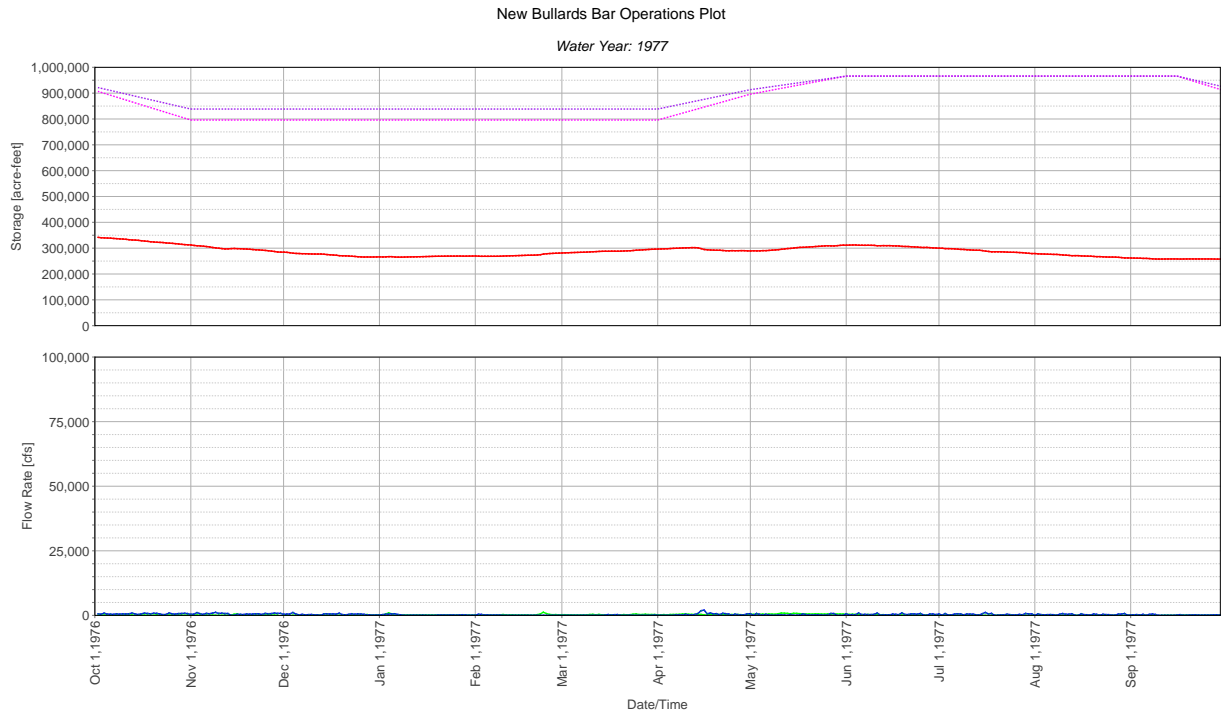
A: Reoperation Plots



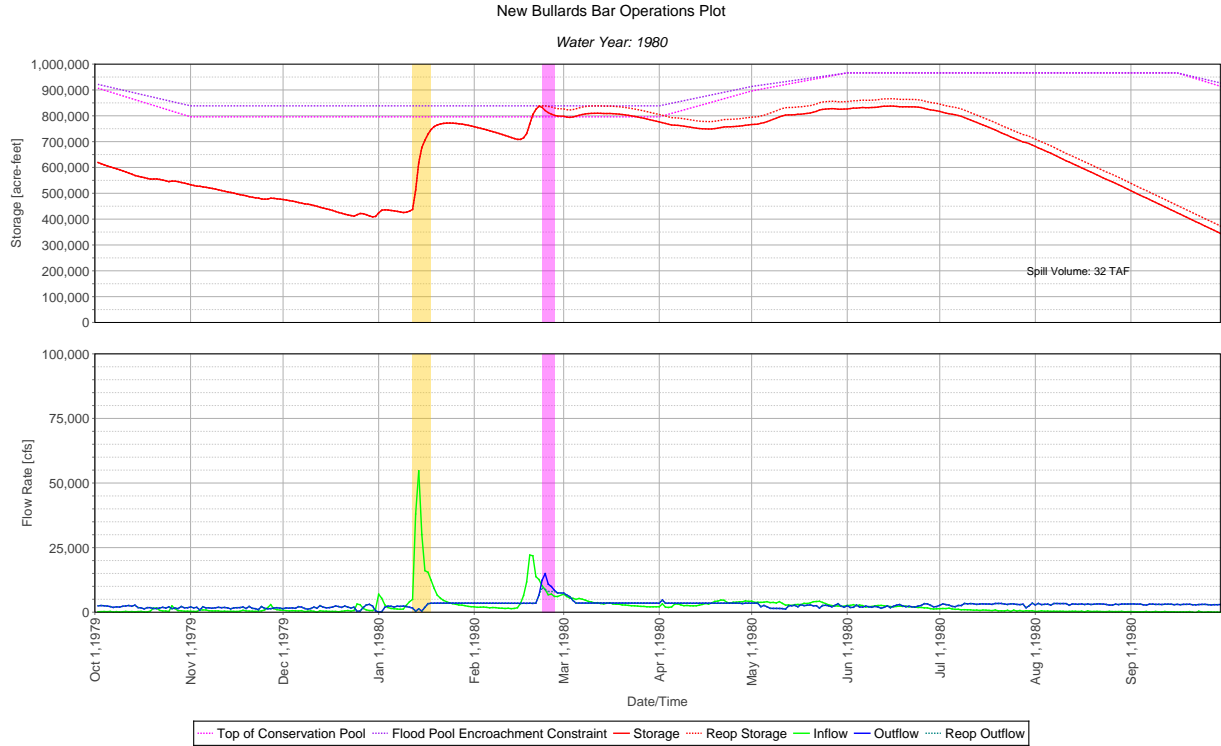
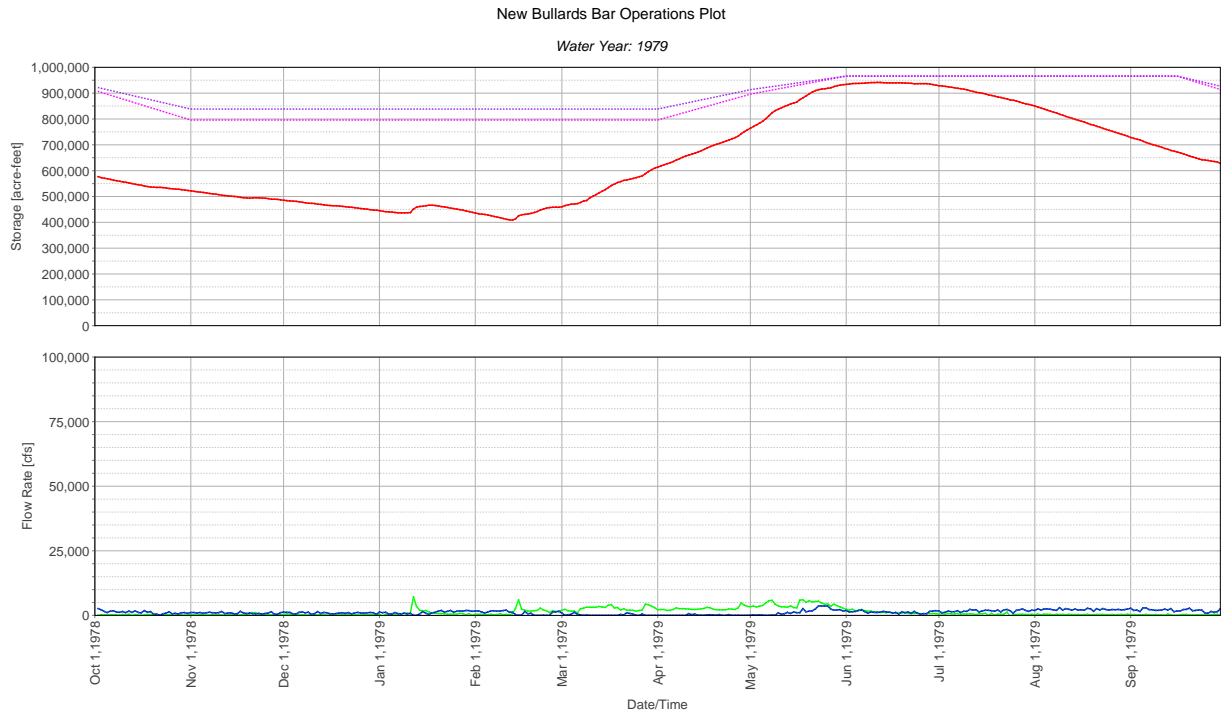
A: Reoperation Plots



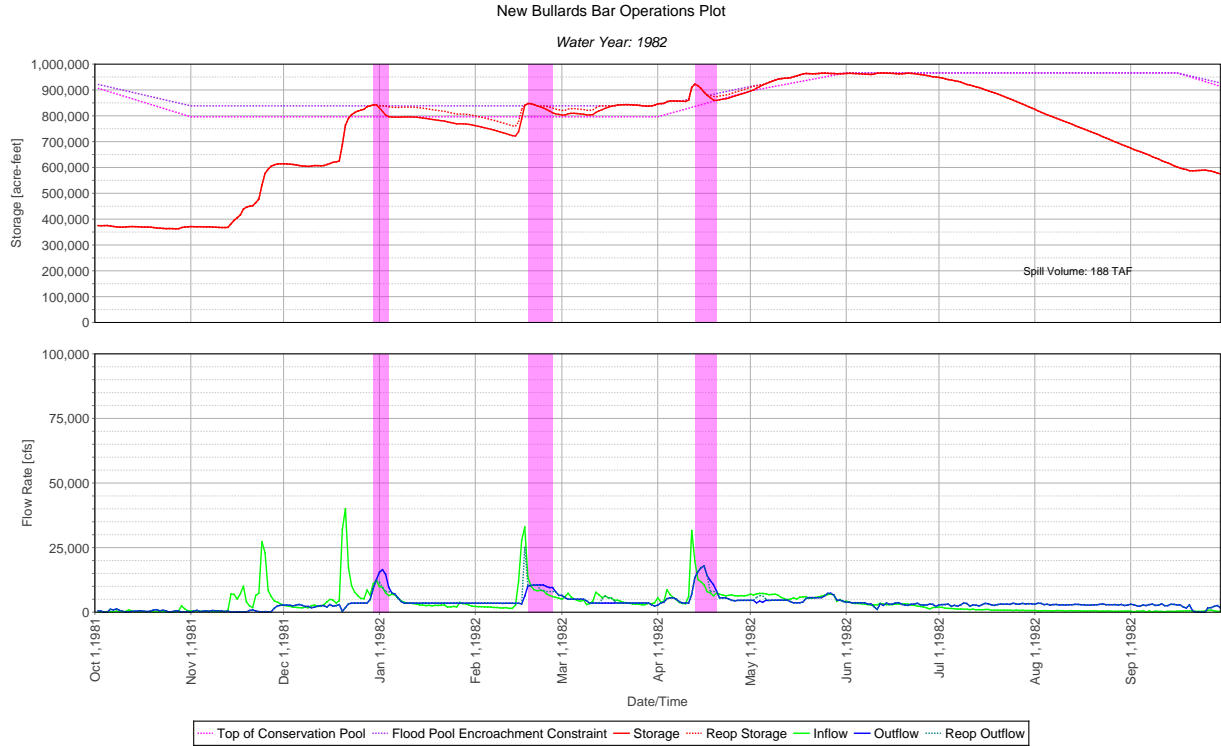
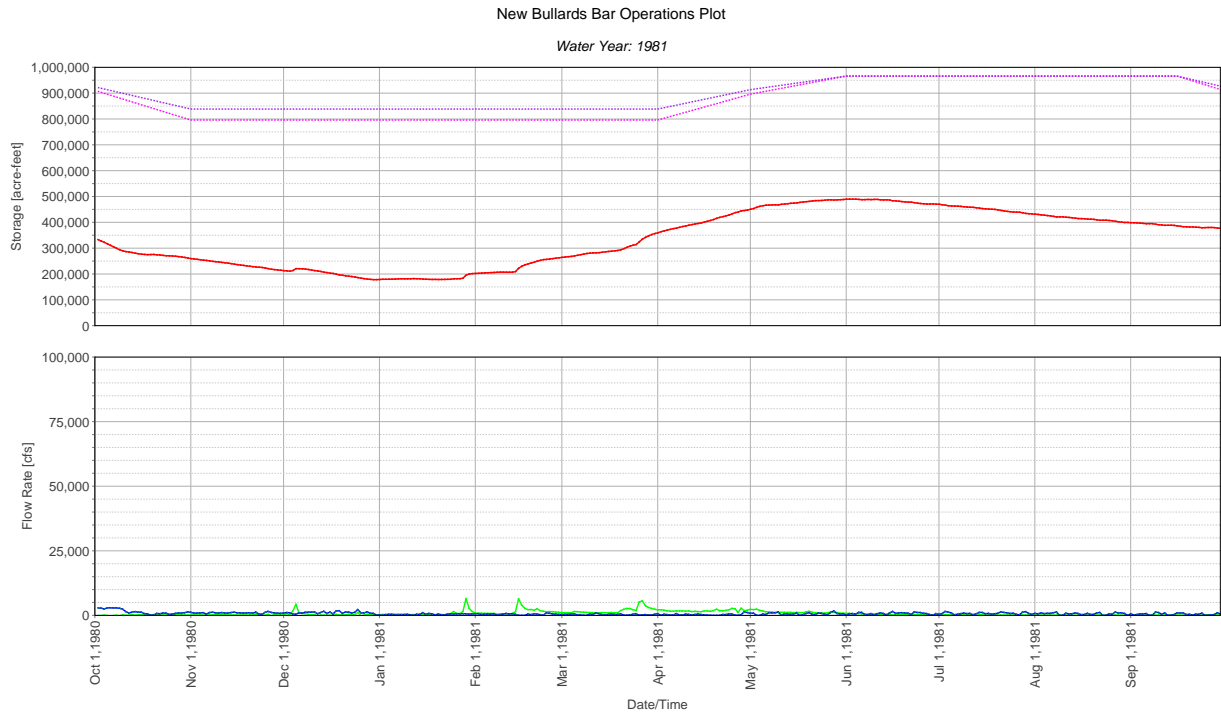
A: Reoperation Plots



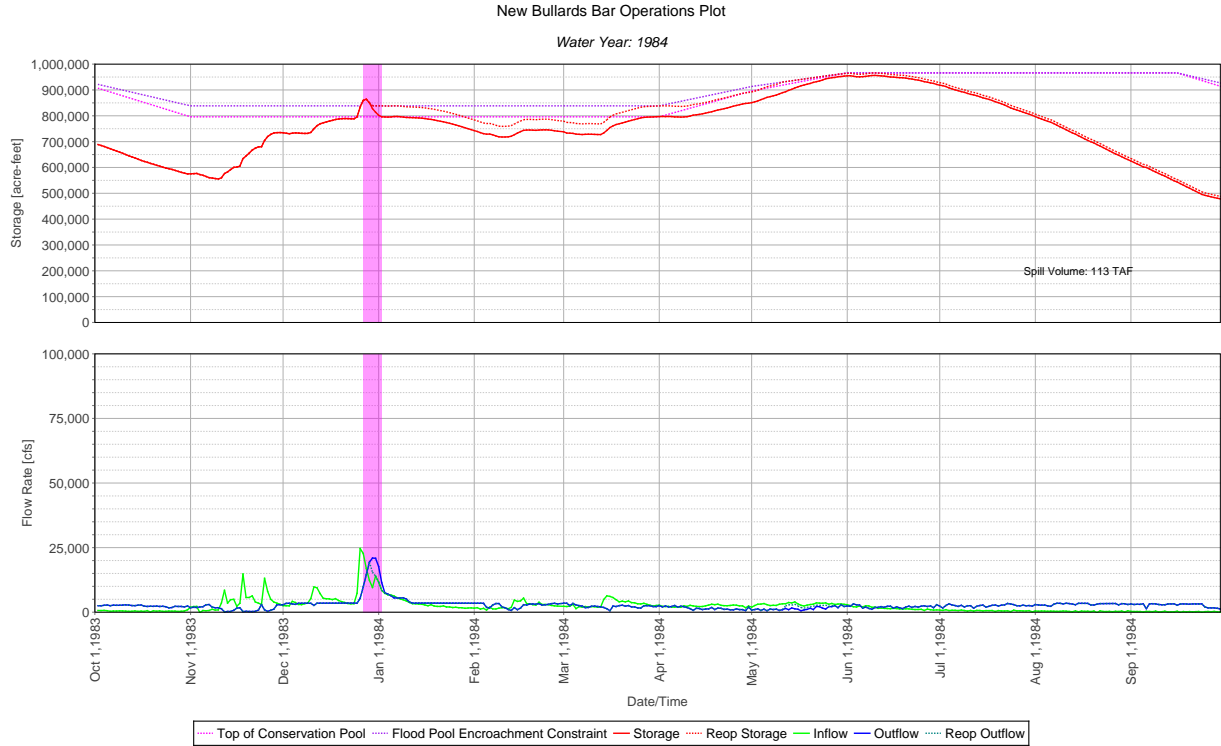
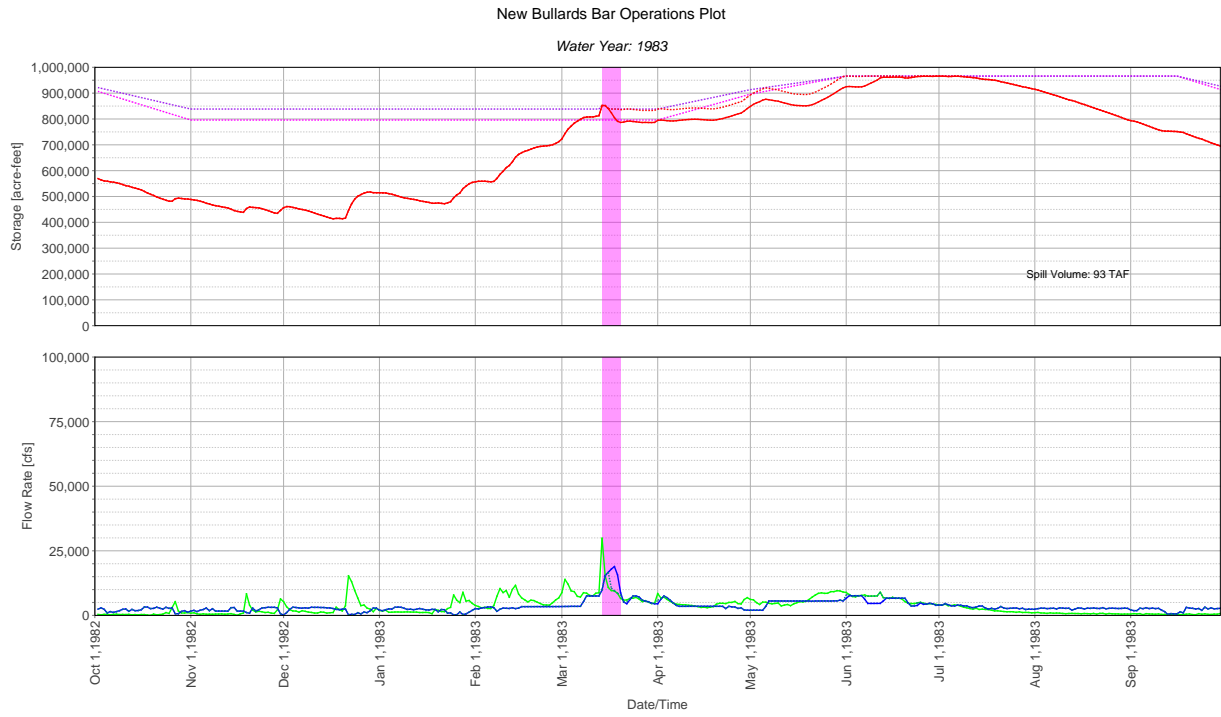
A: Reoperation Plots



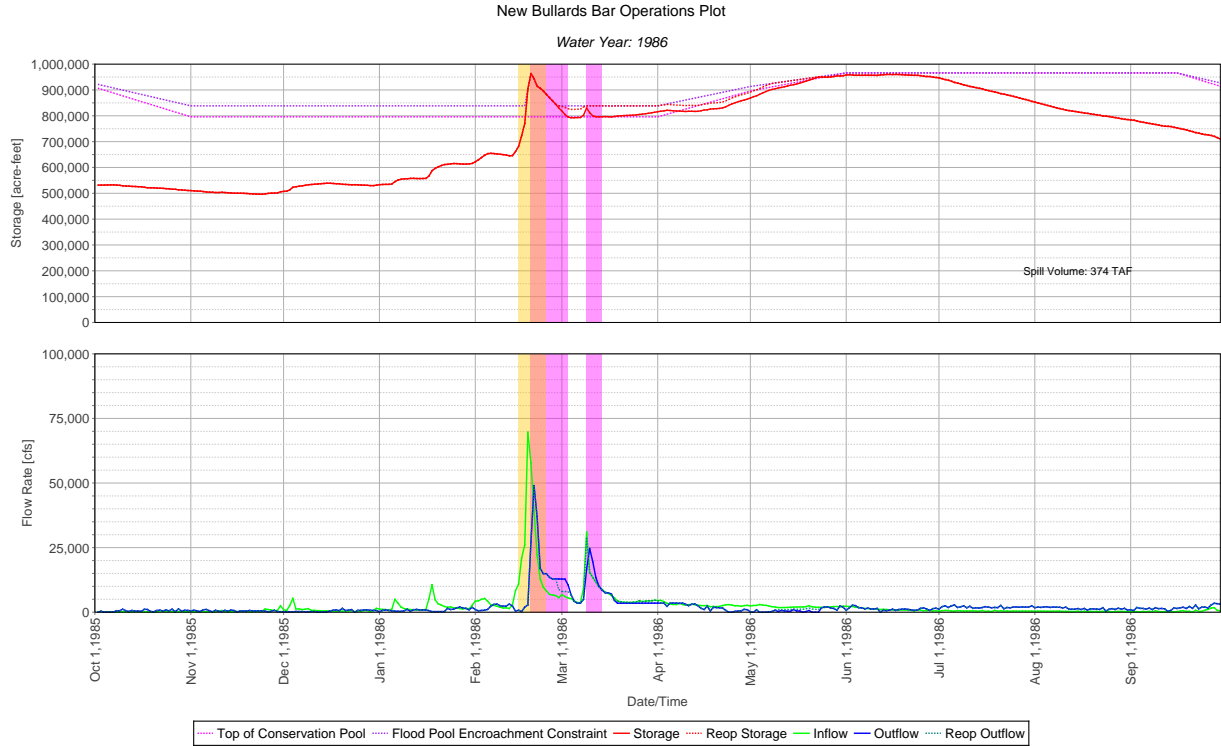
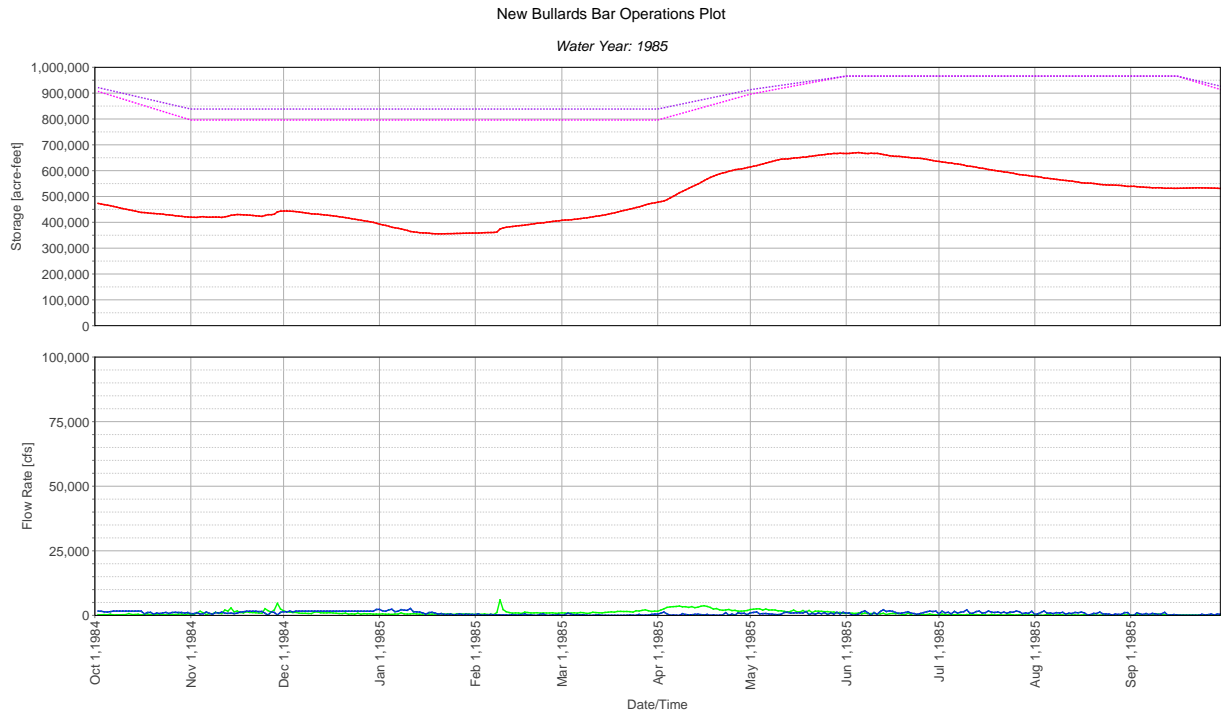
A: Reoperation Plots



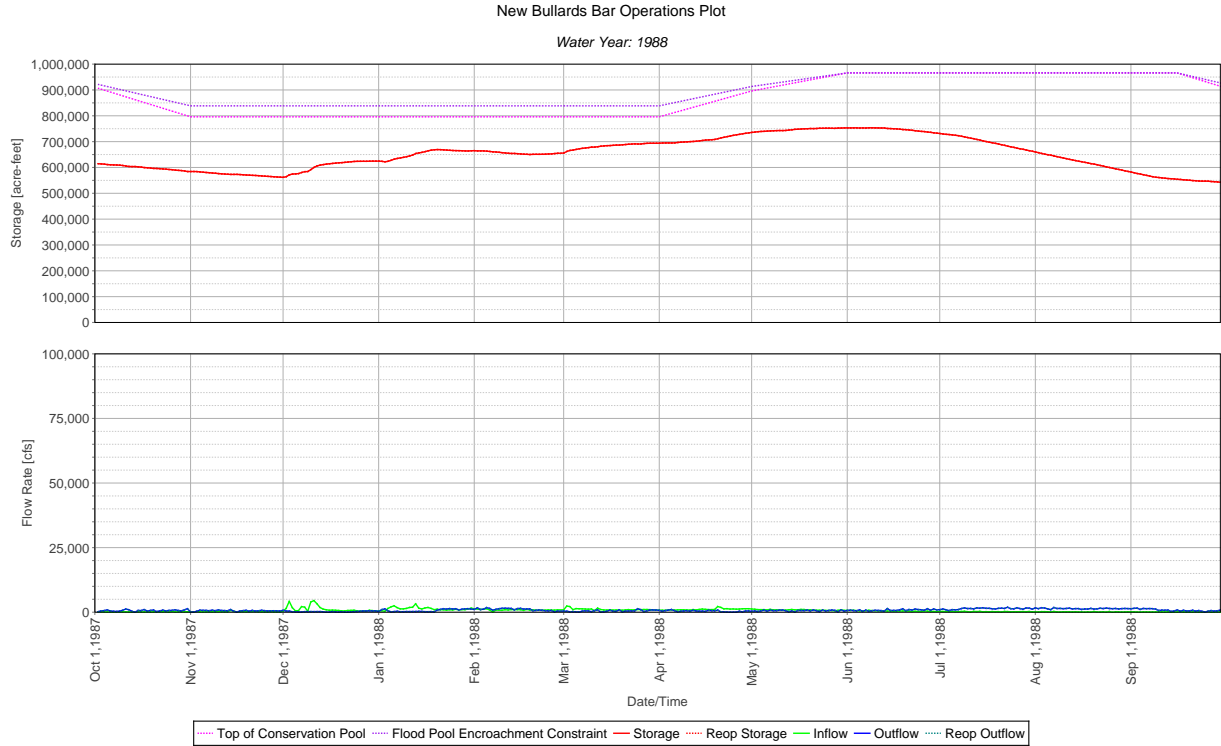
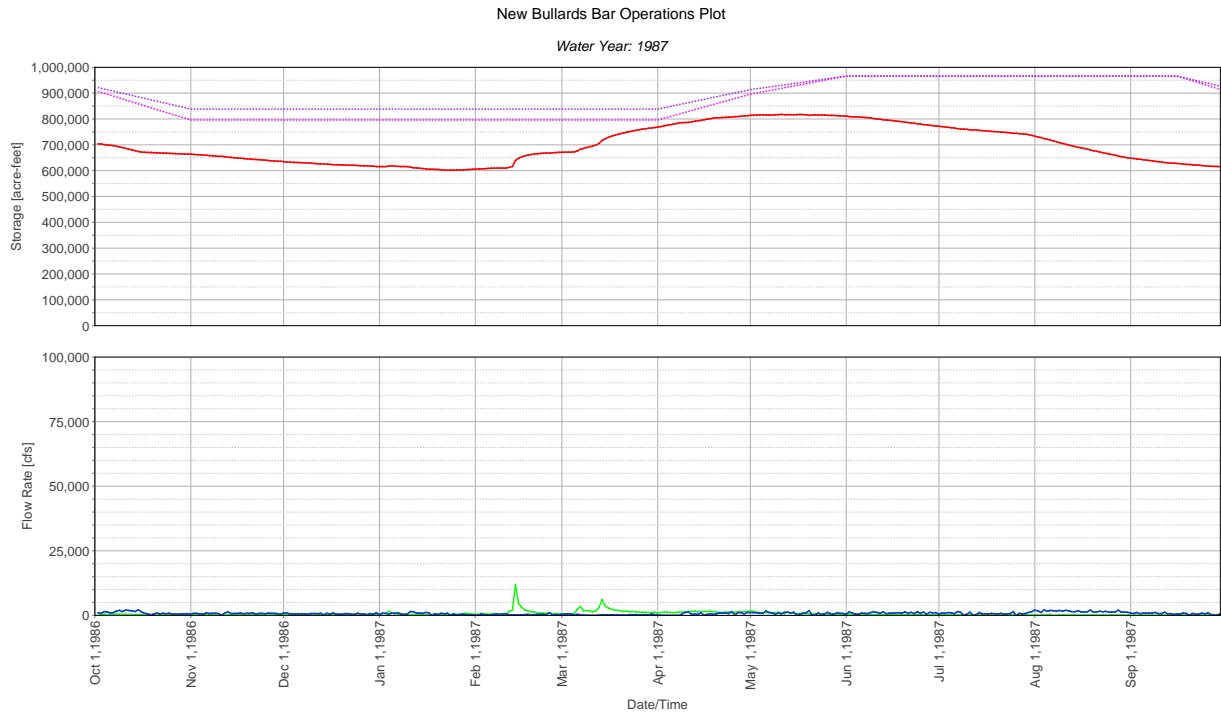
A: Reoperation Plots



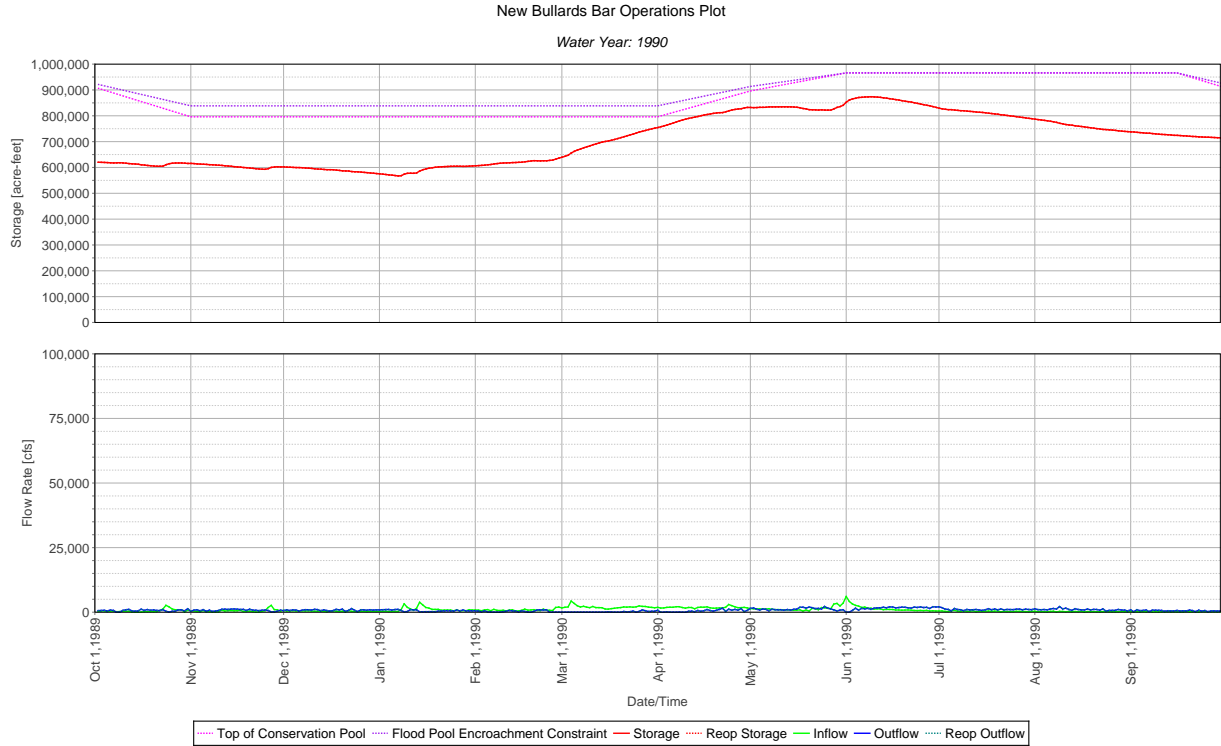
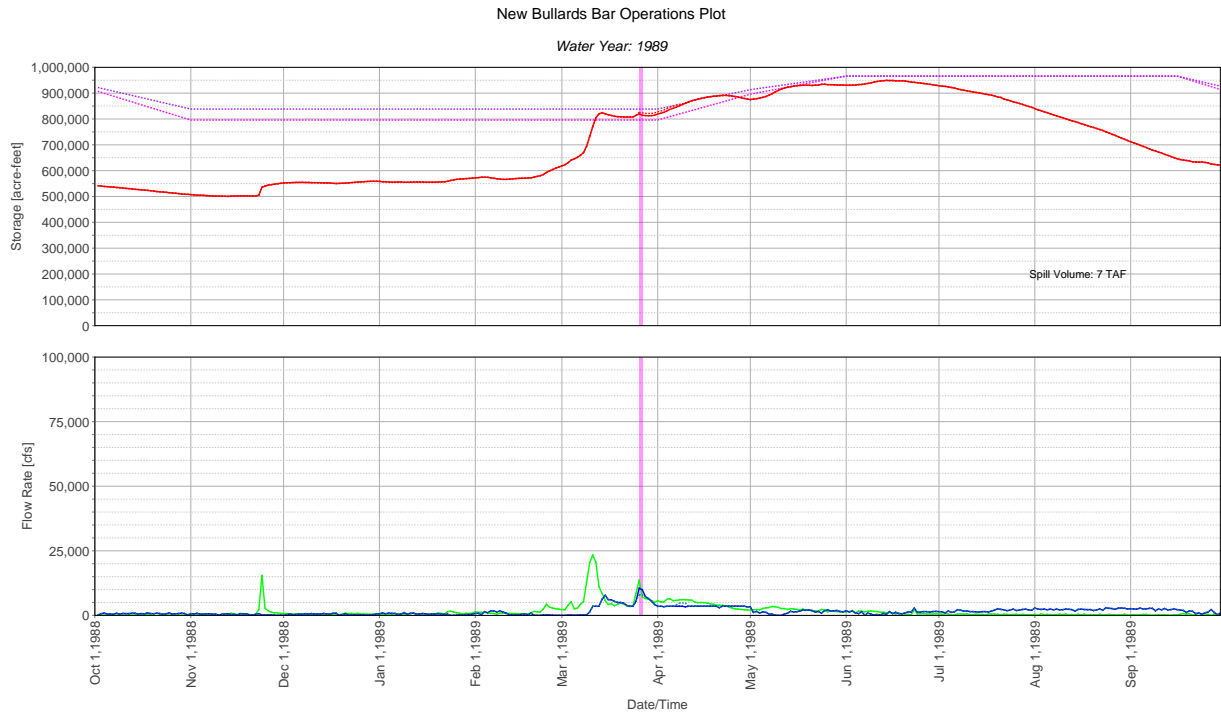
A: Reoperation Plots



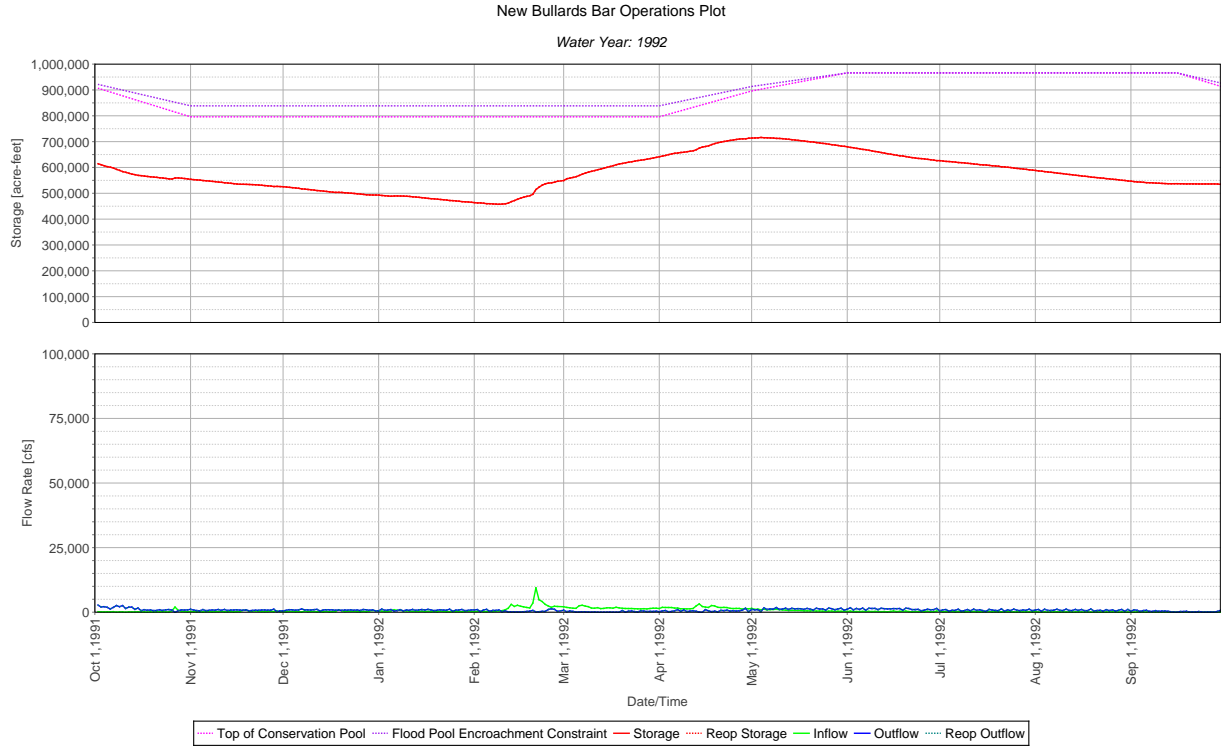
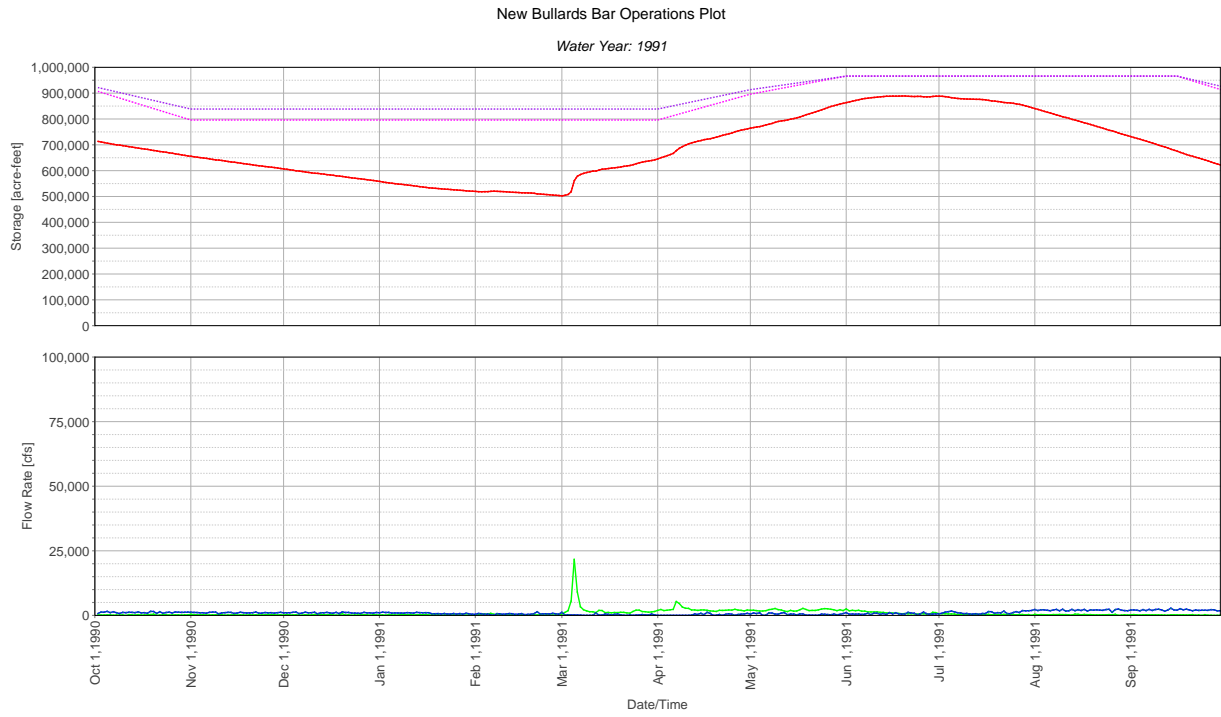
A: Reoperation Plots



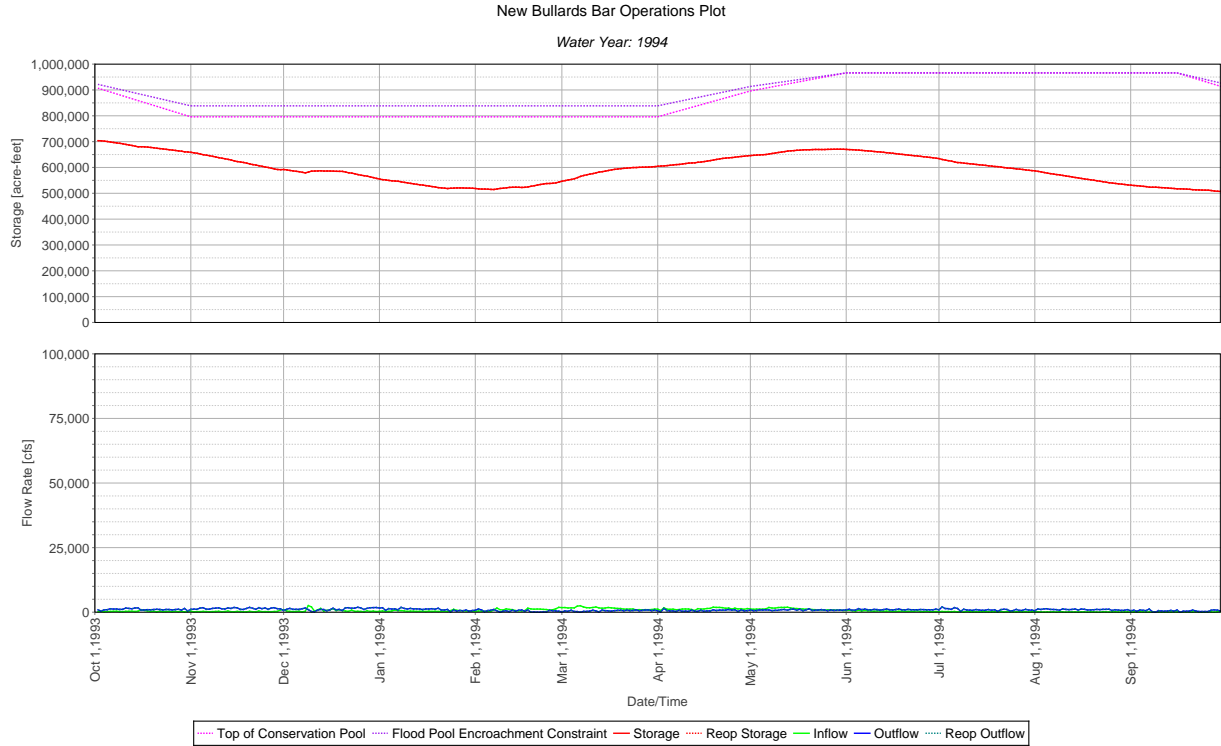
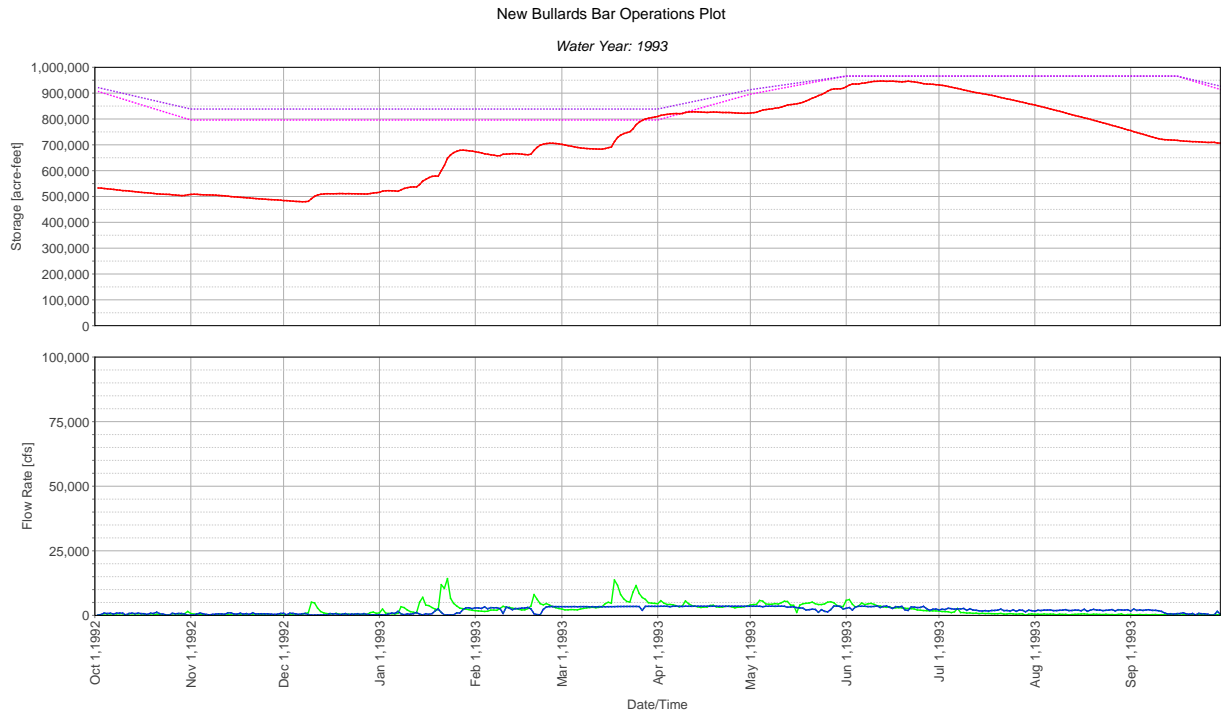
A: Reoperation Plots



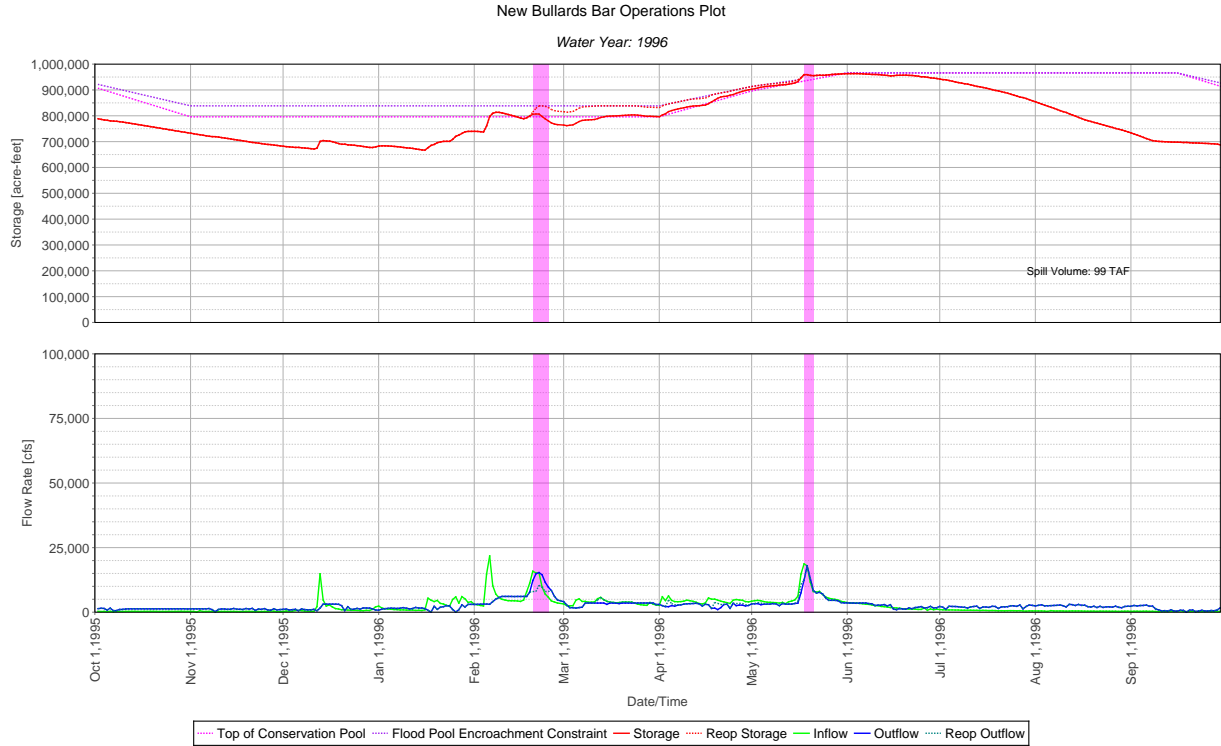
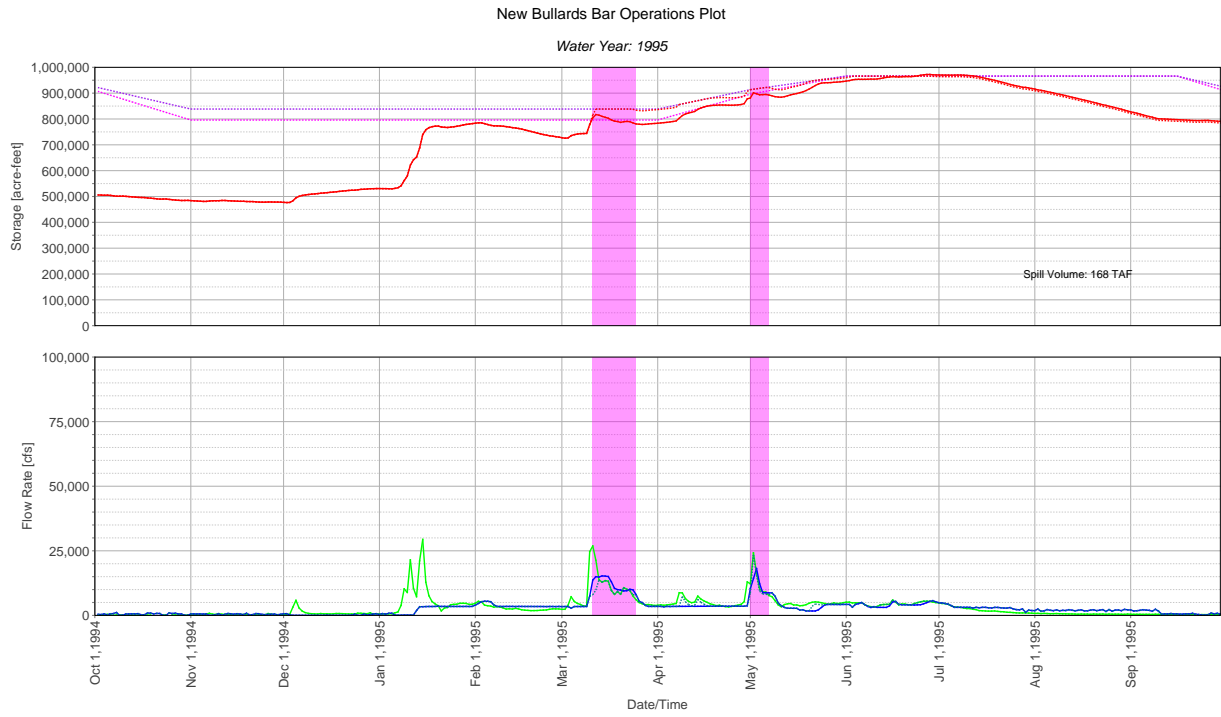
A: Reoperation Plots



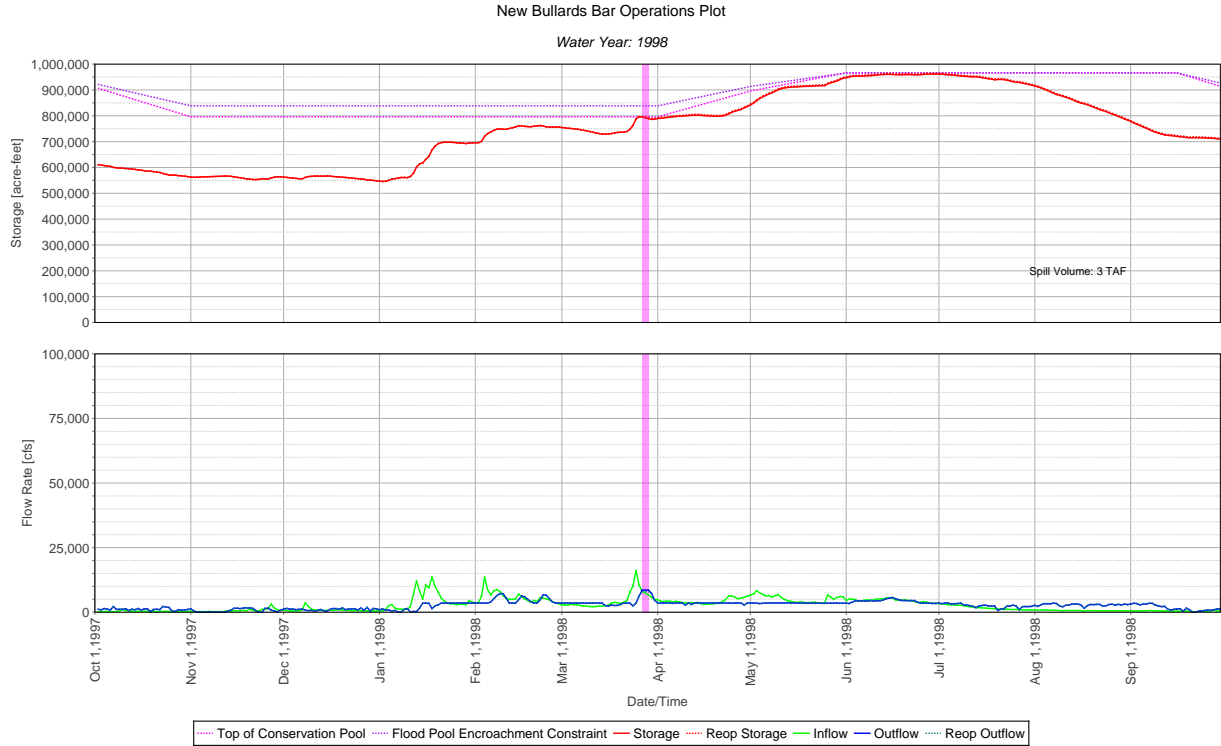
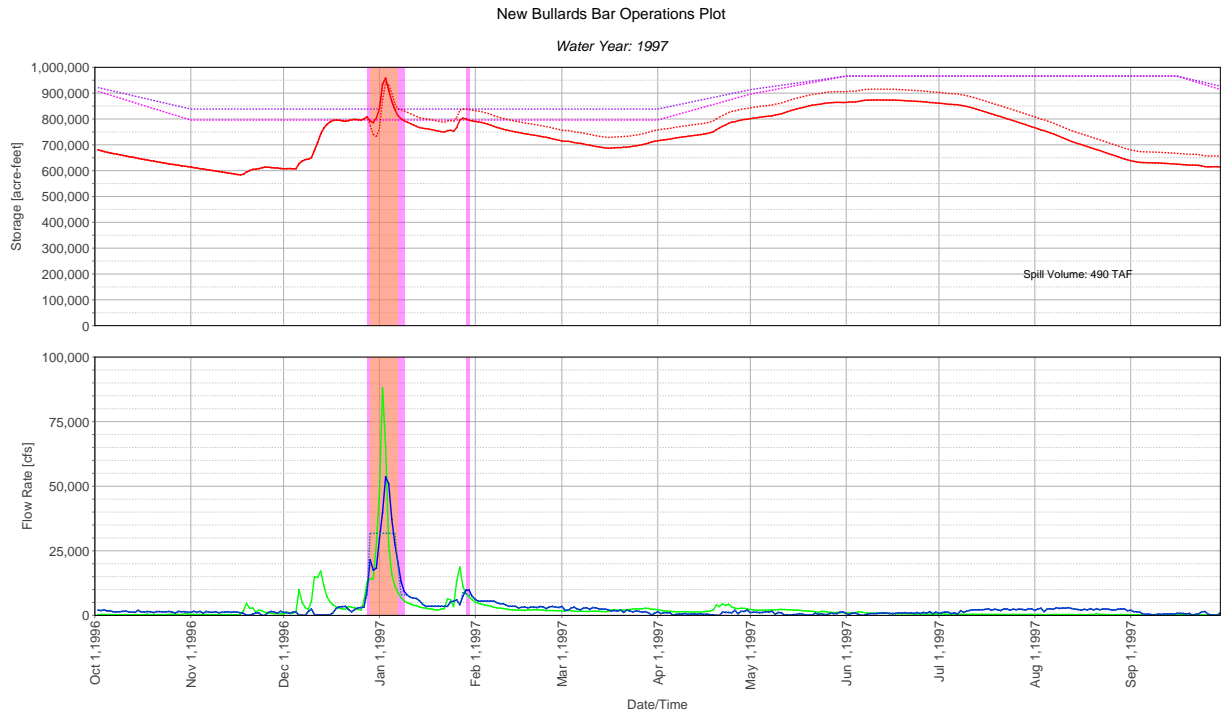
A: Reoperation Plots



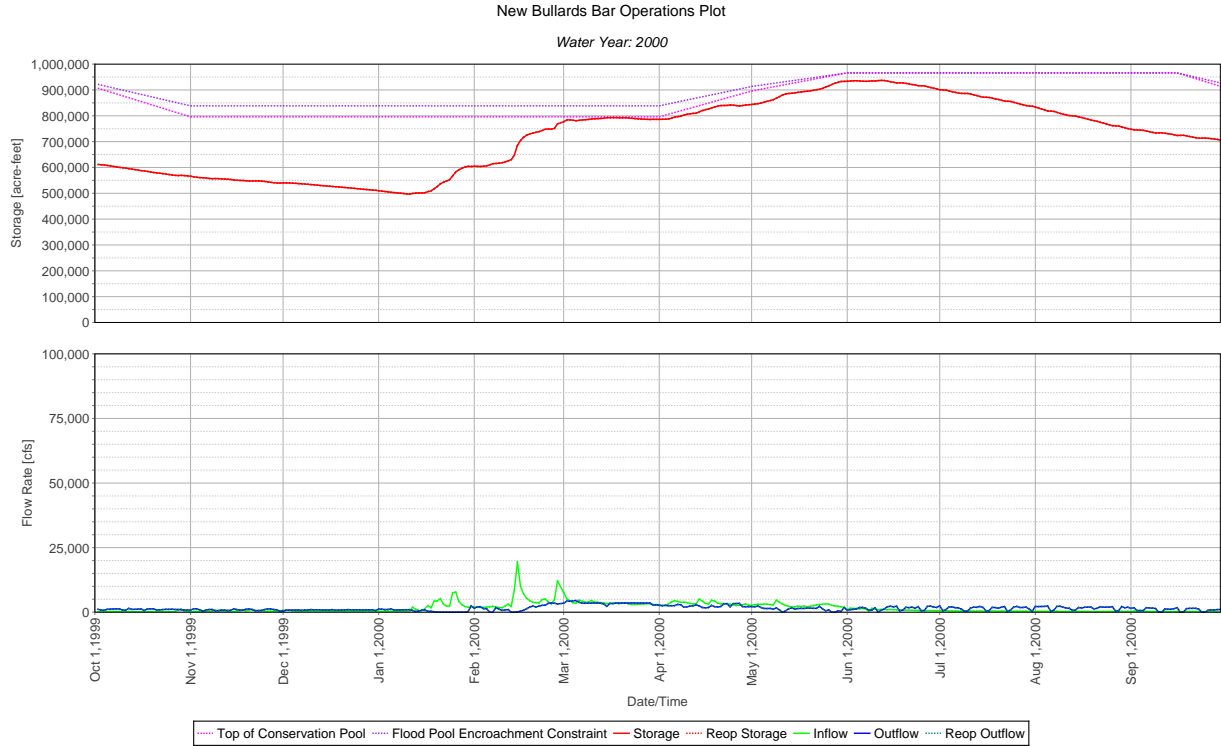
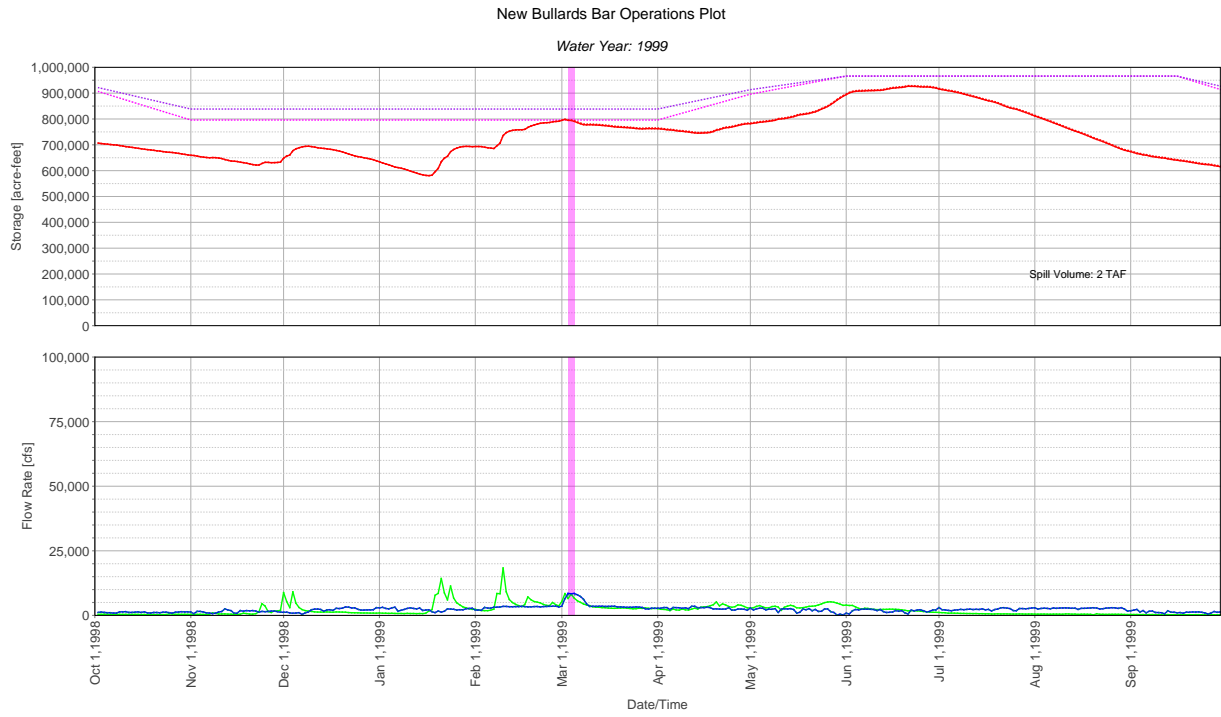
A: Reoperation Plots



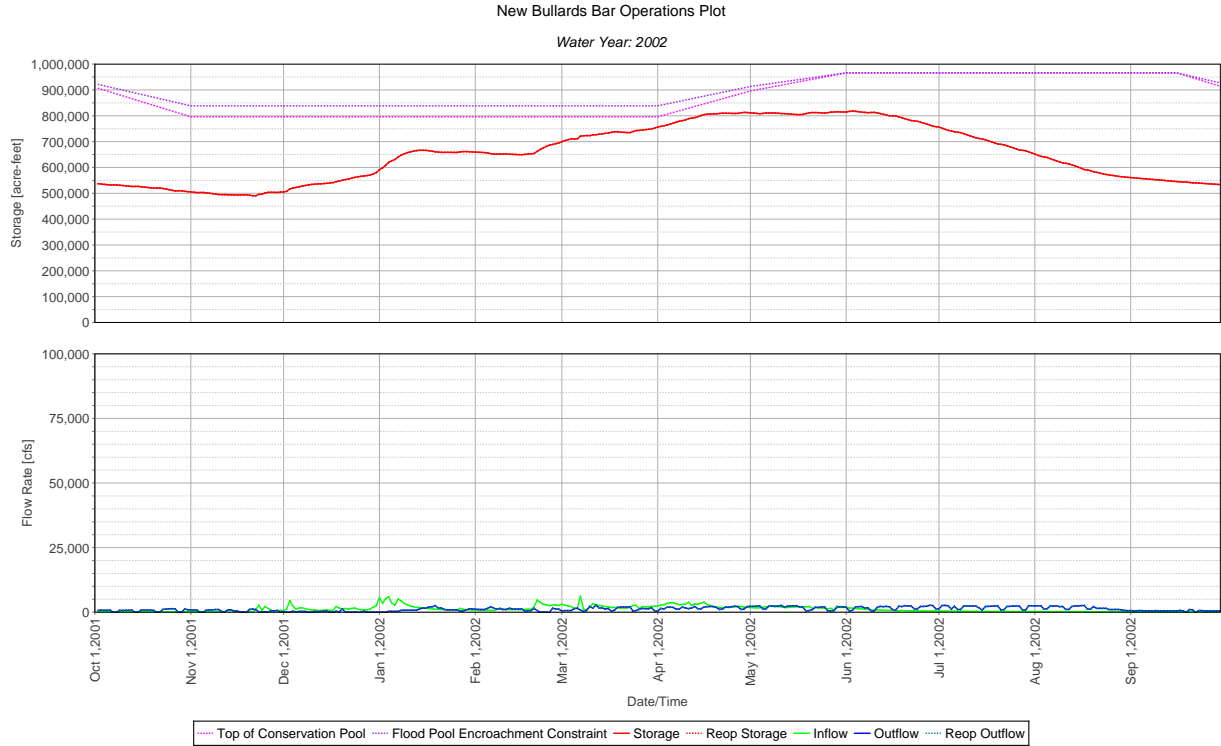
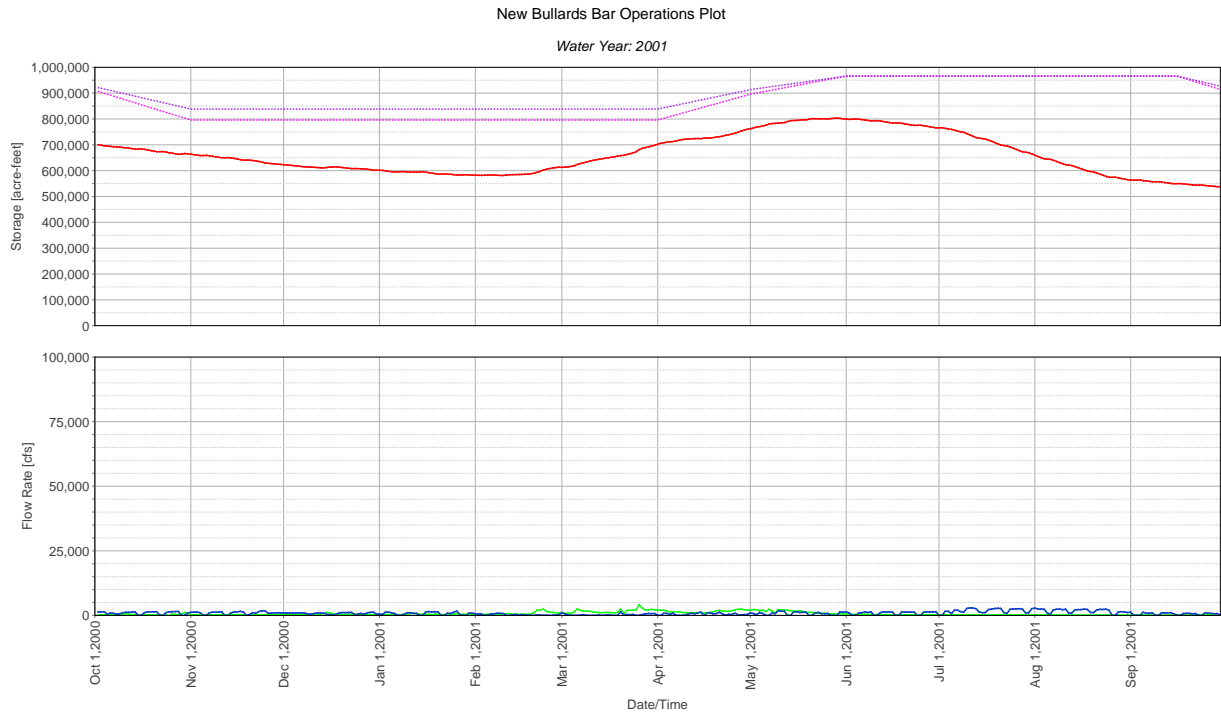
A: Reoperation Plots



A: Reoperation Plots

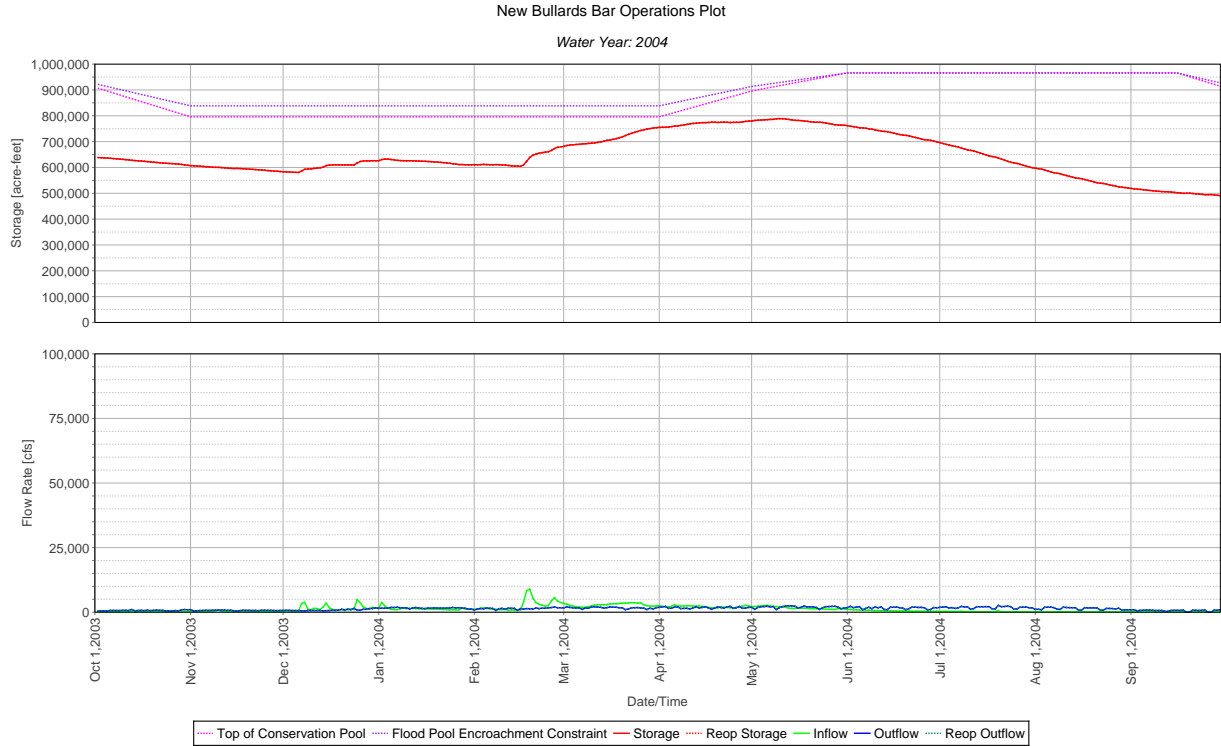
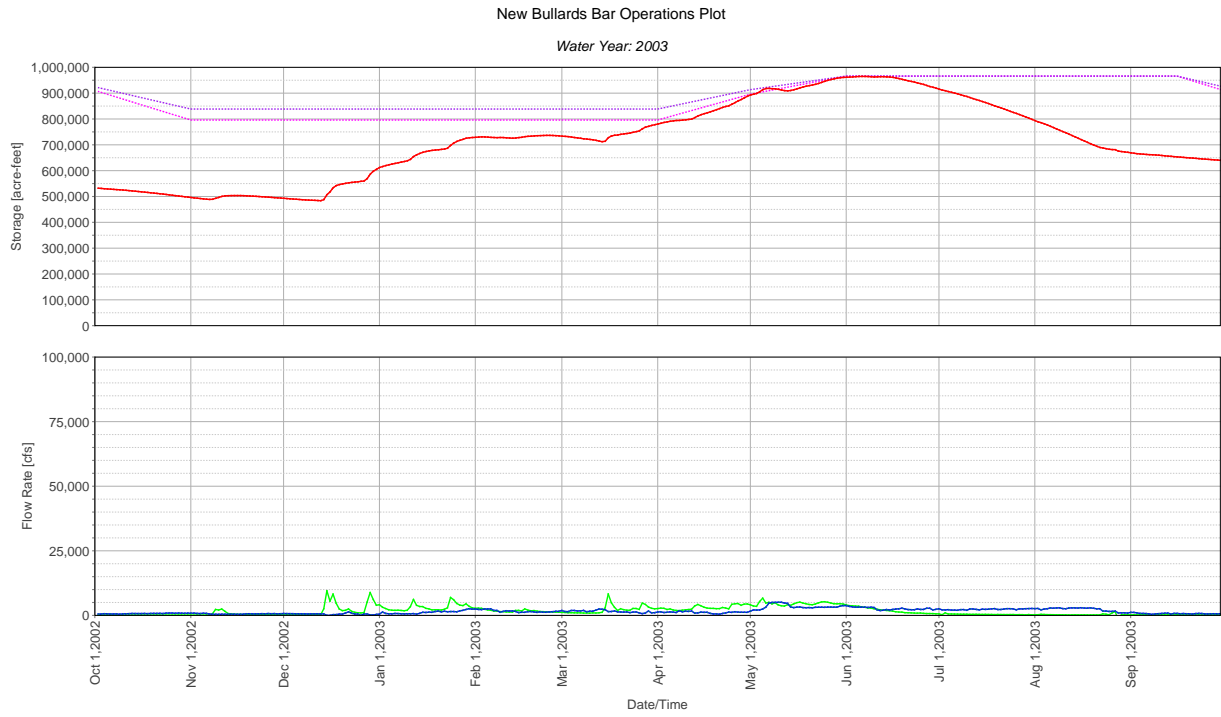


A: Reoperation Plots



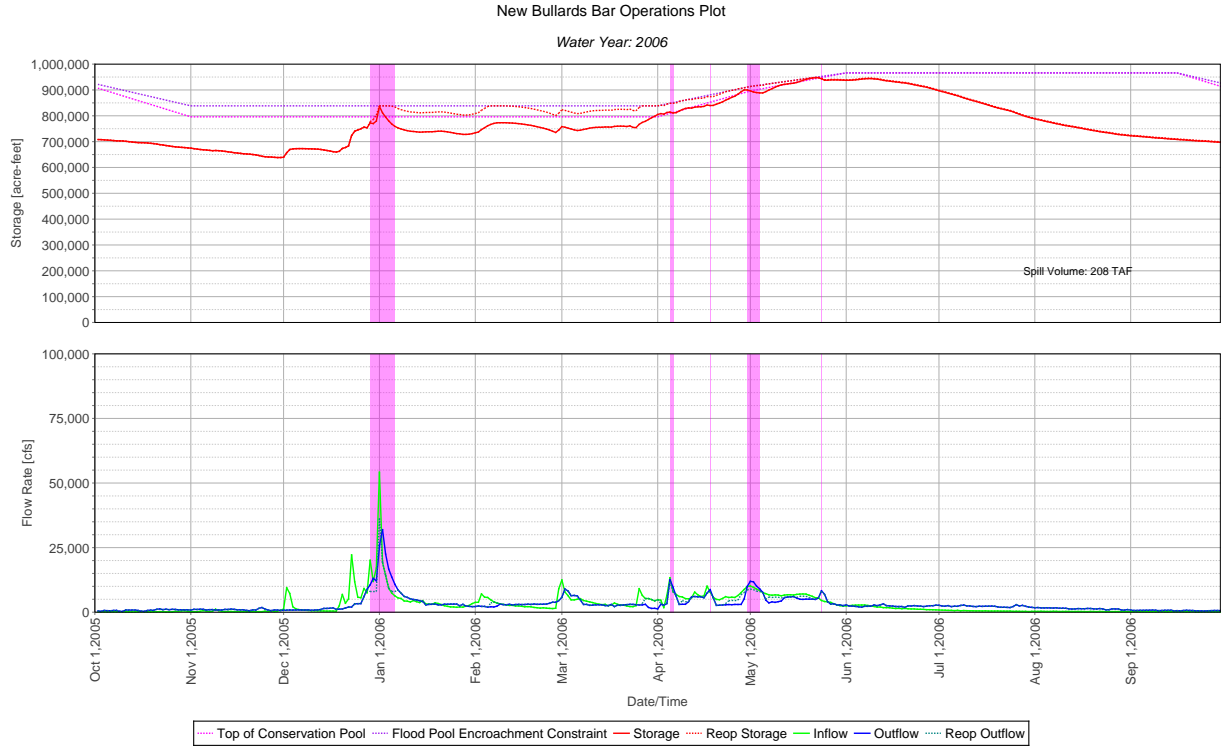
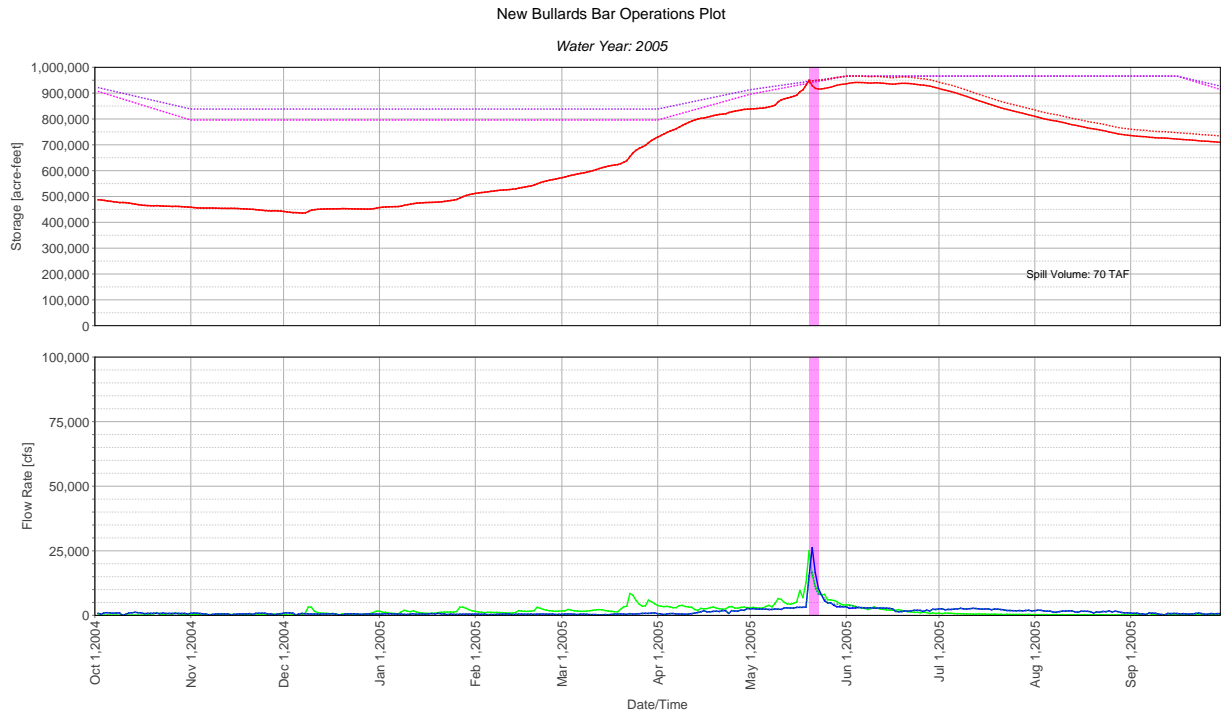
..... Top of Conservation Pool
 Flood Pool Encroachment Constraint
 —— Storage
 Reop Storage
 —— Inflow
 —— Outflow
 Reop Outflow

A: Reoperation Plots

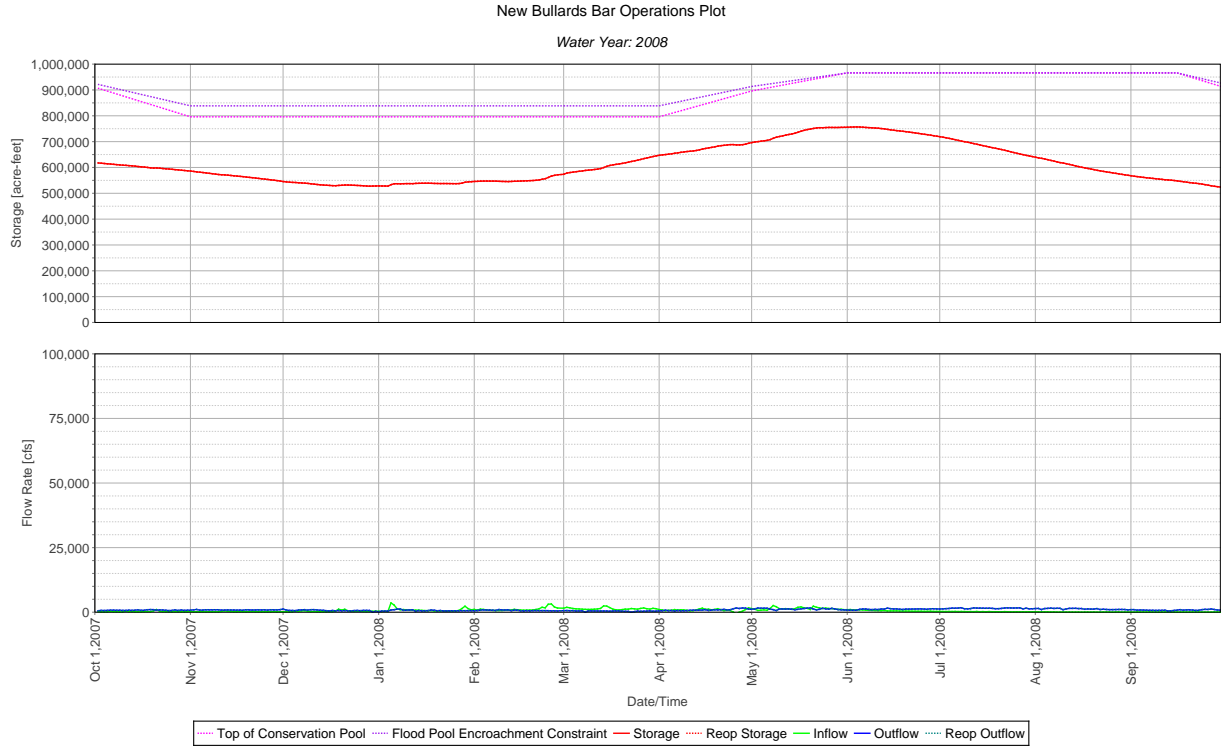
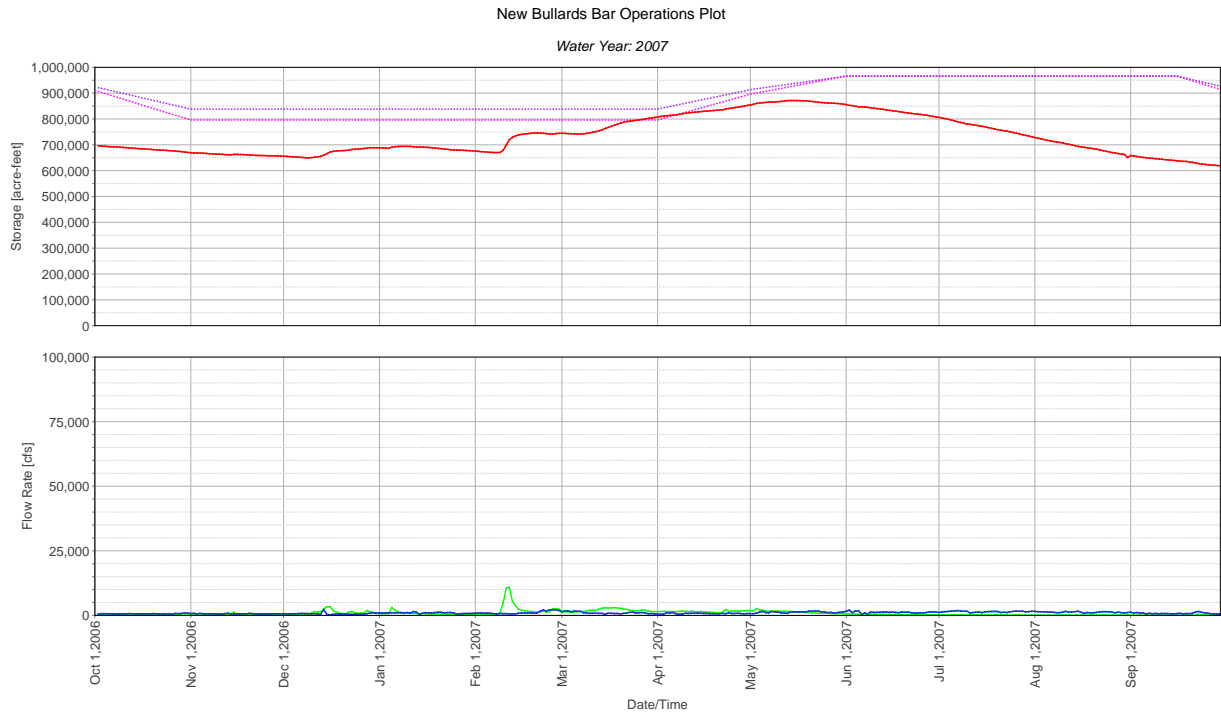


..... Top of Conservation Pool
 Flood Pool Encroachment Constraint
 ——— Storage
 Reop Storage
 ——— Inflow
 ——— Outflow
 Reop Outflow

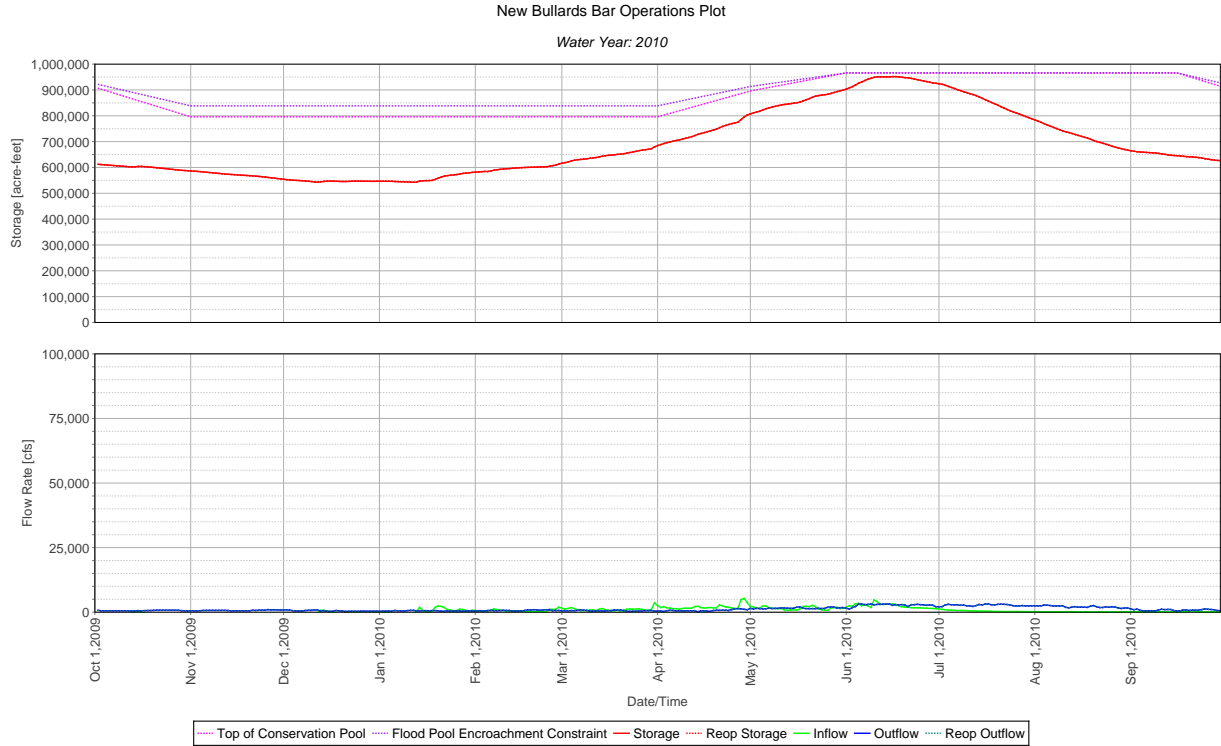
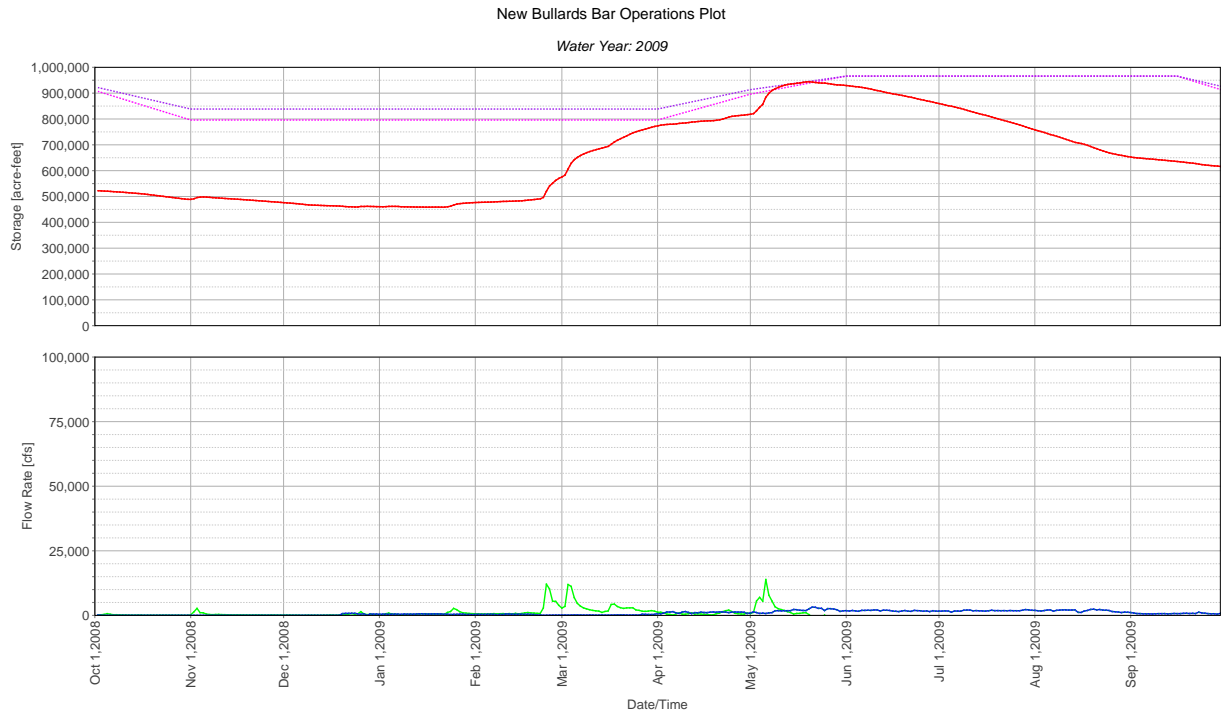
A: Reoperation Plots



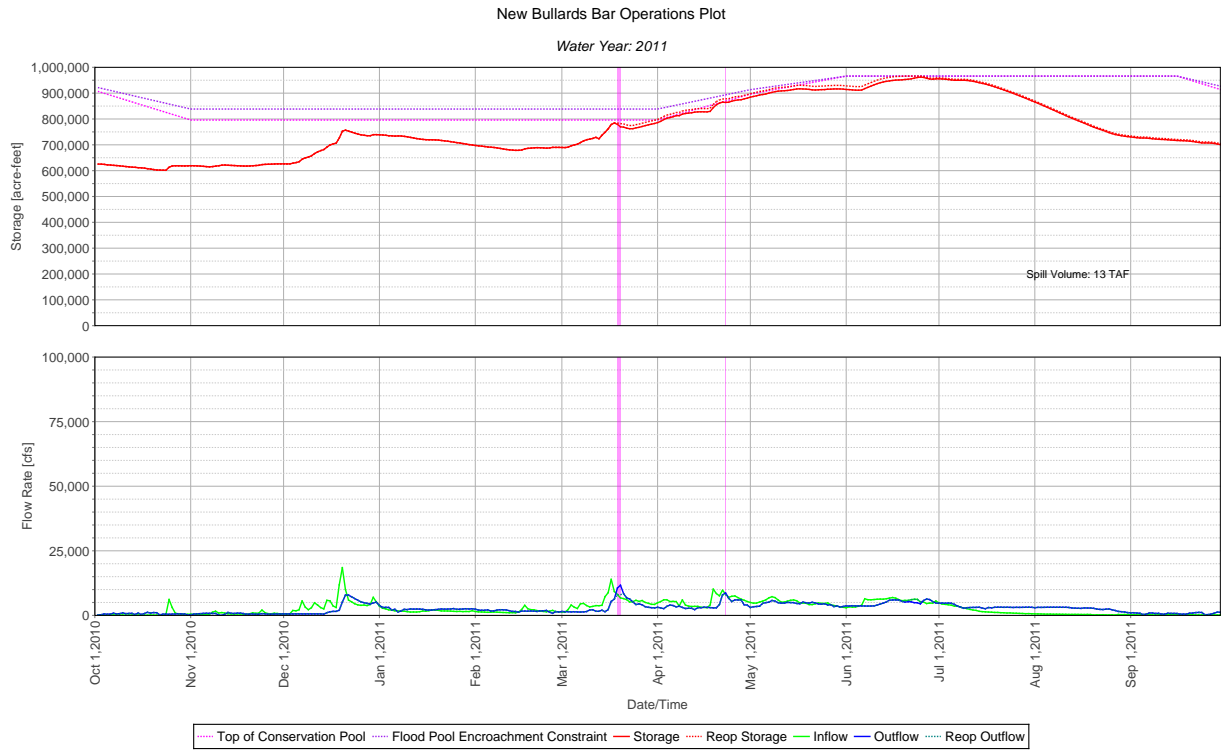
A: Reoperation Plots



A: Reoperation Plots



A: Reoperation Plots



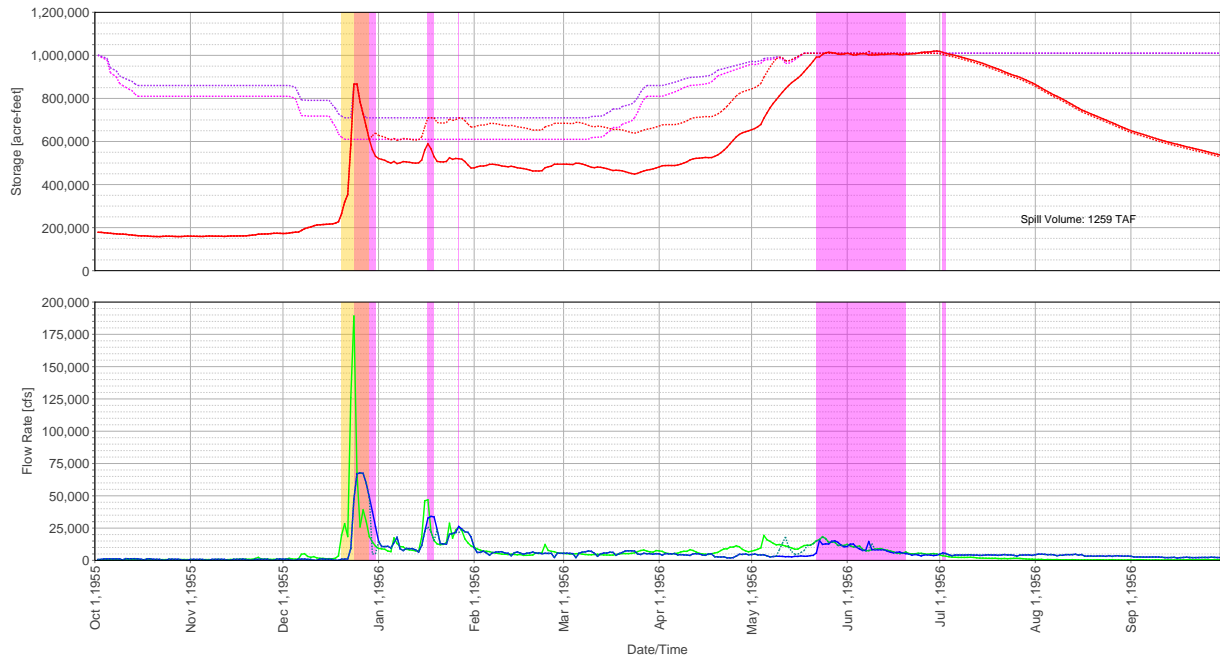
A.4 Folsom

The following section includes the reoperation plots for Folsom Dam.

A: Reoperation Plots

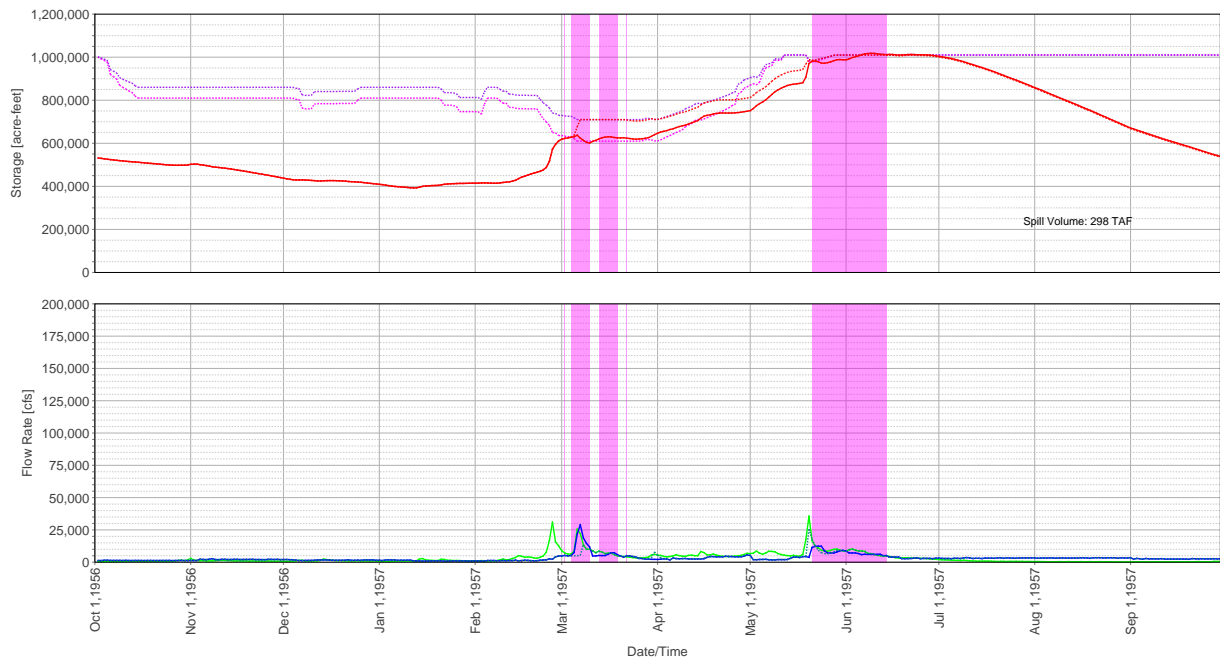
Folsom Operations Plot

Water Year: 1956



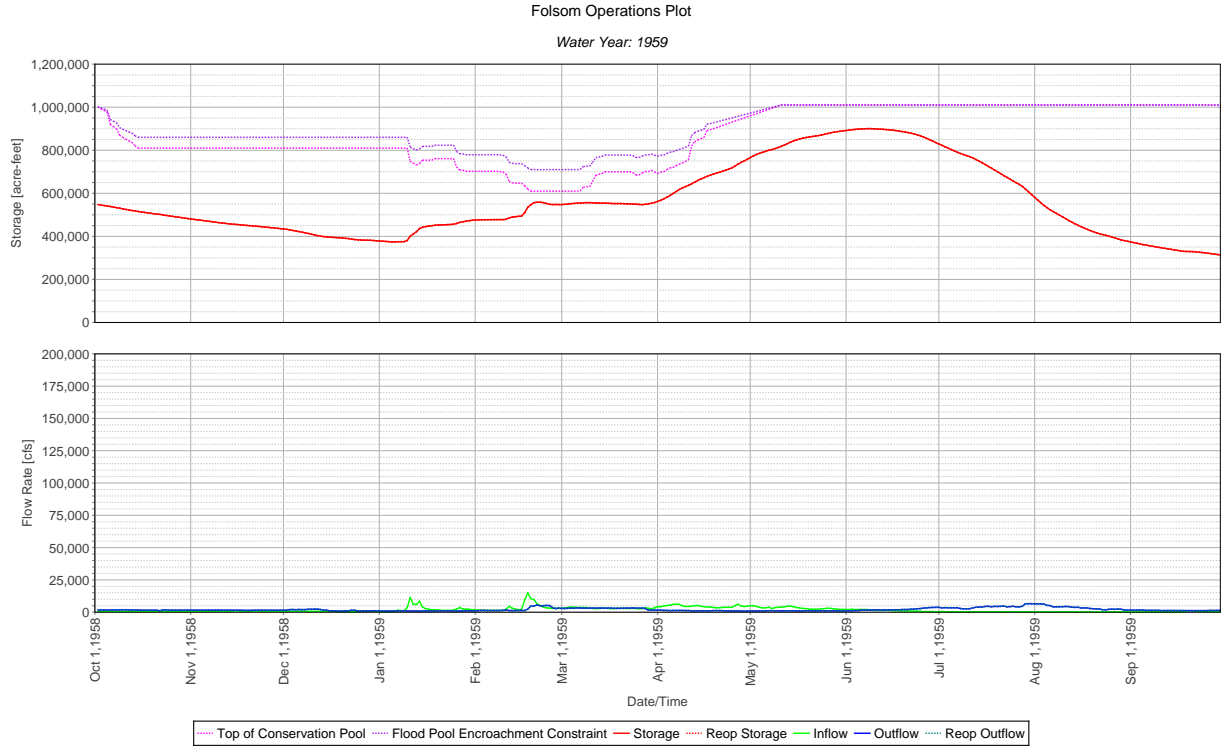
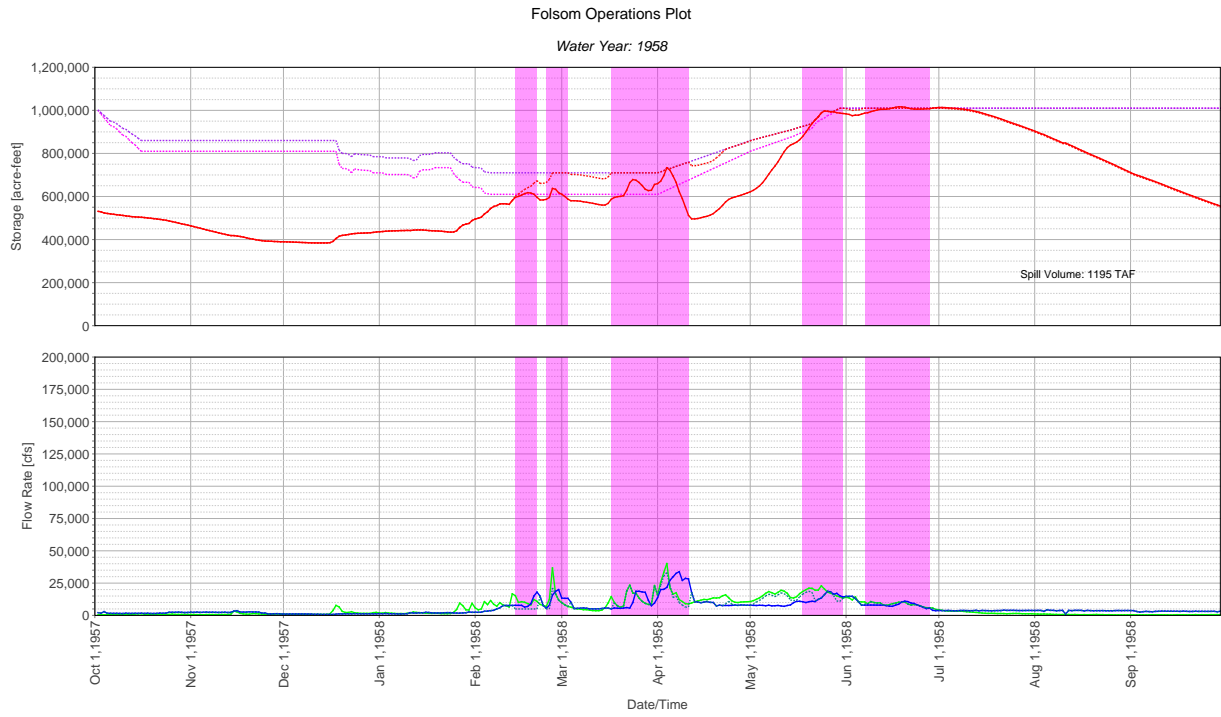
Folsom Operations Plot

Water Year: 1957

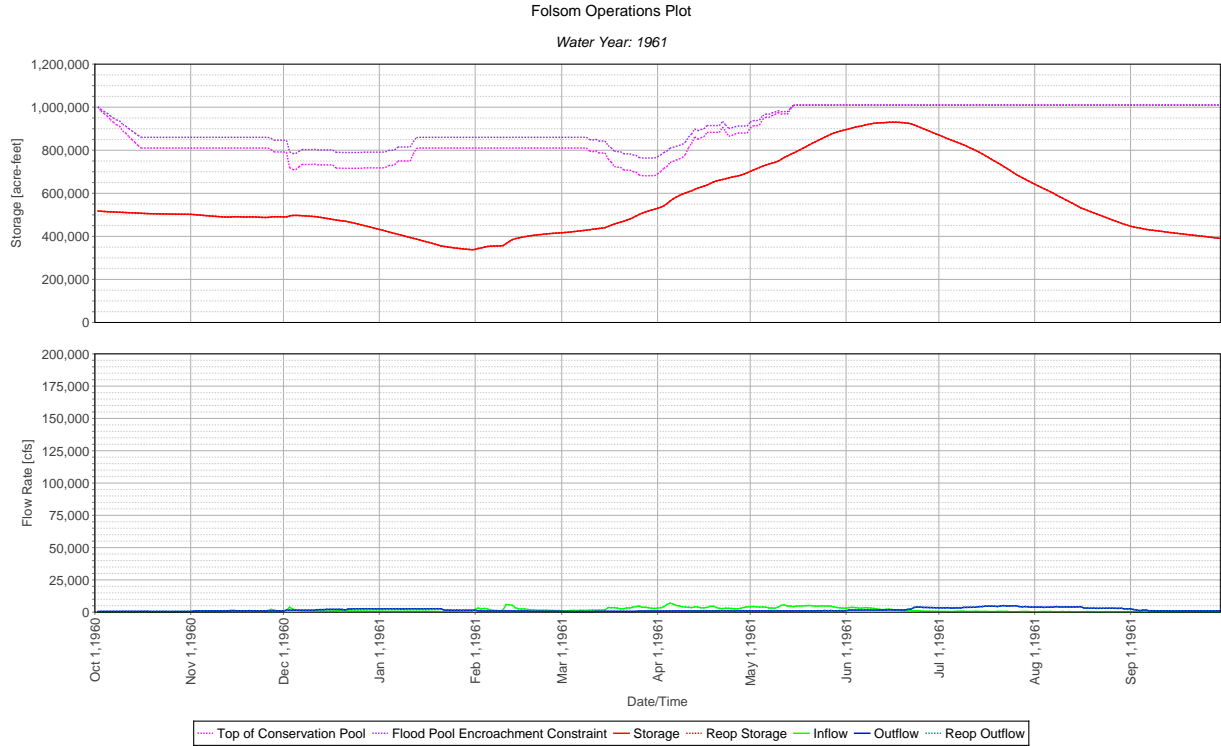
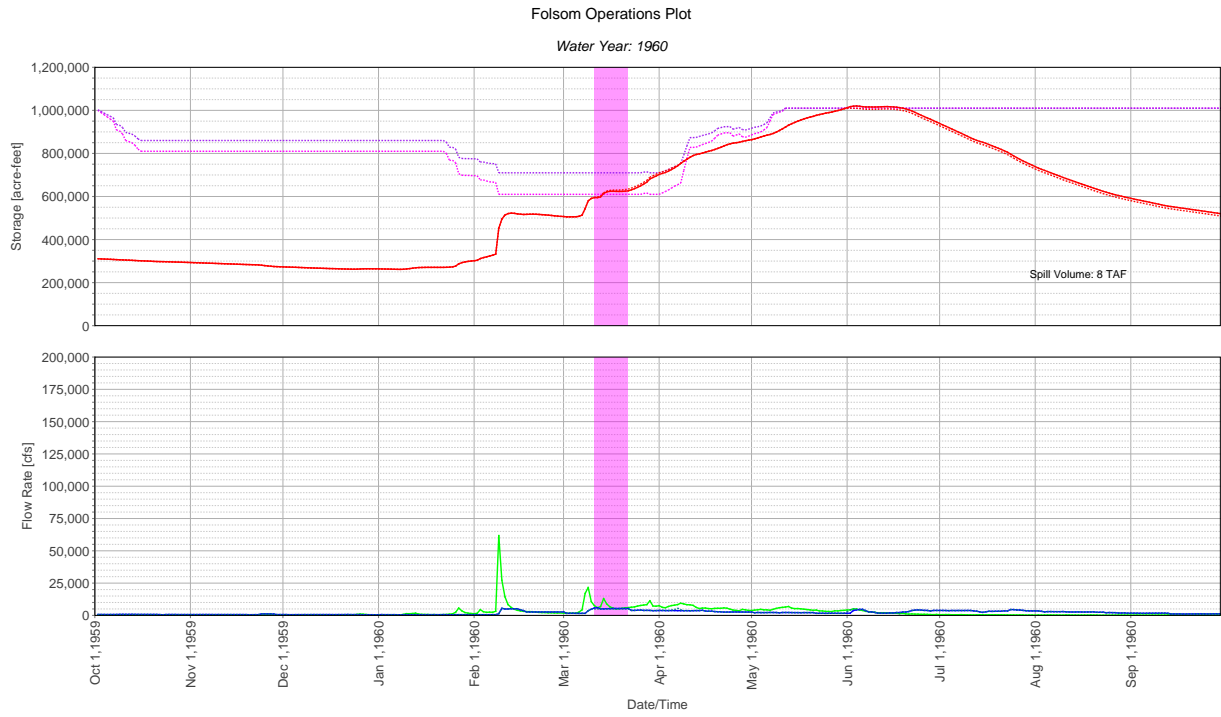


..... Top of Conservation Pool
 Flood Pool Encroachment Constraint
 —— Storage
 Reop Storage
 —— Inflow
 —— Outflow
 Reop Outflow

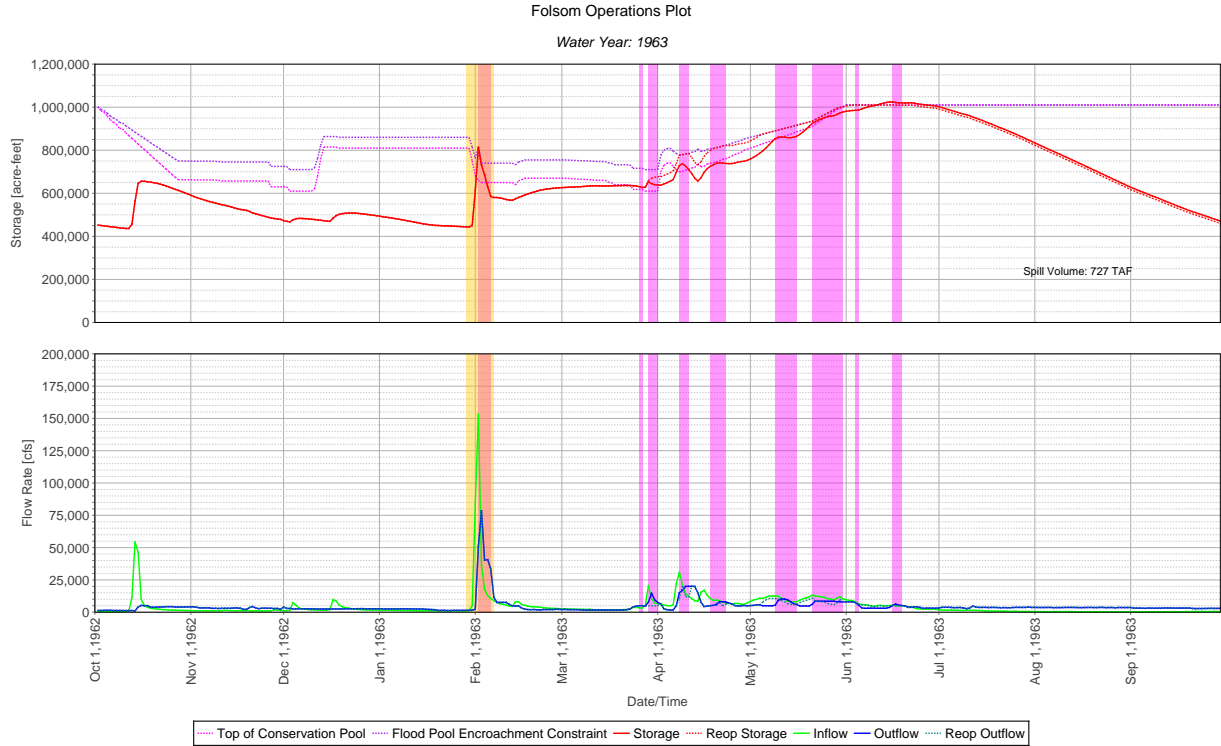
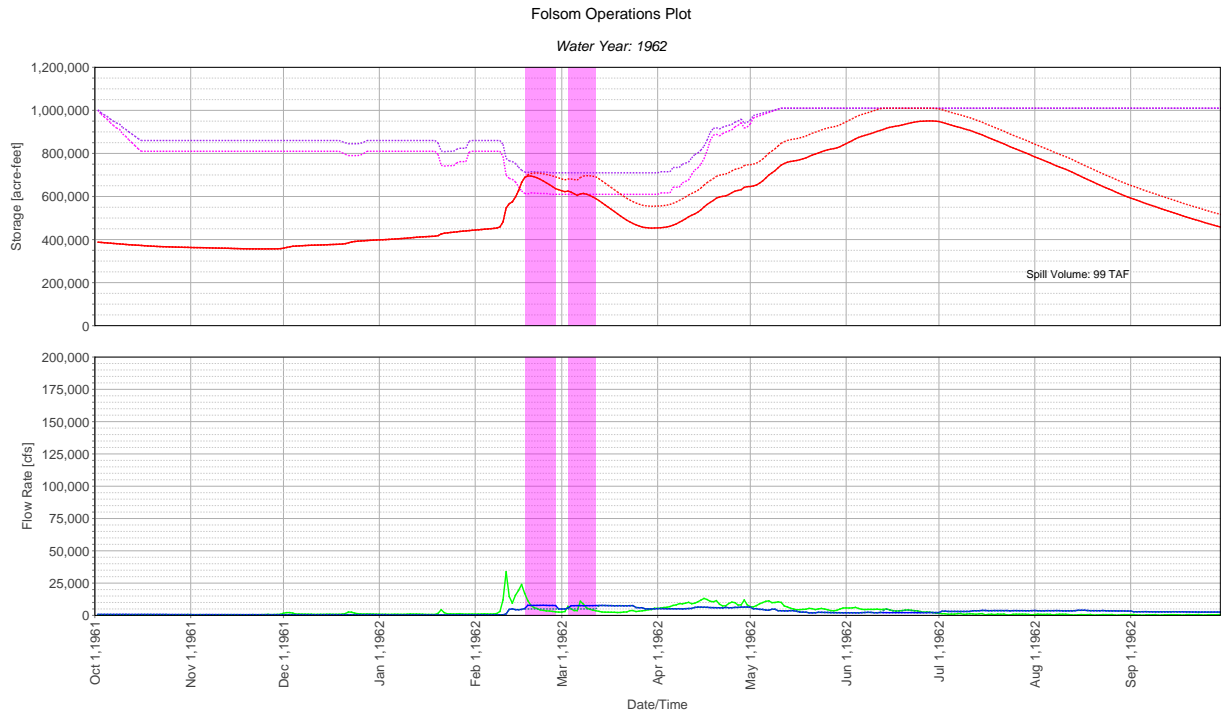
A: Reoperation Plots



A: Reoperation Plots



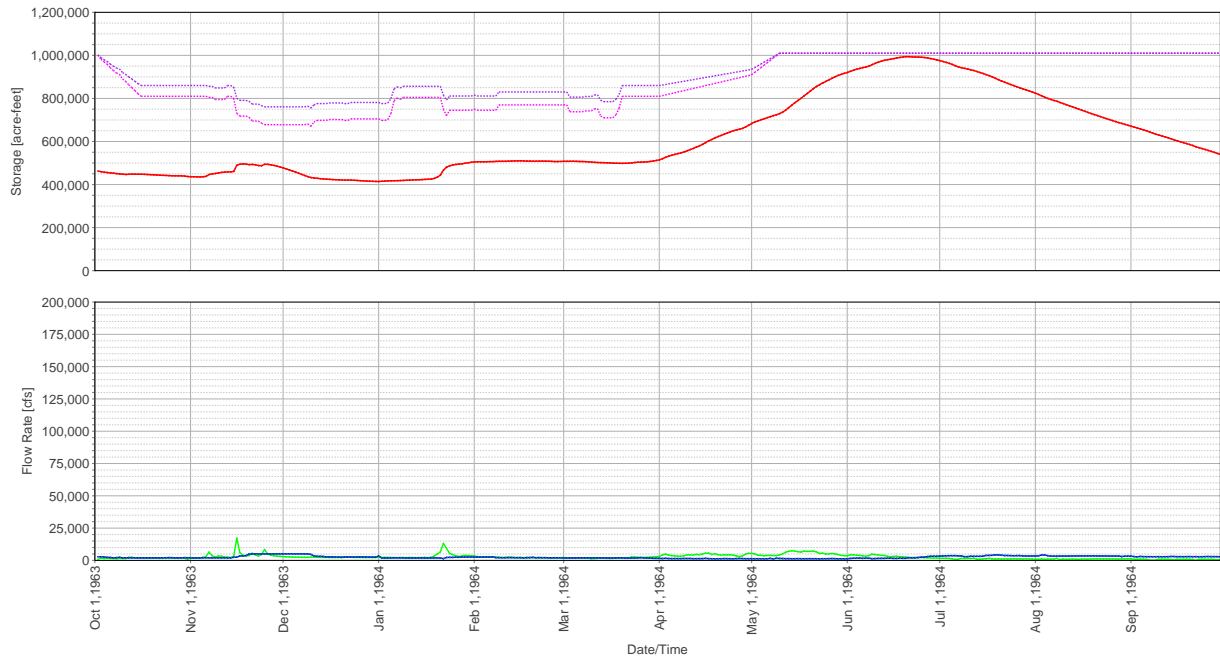
A: Reoperation Plots



A: Reoperation Plots

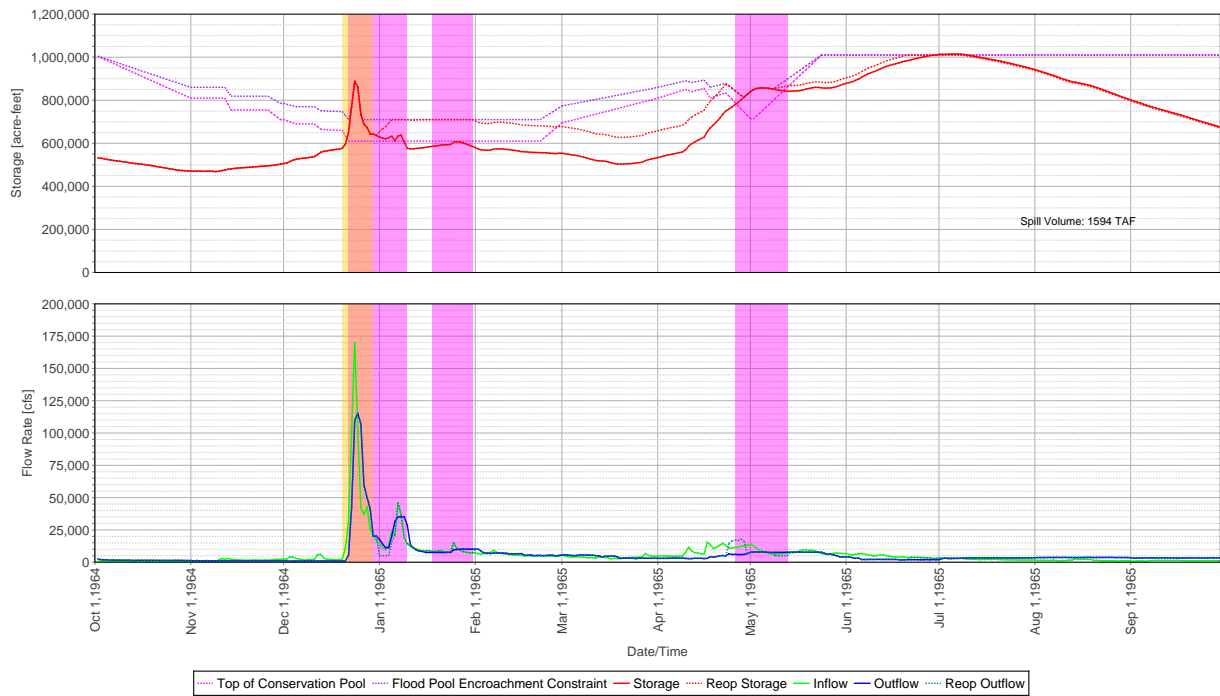
Folsom Operations Plot

Water Year: 1964



Folsom Operations Plot

Water Year: 1965

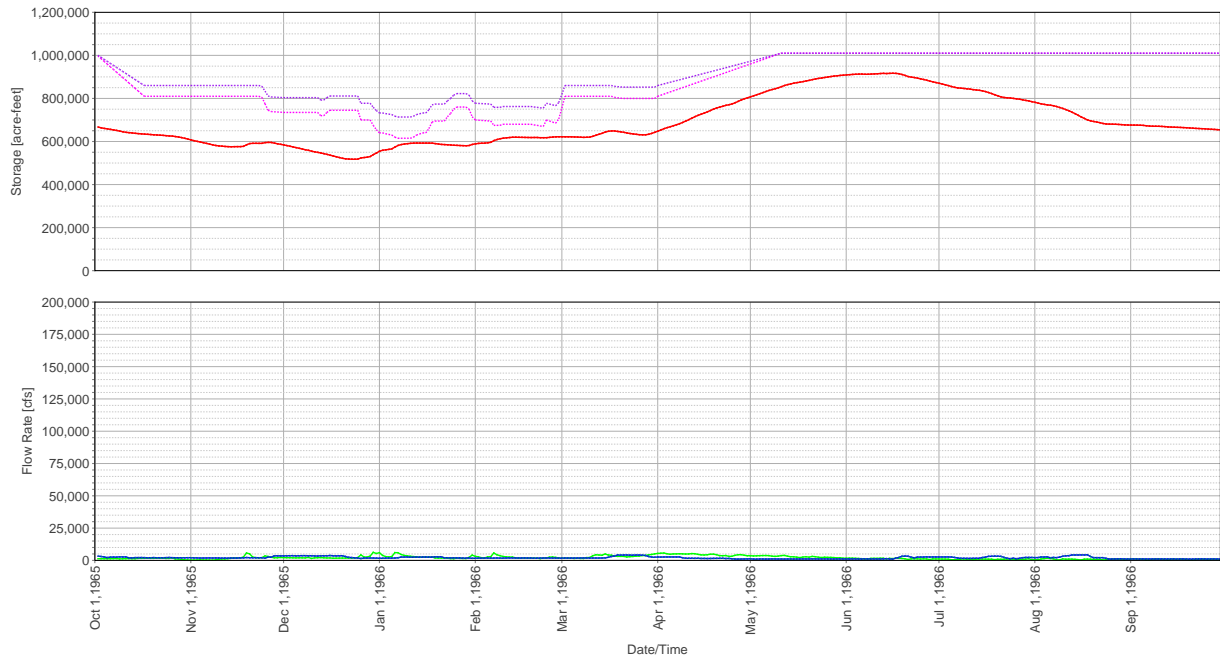


---- Top of Conservation Pool
 ---- Flood Pool Encroachment Constraint
 — Storage
 ---- Reop Storage
 — Inflow
 — Outflow
 ---- Reop Outflow

A: Reoperation Plots

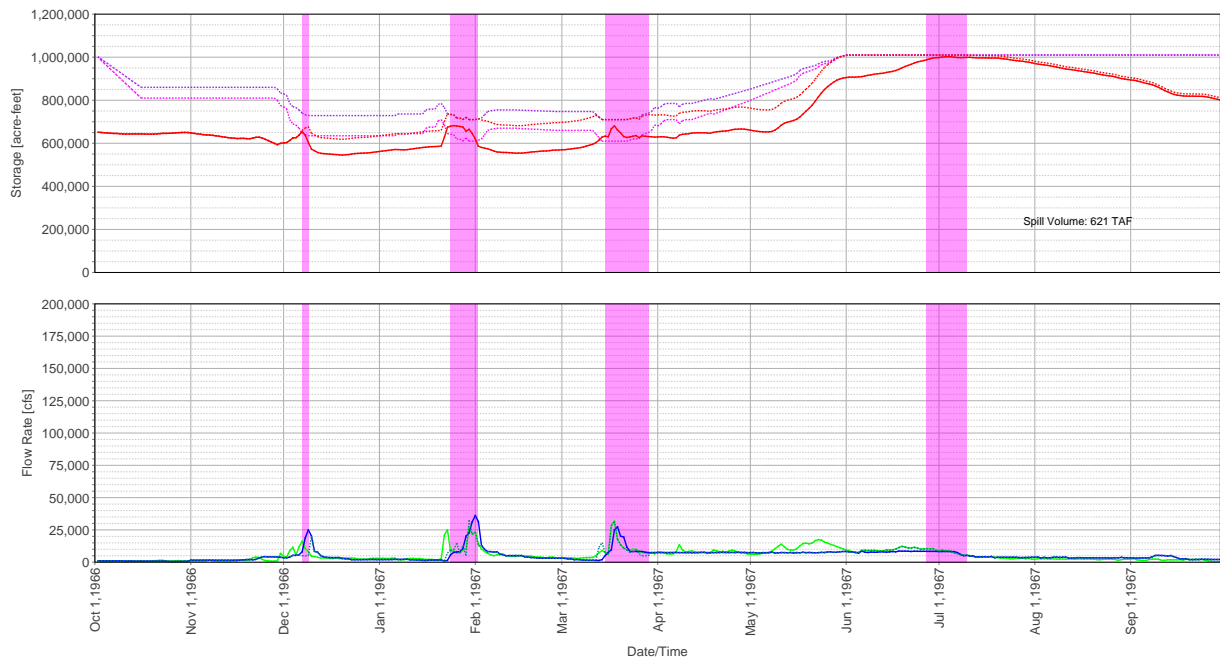
Folsom Operations Plot

Water Year: 1966



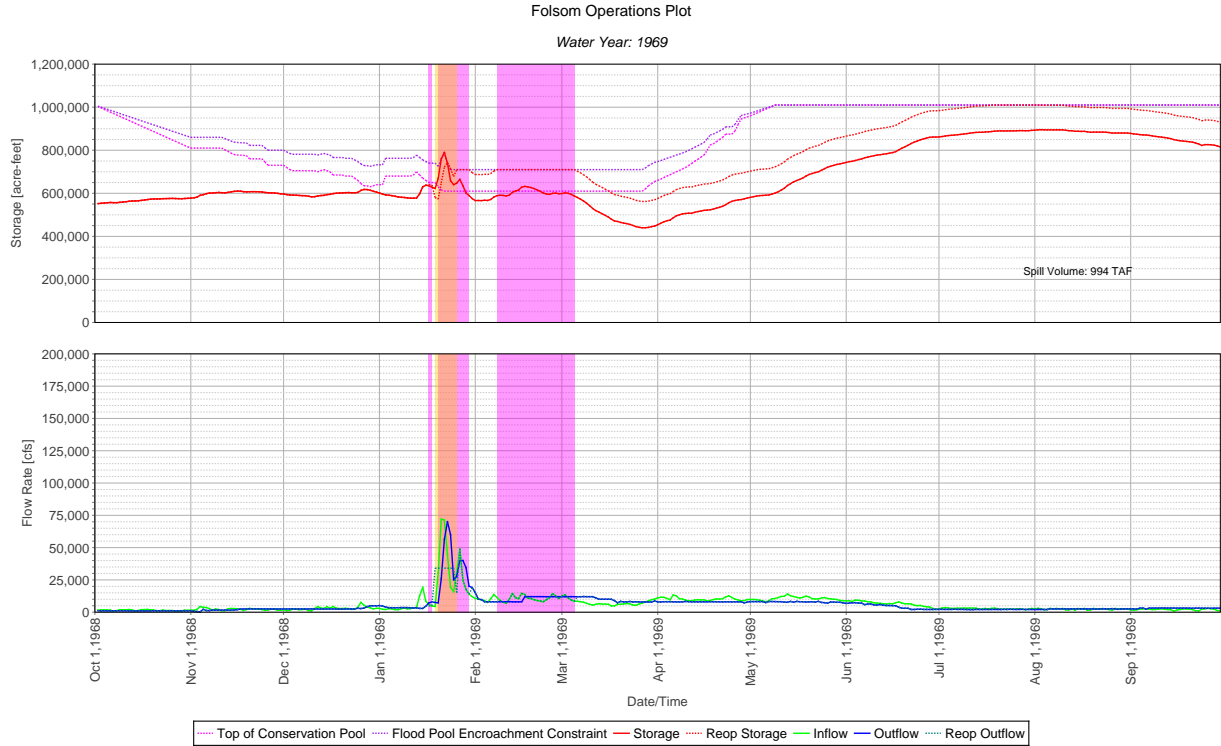
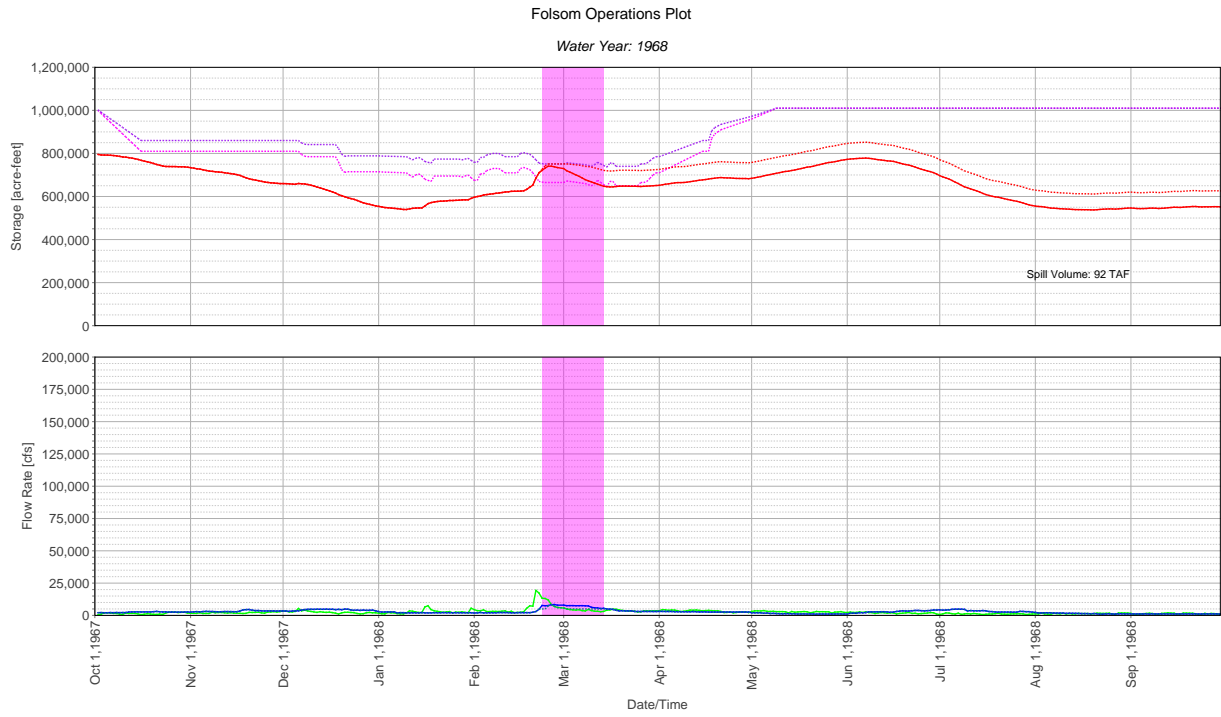
Folsom Operations Plot

Water Year: 1967

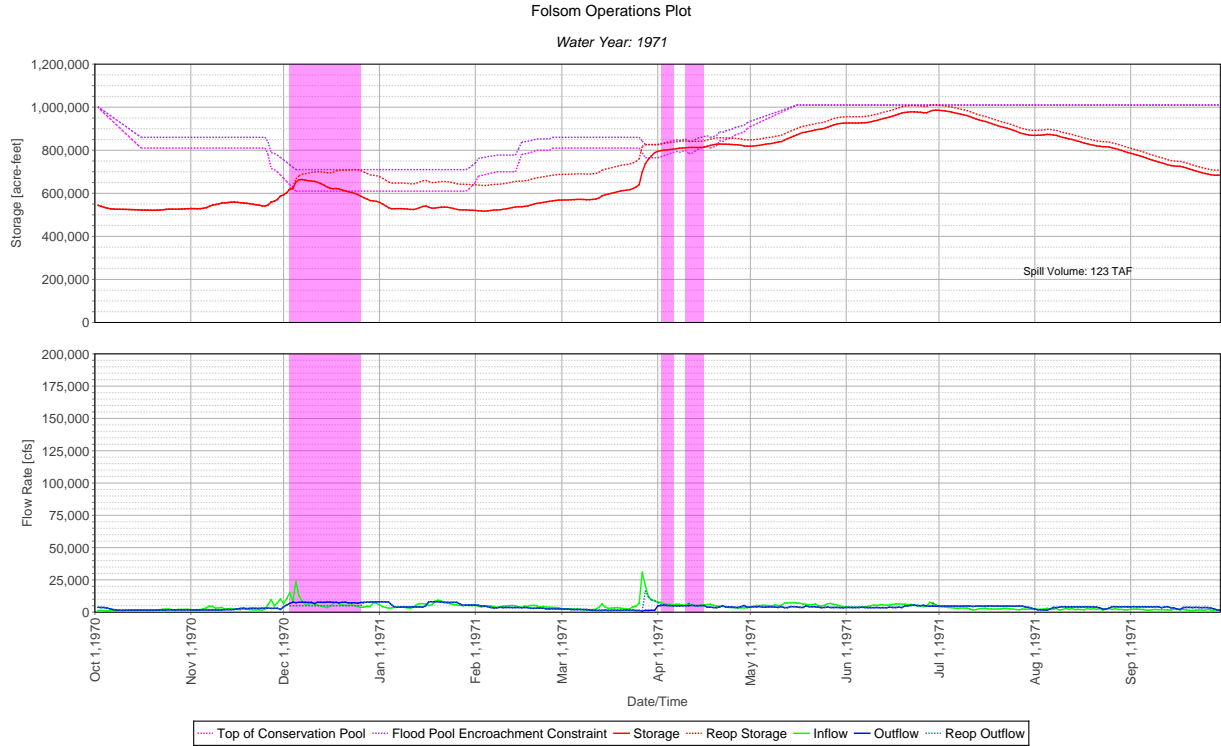
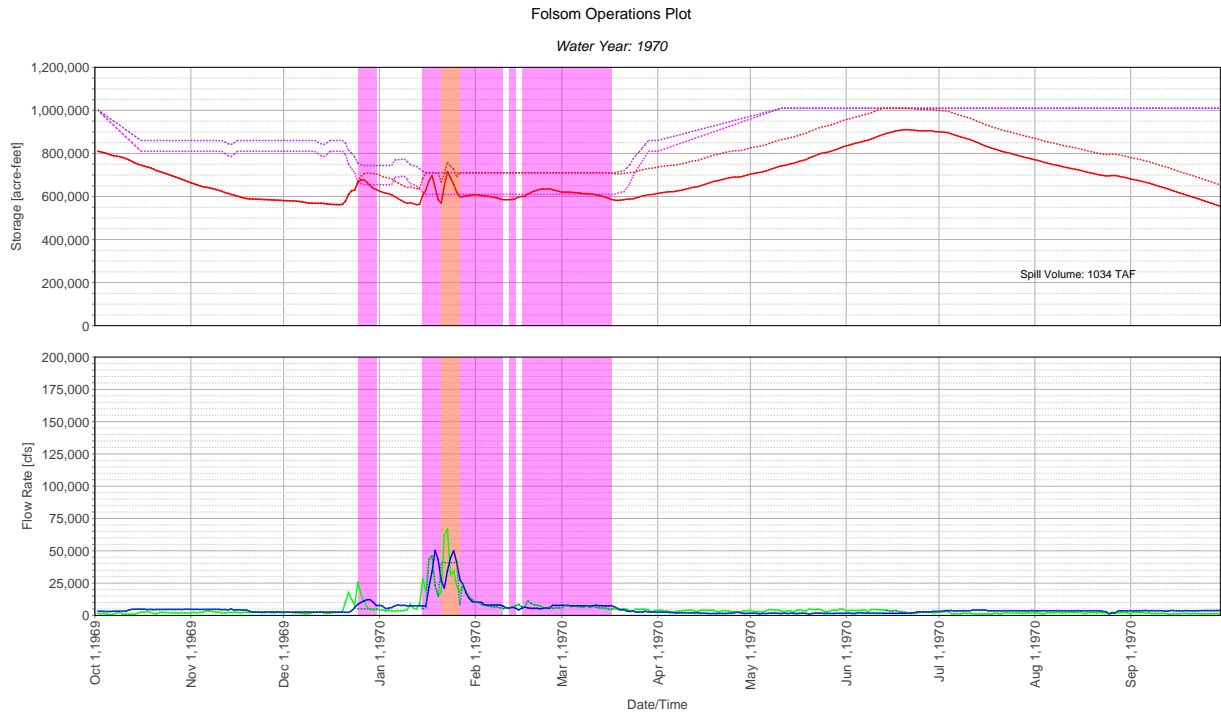


..... Top of Conservation Pool
 Flood Pool Encroachment Constraint
 —— Storage
 Reop Storage
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 —— Outflow
 Reop Outflow

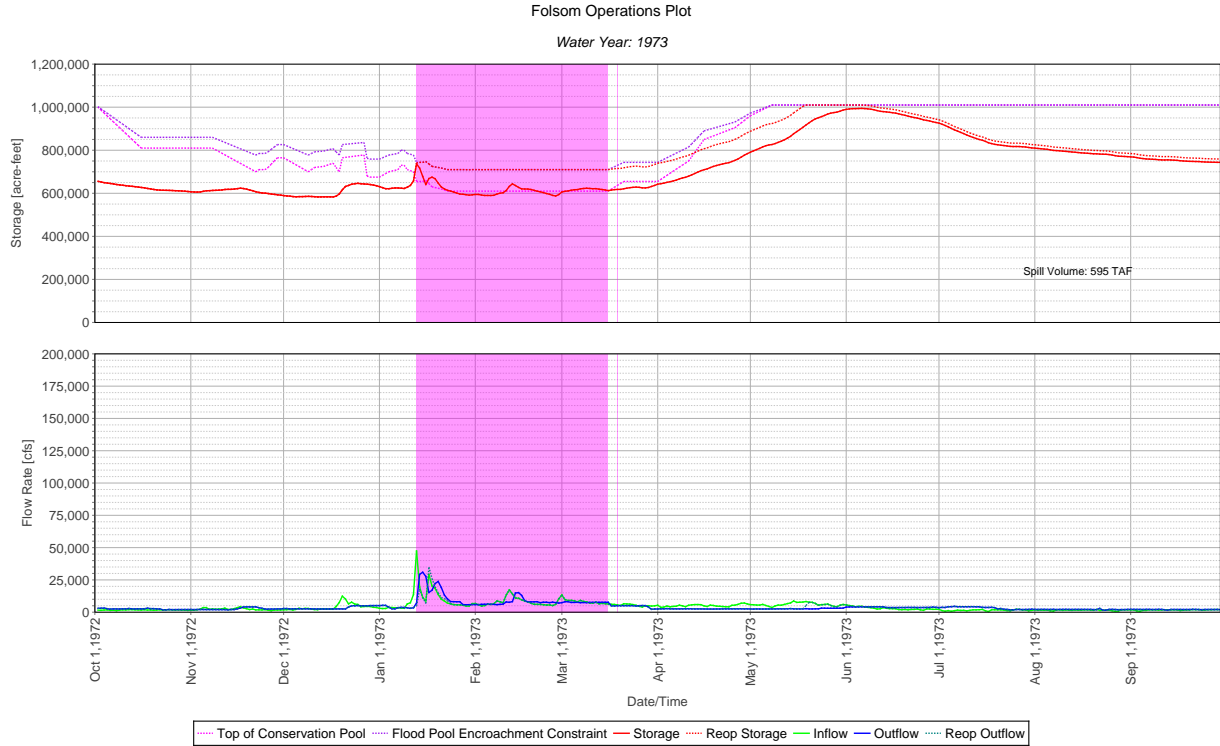
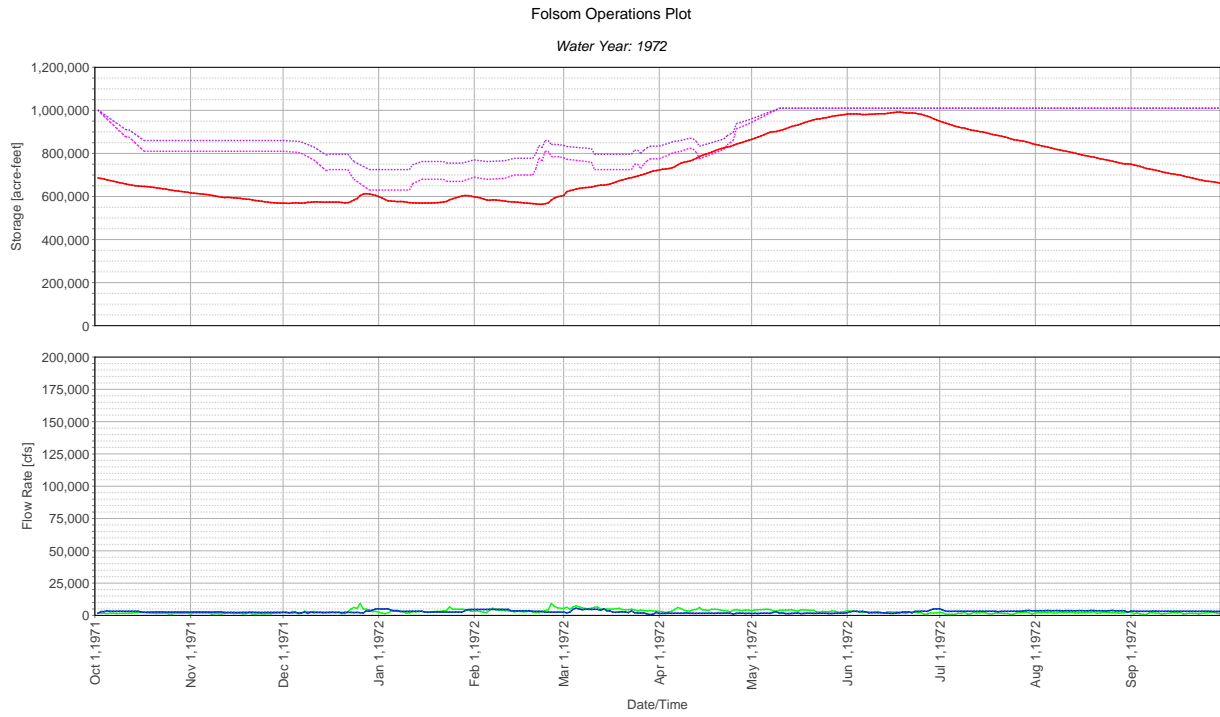
A: Reoperation Plots



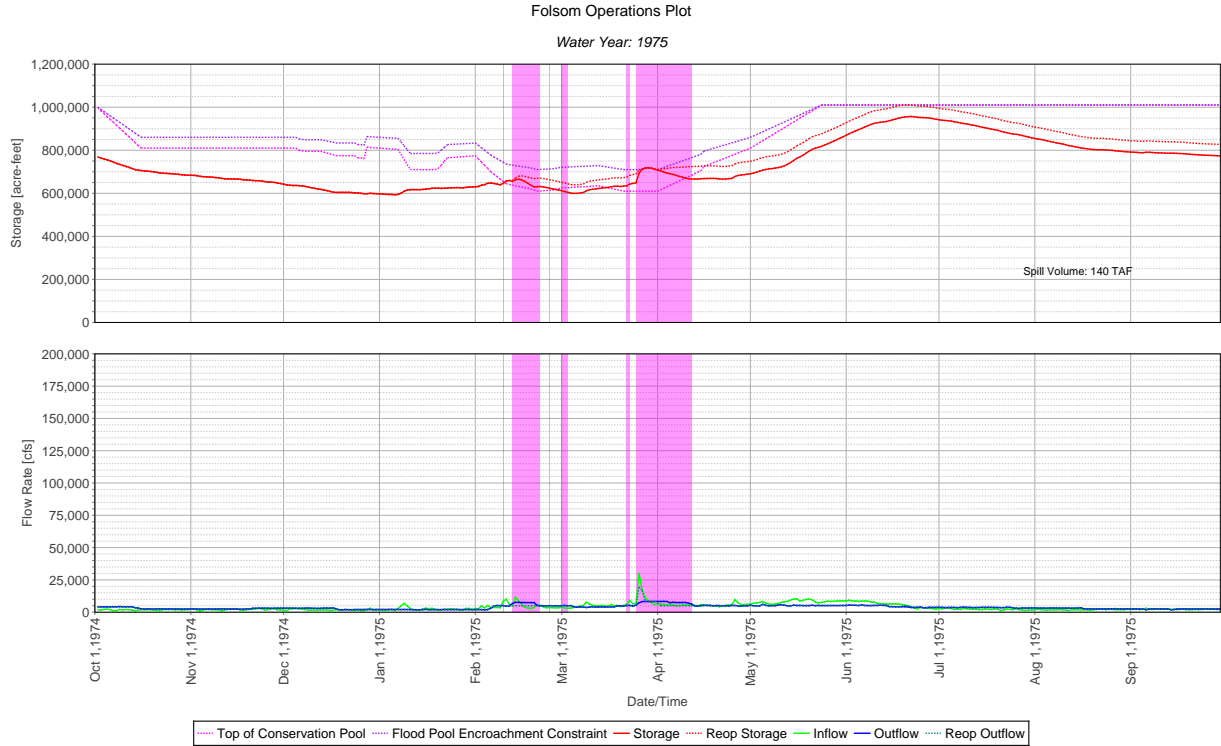
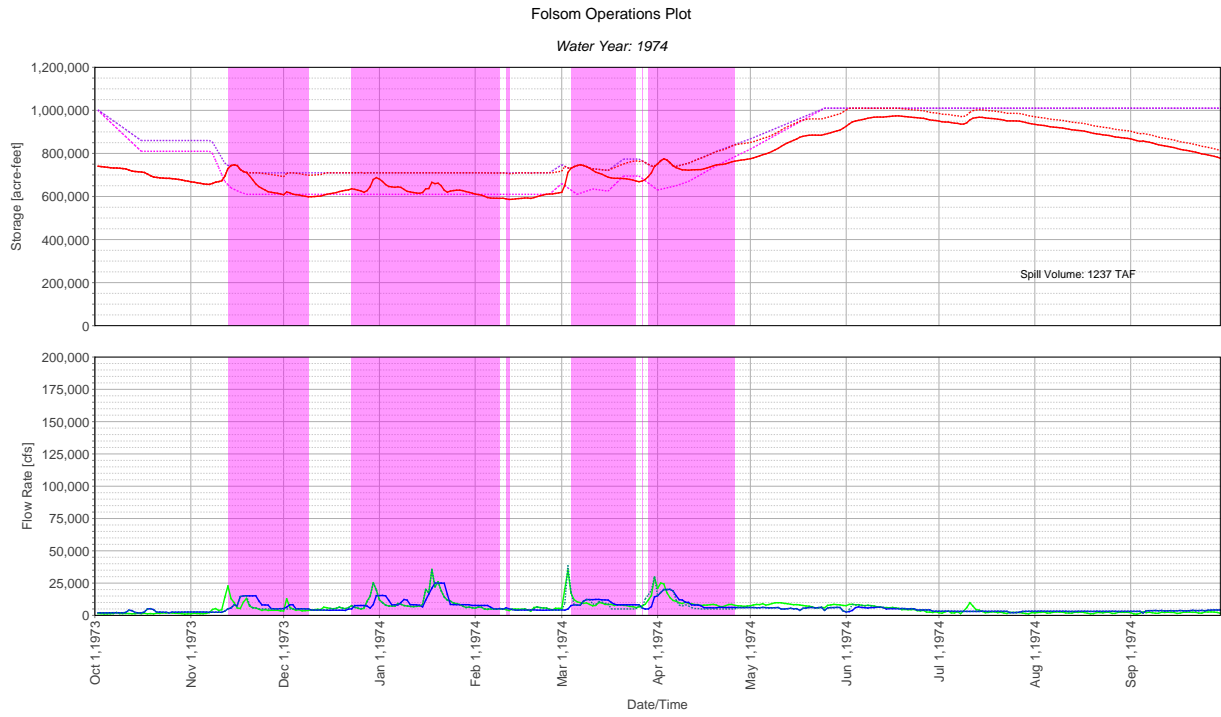
A: Reoperation Plots



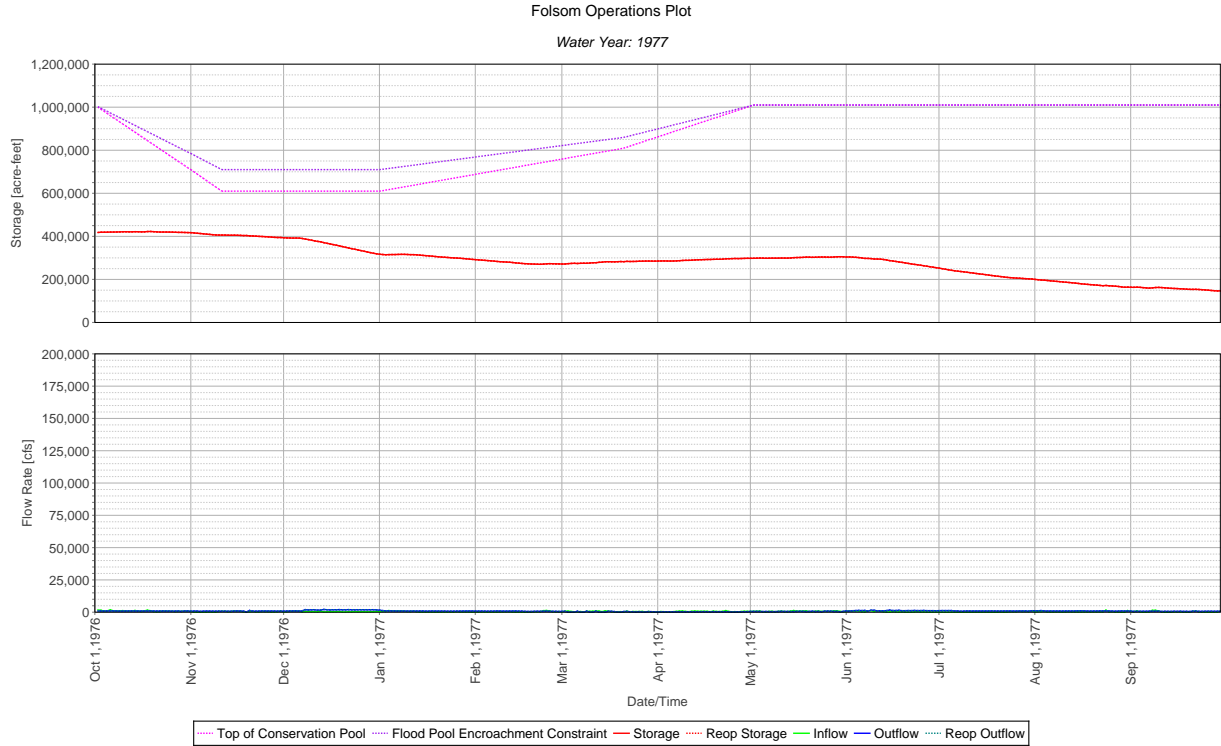
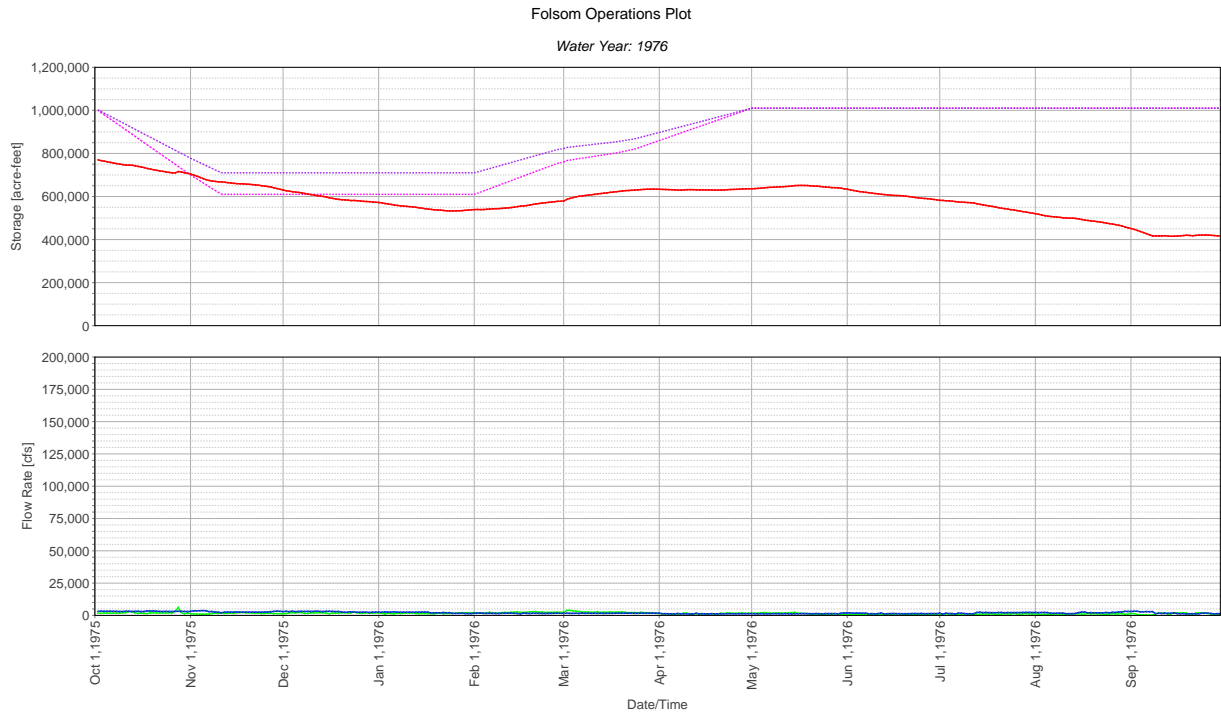
A: Reoperation Plots



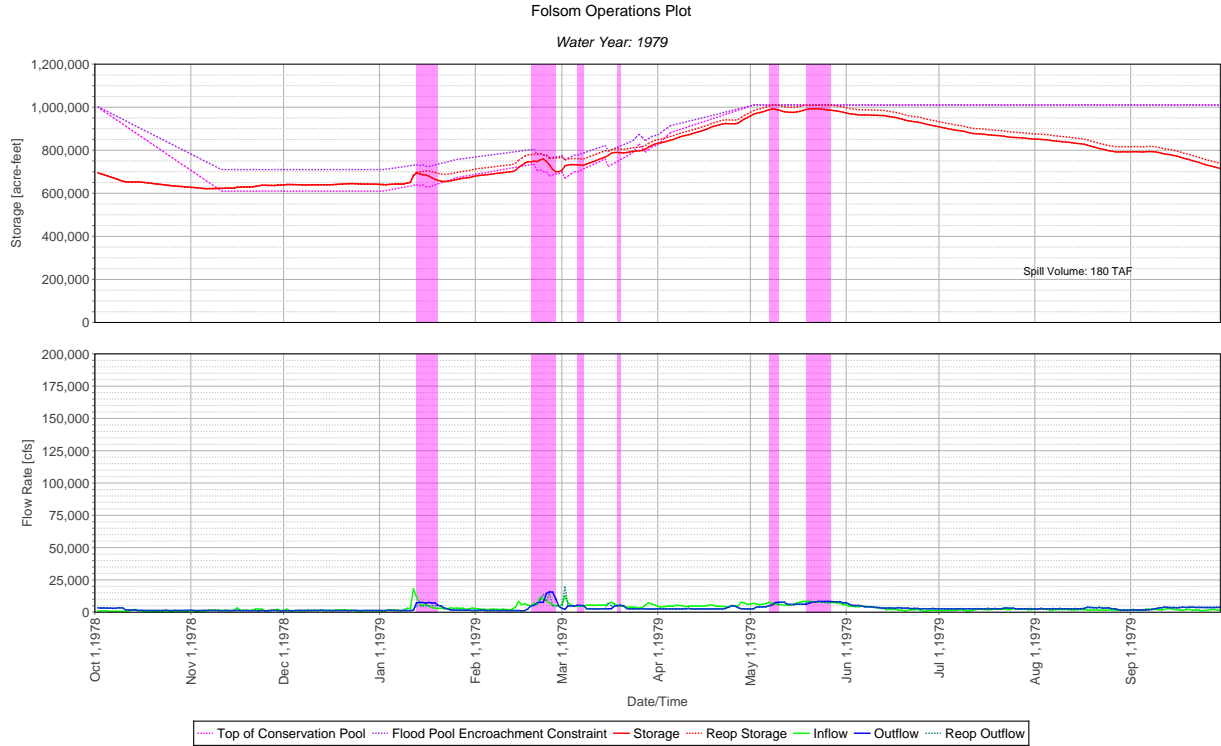
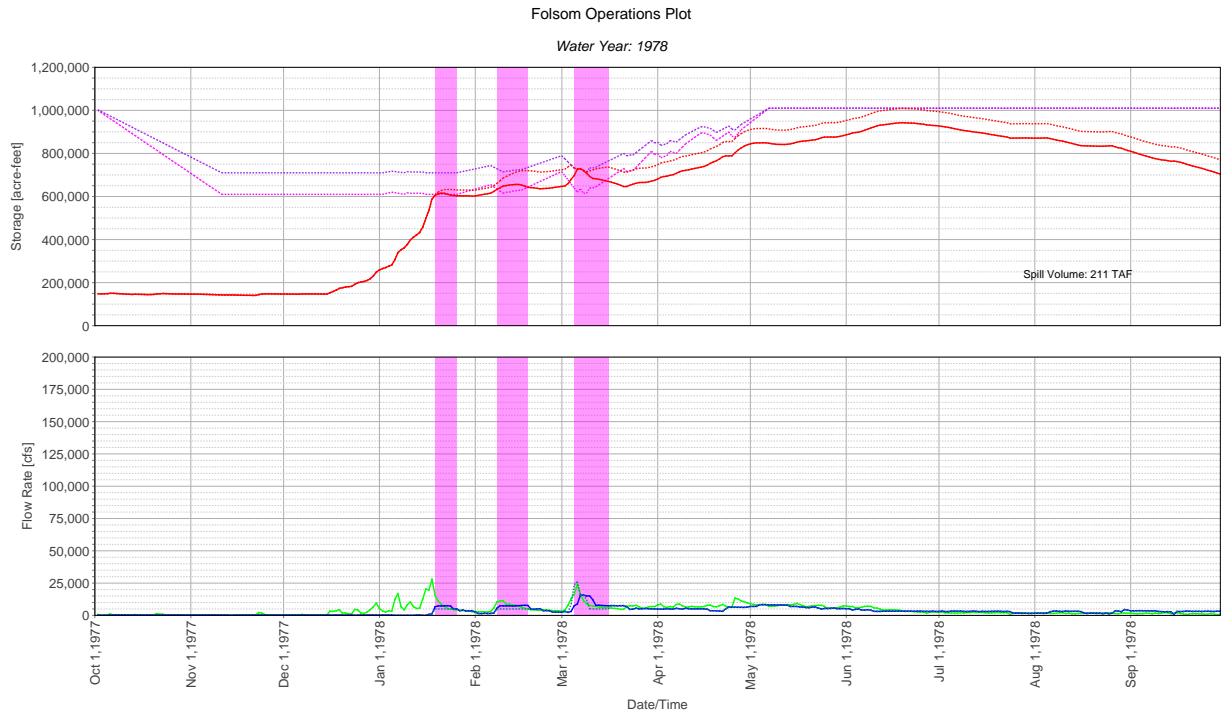
A: Reoperation Plots



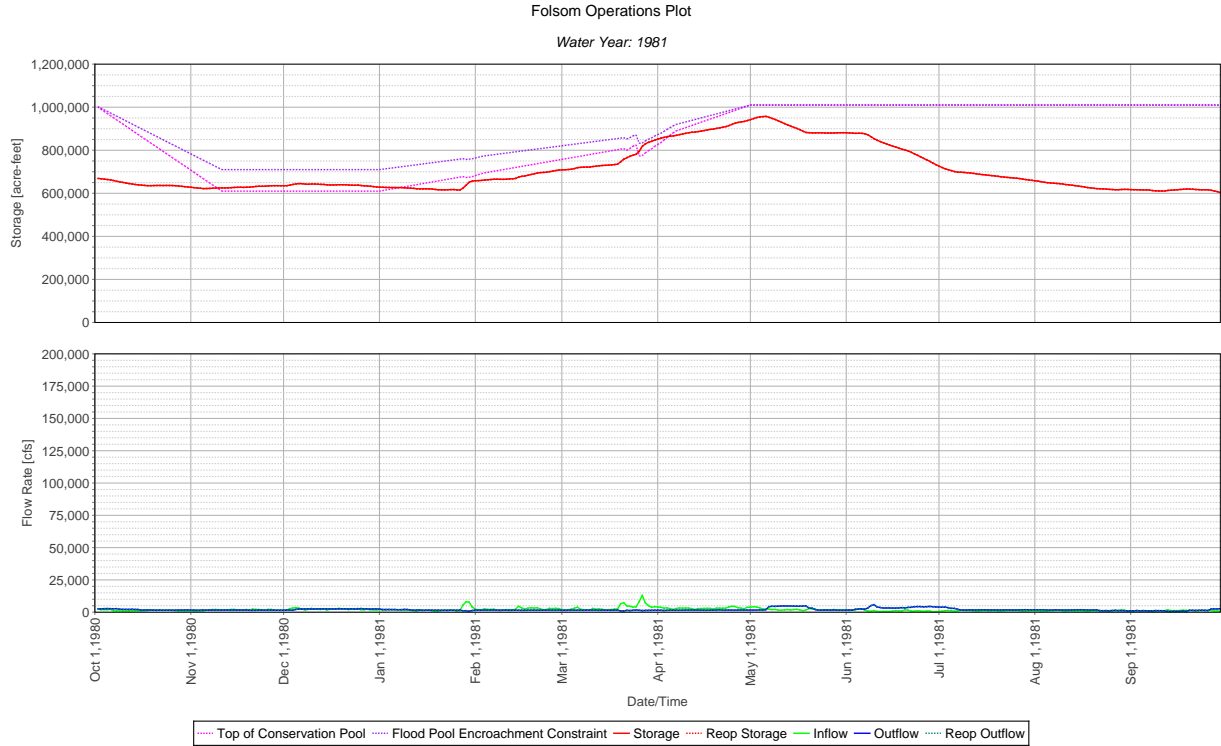
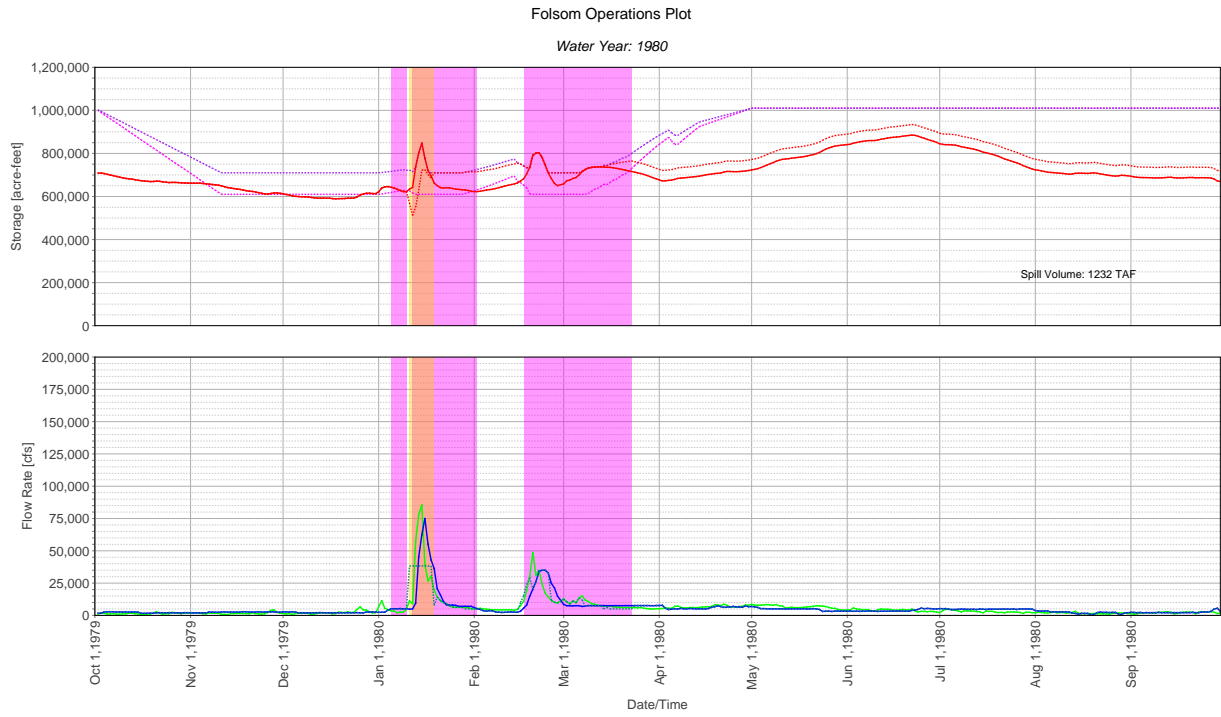
A: Reoperation Plots



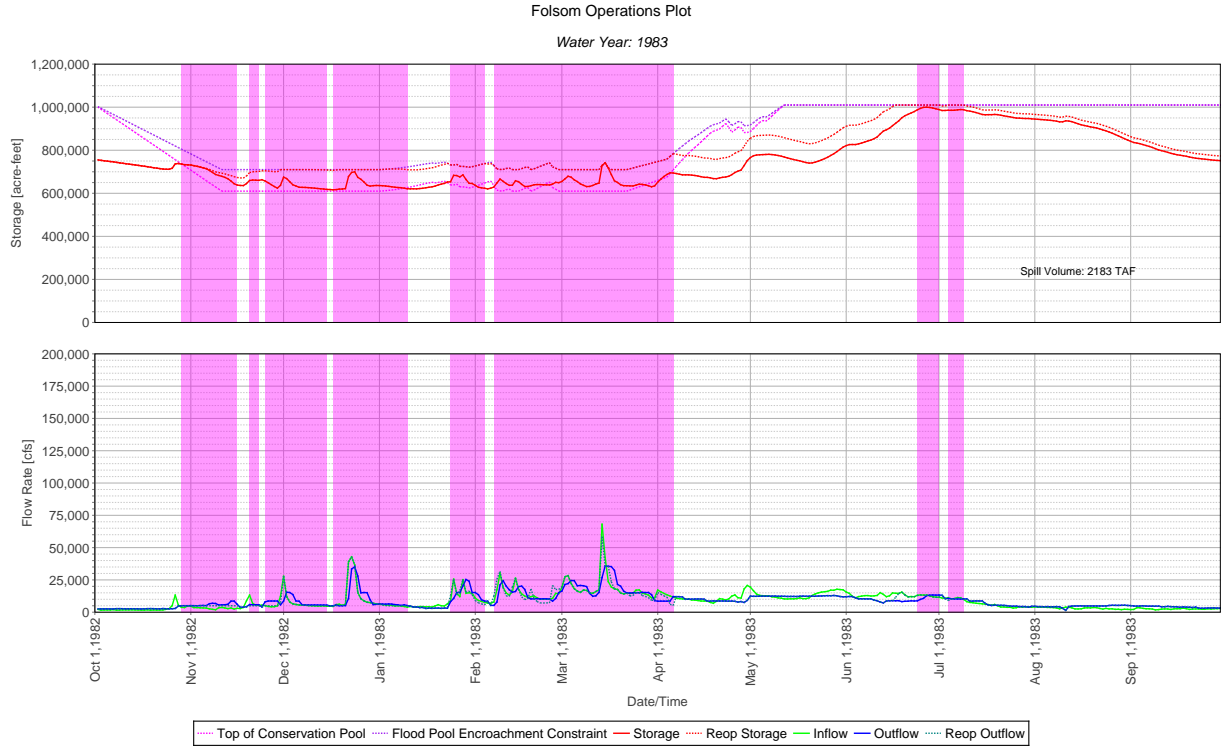
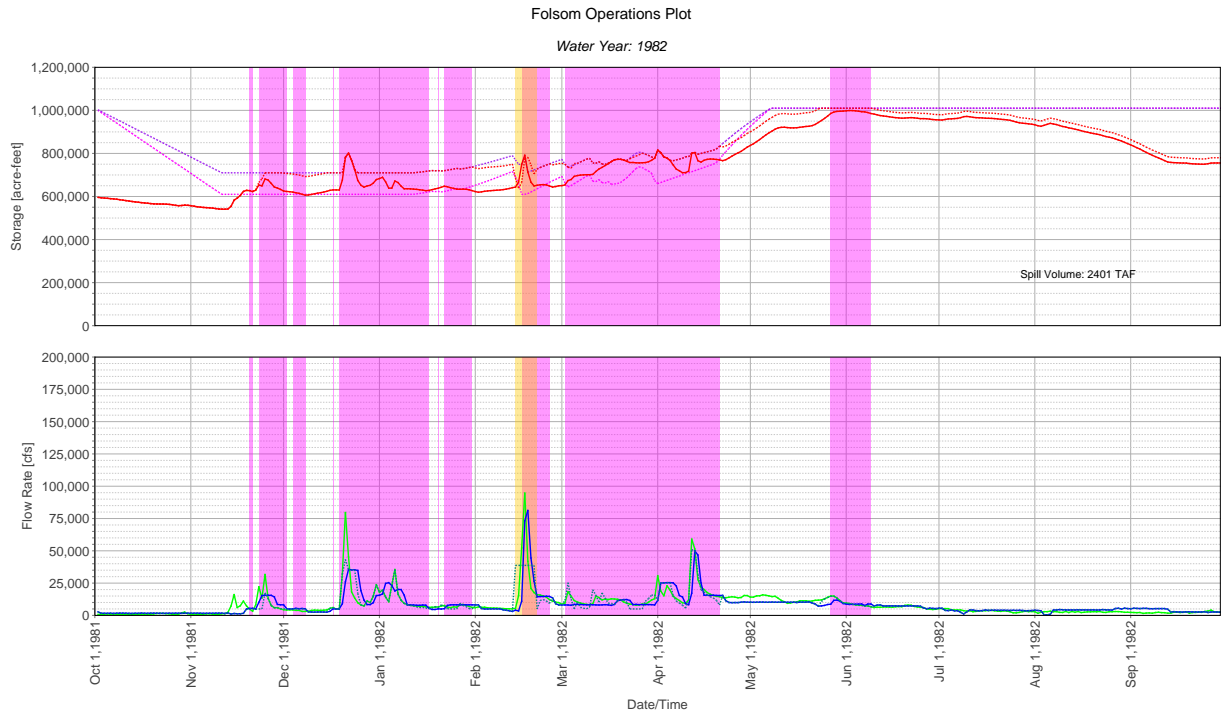
A: Reoperation Plots



A: Reoperation Plots



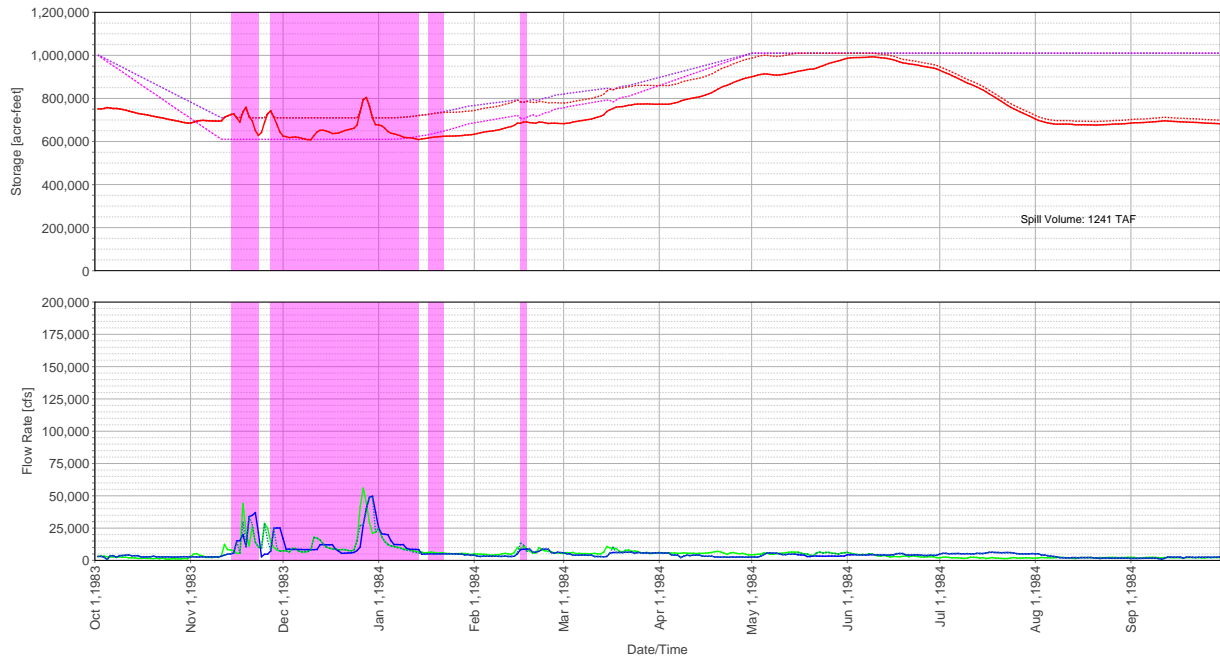
A: Reoperation Plots



A: Reoperation Plots

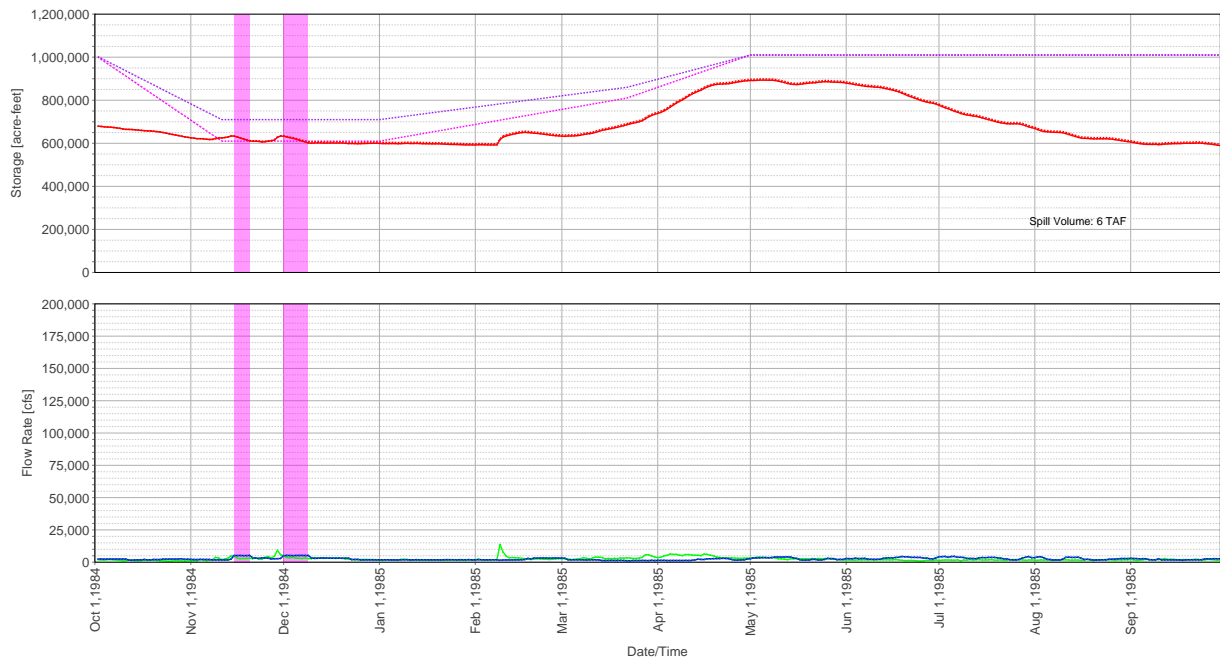
Folsom Operations Plot

Water Year: 1984



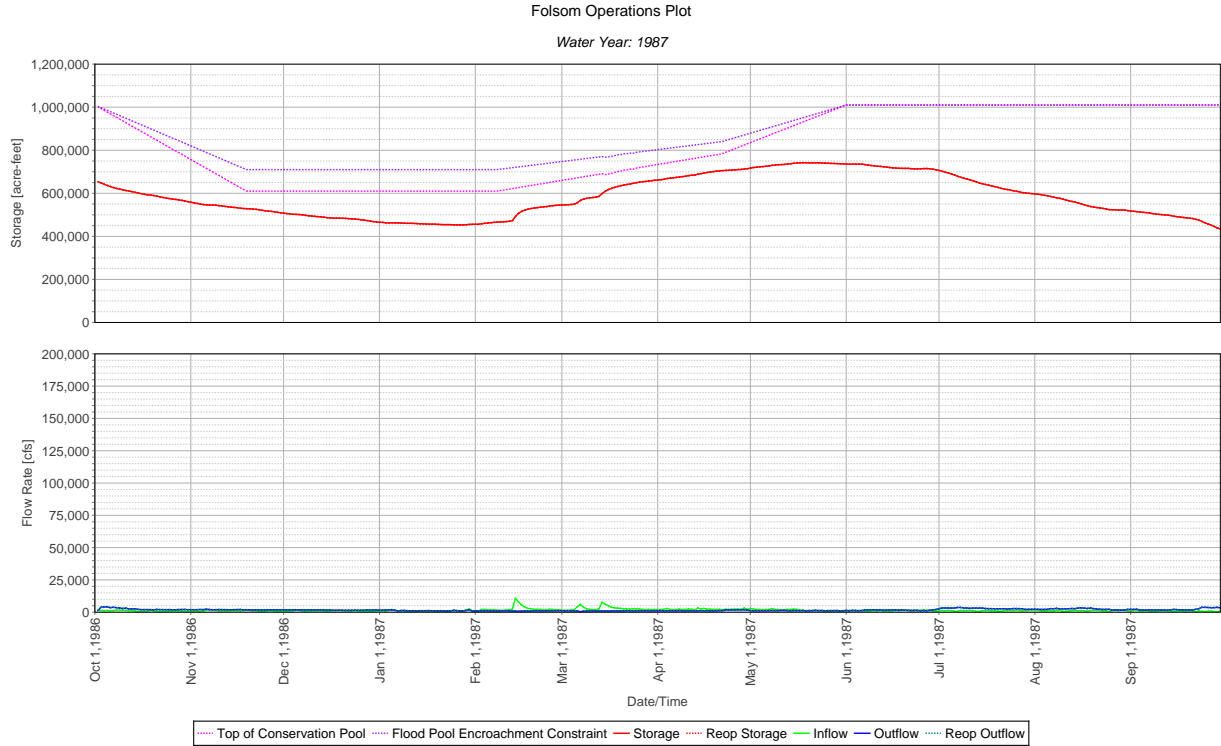
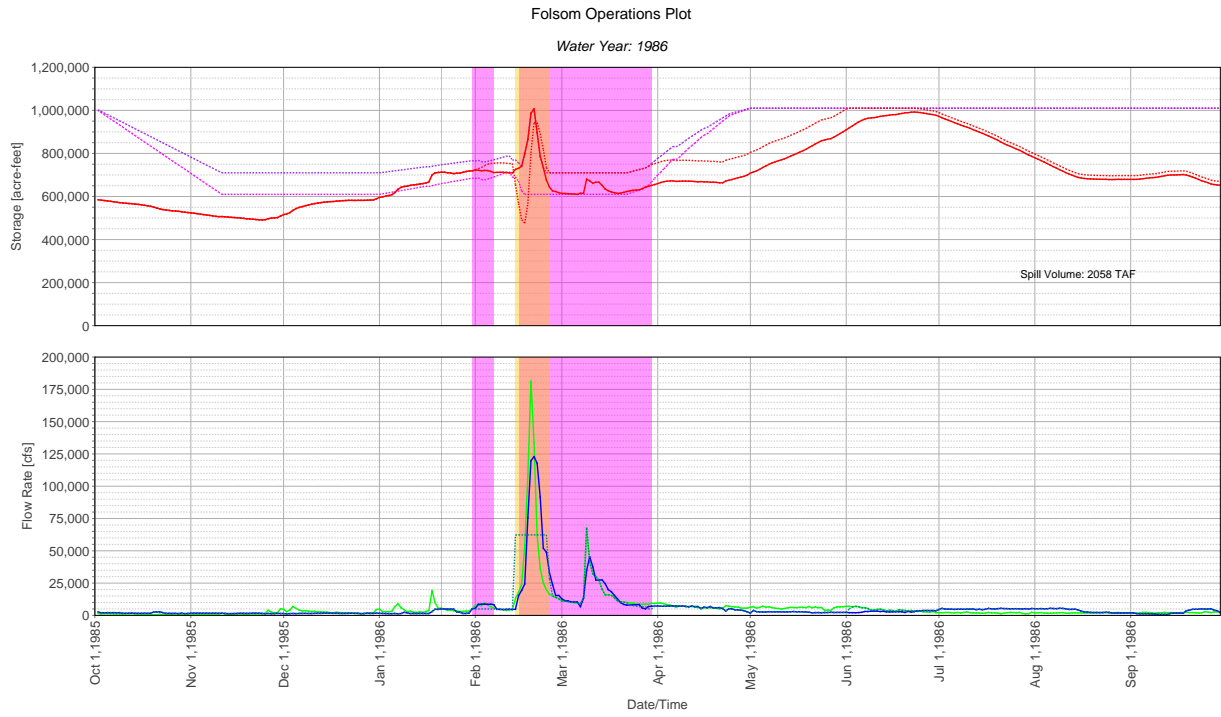
Folsom Operations Plot

Water Year: 1985



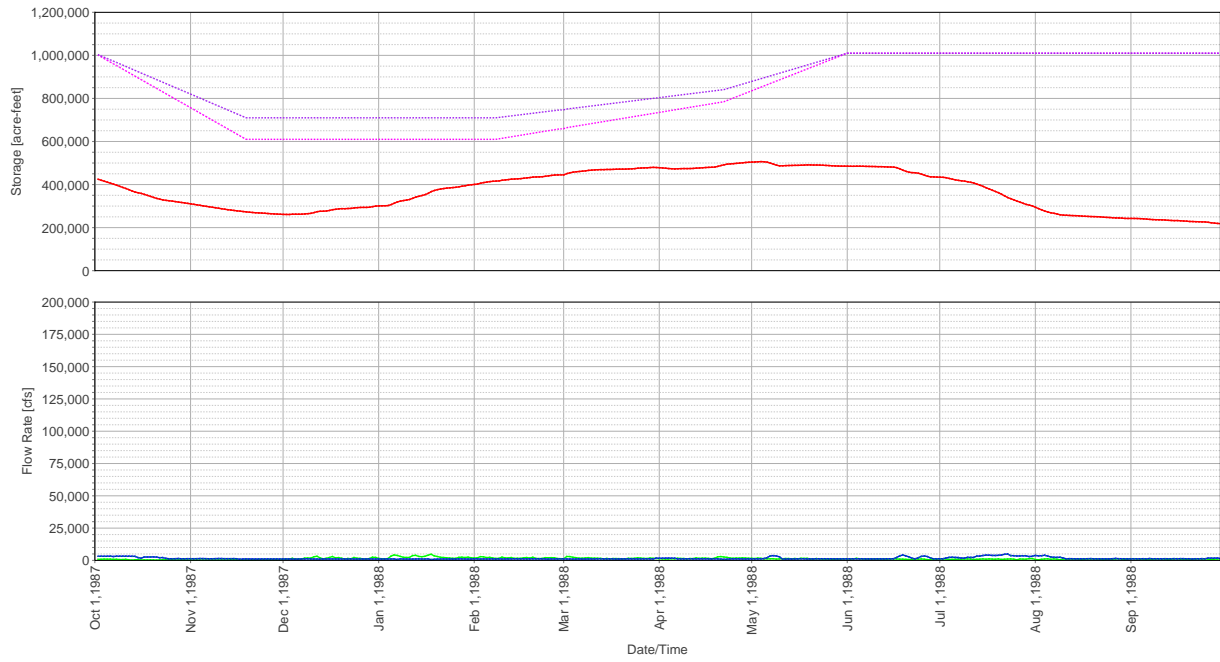
..... Top of Conservation Pool
 Flood Pool Encroachment Constraint
 —— Storage
 Reop Storage
 —— Inflow
 —— Outflow
 Reop Outflow

A: Reoperation Plots

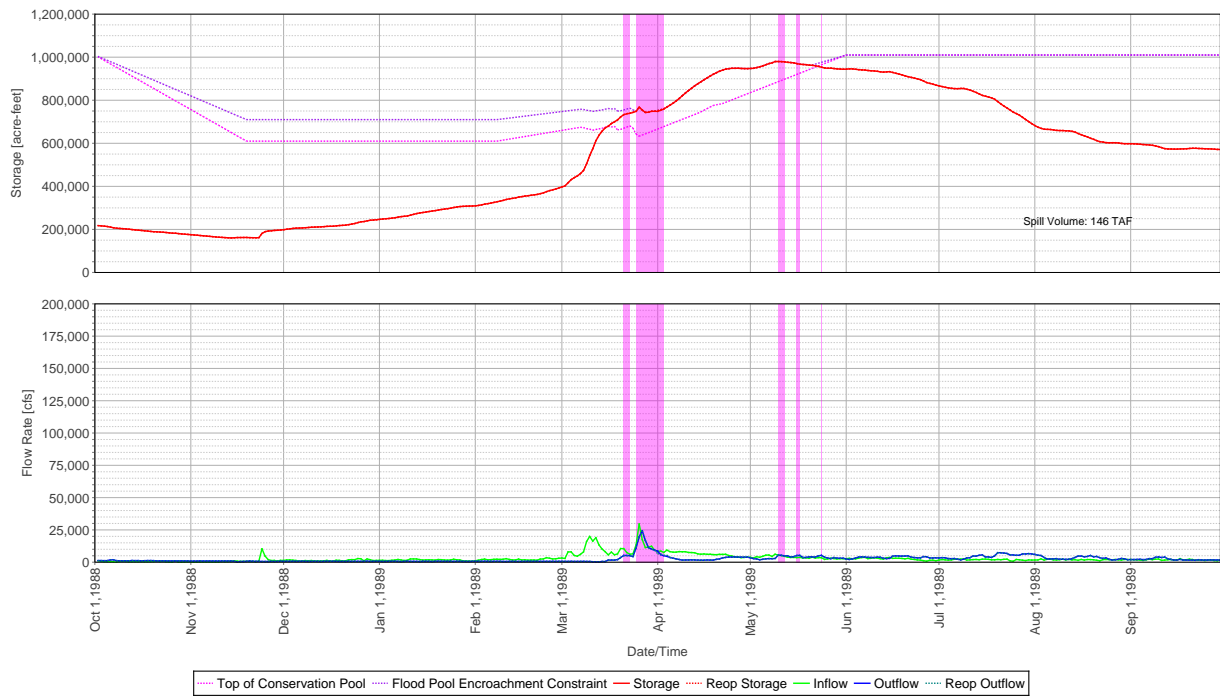


A: Reoperation Plots

Folsom Operations Plot
Water Year: 1988



Folsom Operations Plot
Water Year: 1989

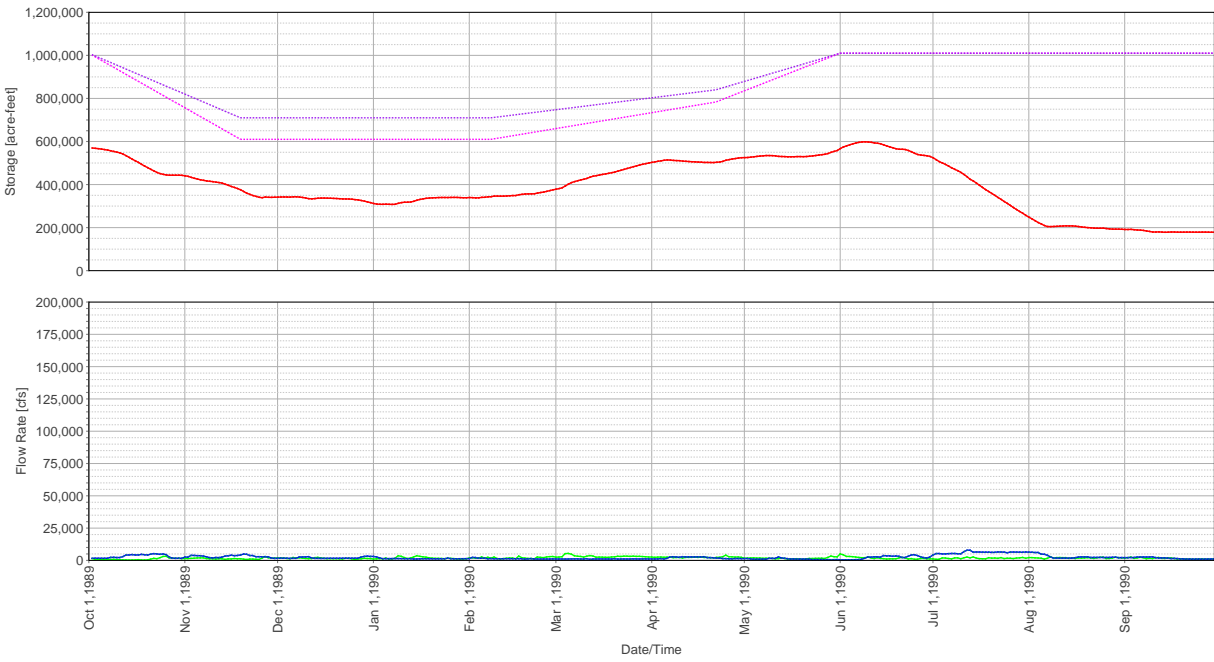


..... Top of Conservation Pool
 - - - - Flood Pool Encroachment Constraint
 —— Storage
 Reop Storage
 —— Inflow
 —— Outflow
 Reop Outflow

A: Reoperation Plots

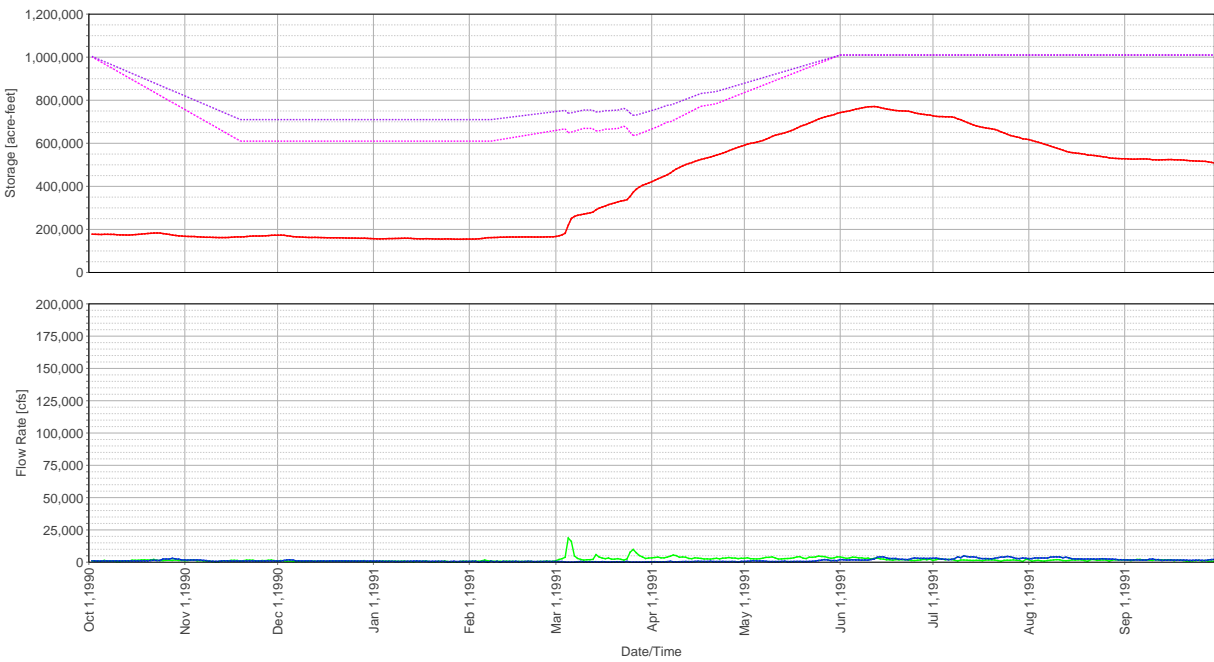
Folsom Operations Plot

Water Year: 1990



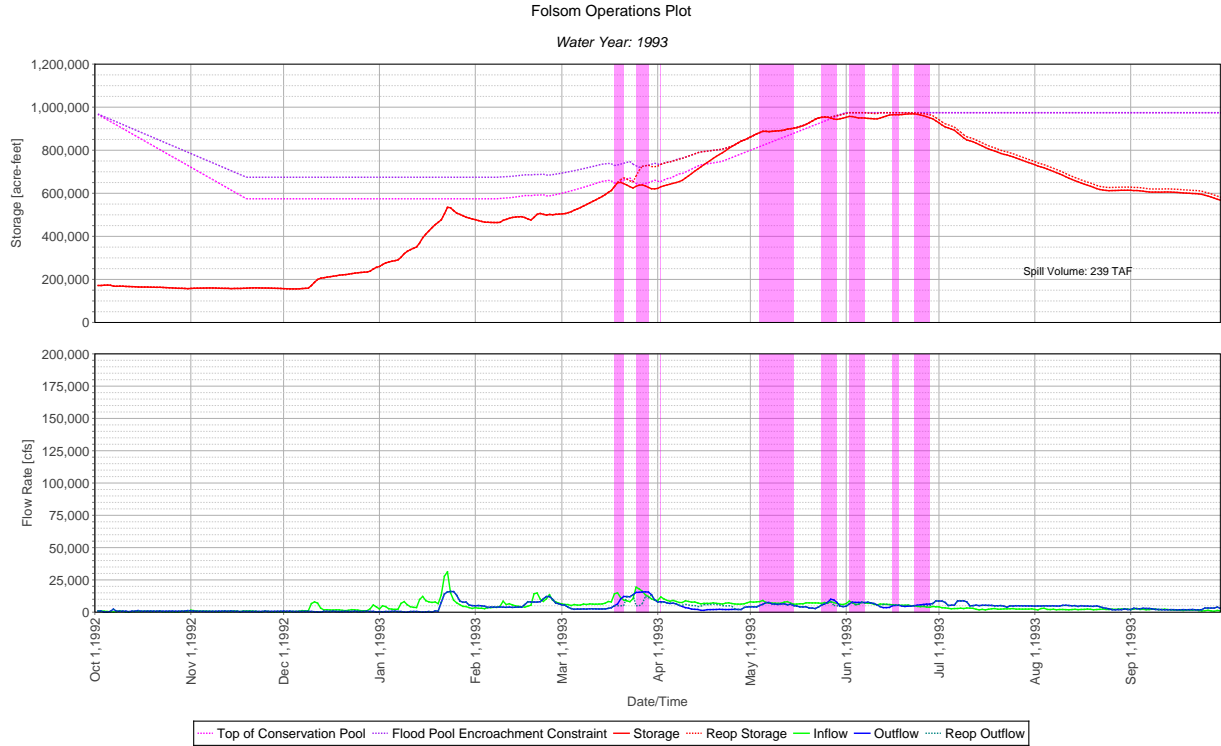
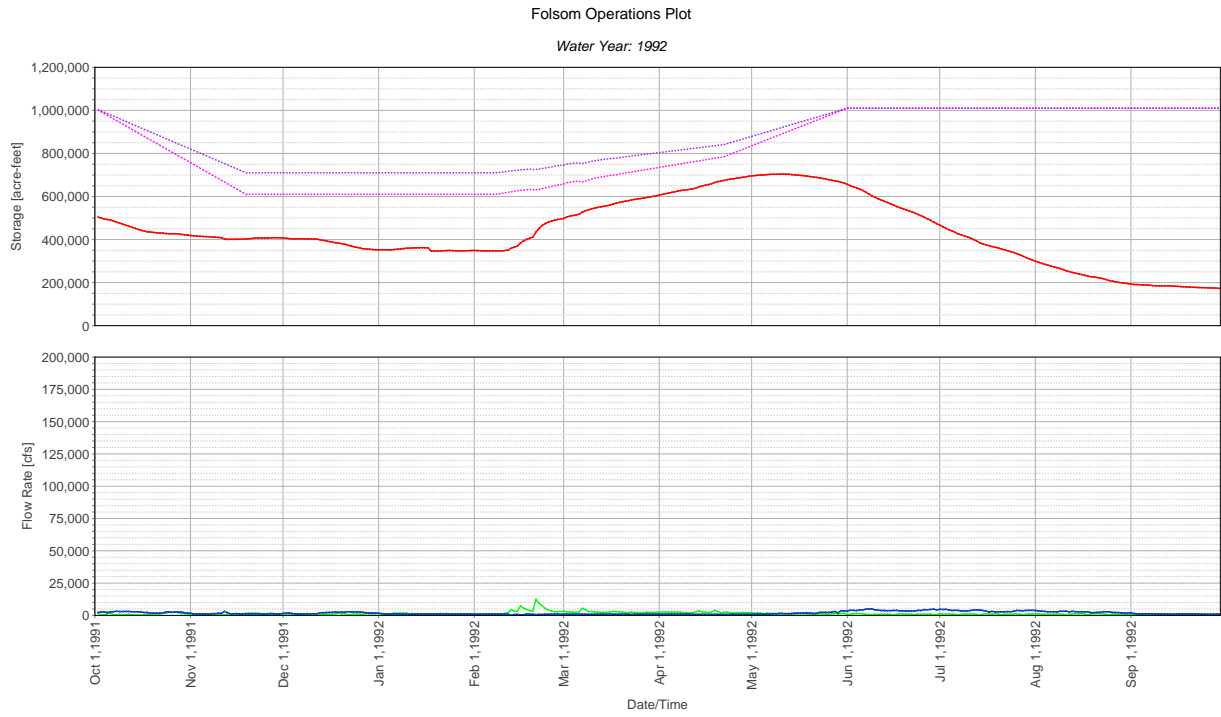
Folsom Operations Plot

Water Year: 1991



..... Top of Conservation Pool
 - - - - Flood Pool Encroachment Constraint
 ——— Storage
 Reop Storage
 ——— Inflow
 ——— Outflow
 Reop Outflow

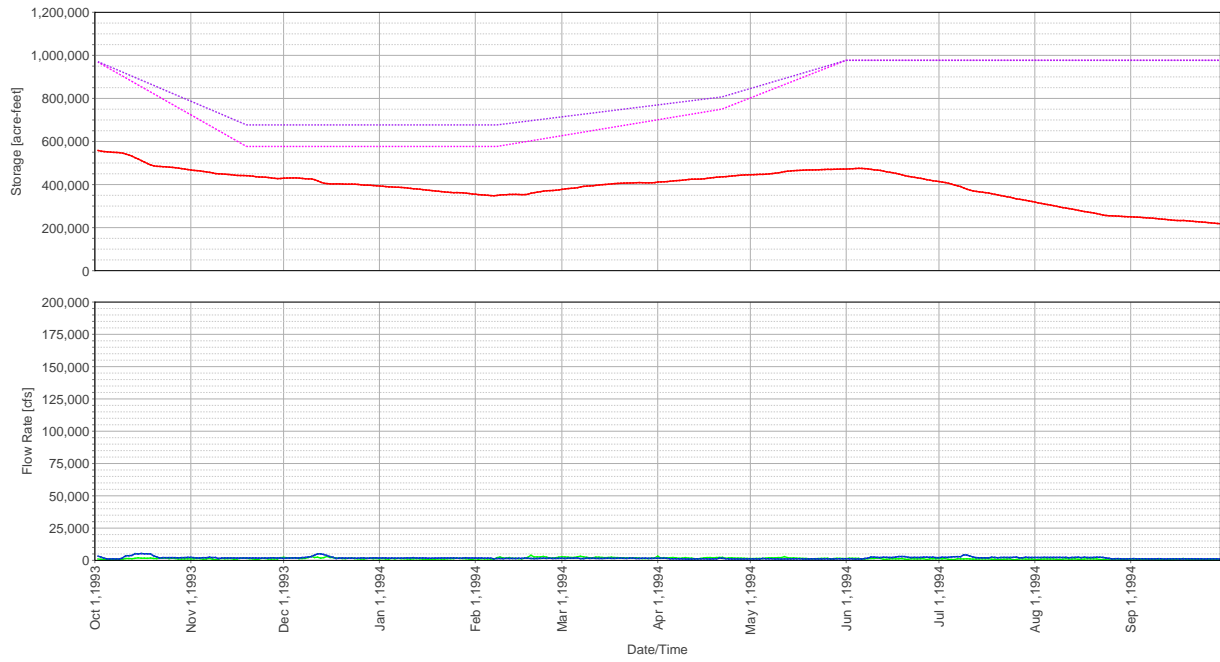
A: Reoperation Plots



A: Reoperation Plots

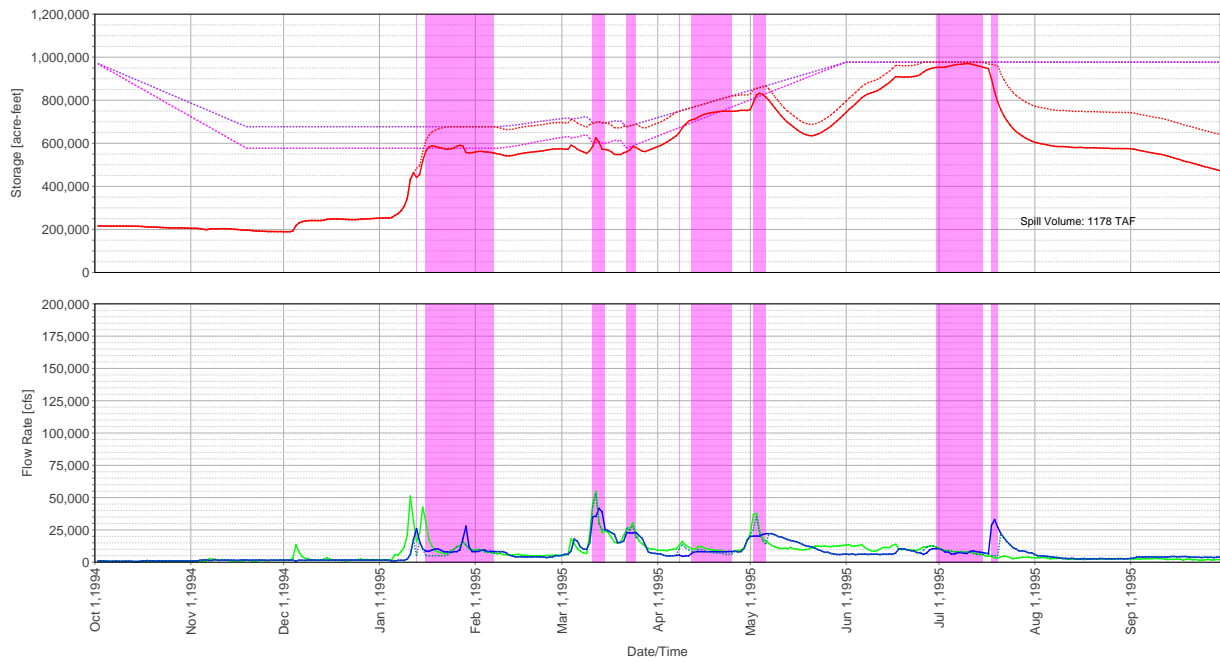
Folsom Operations Plot

Water Year: 1994



Folsom Operations Plot

Water Year: 1995

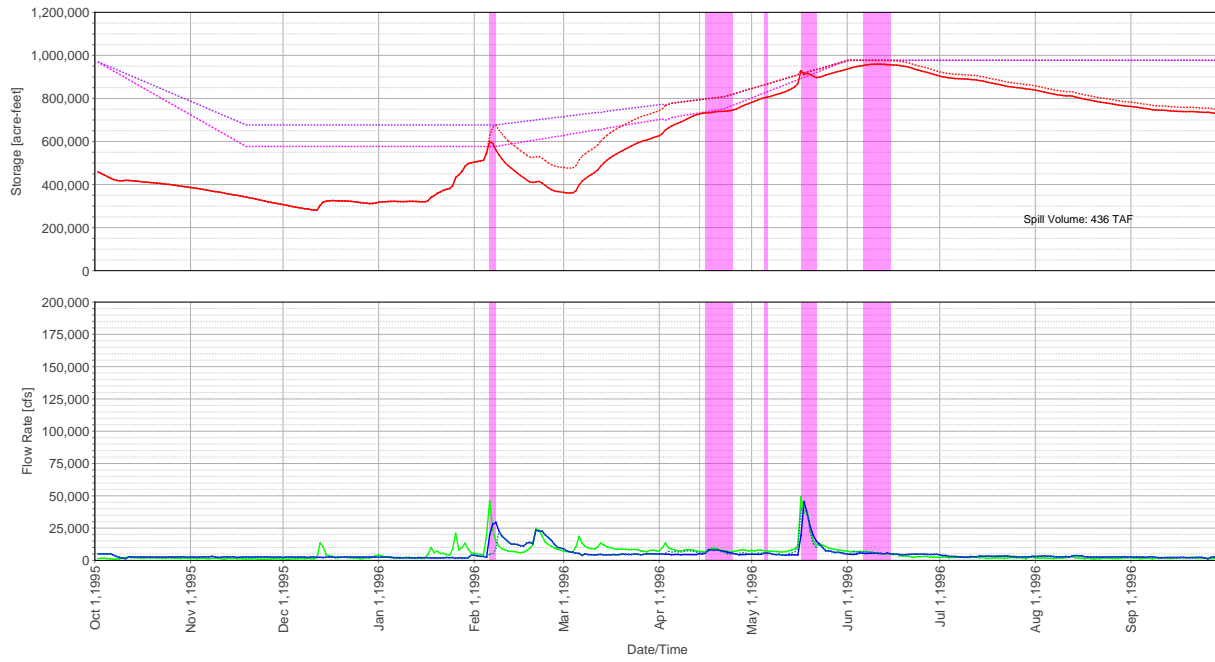


..... Top of Conservation Pool
 Flood Pool Encroachment Constraint
 ——— Storage
 Reop Storage
 ——— Inflow
 ——— Outflow
 Reop Outflow

A: Reoperation Plots

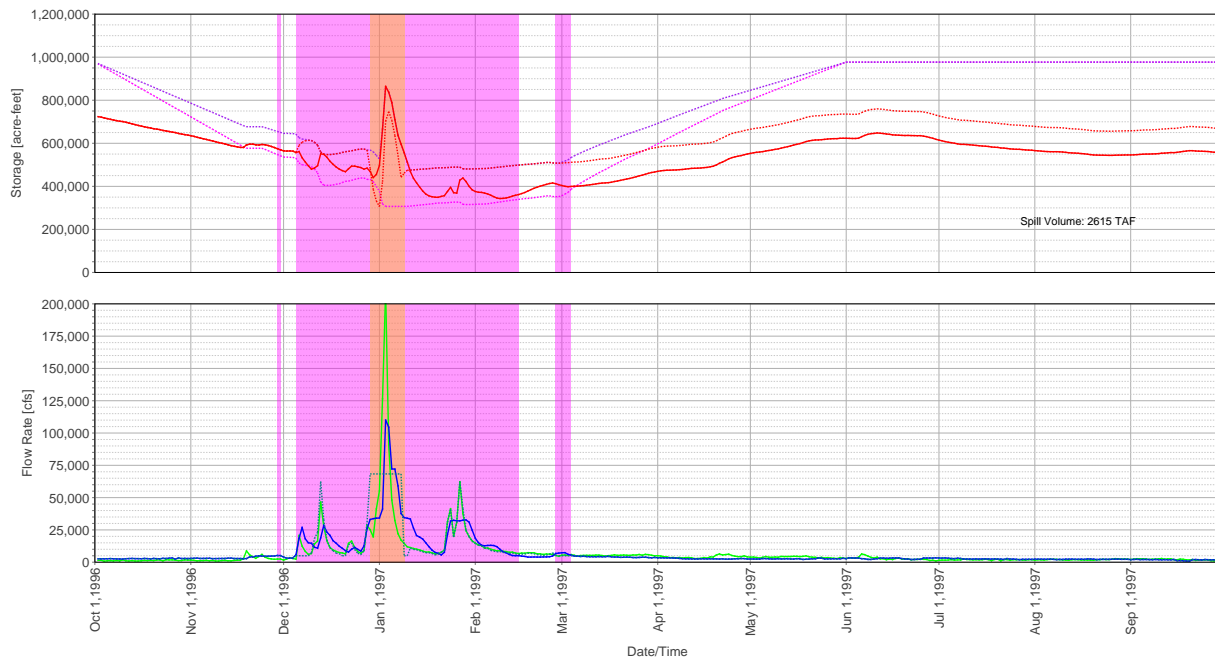
Folsom Operations Plot

Water Year: 1996



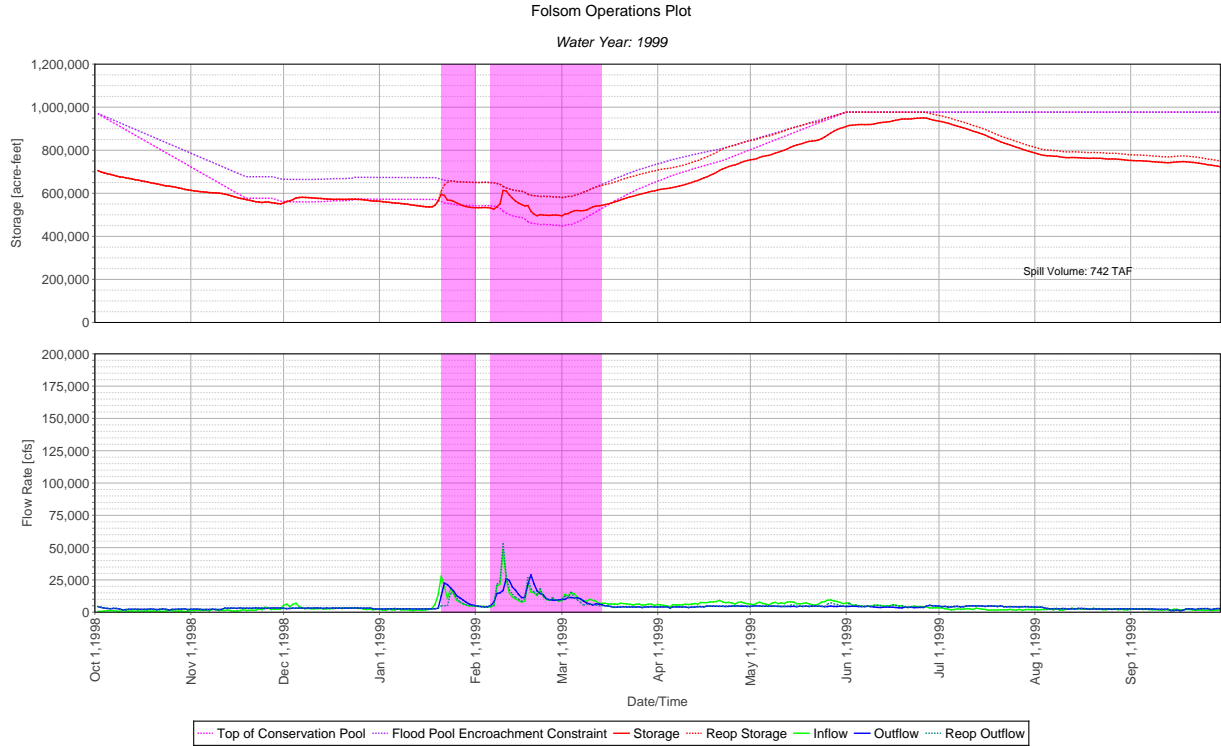
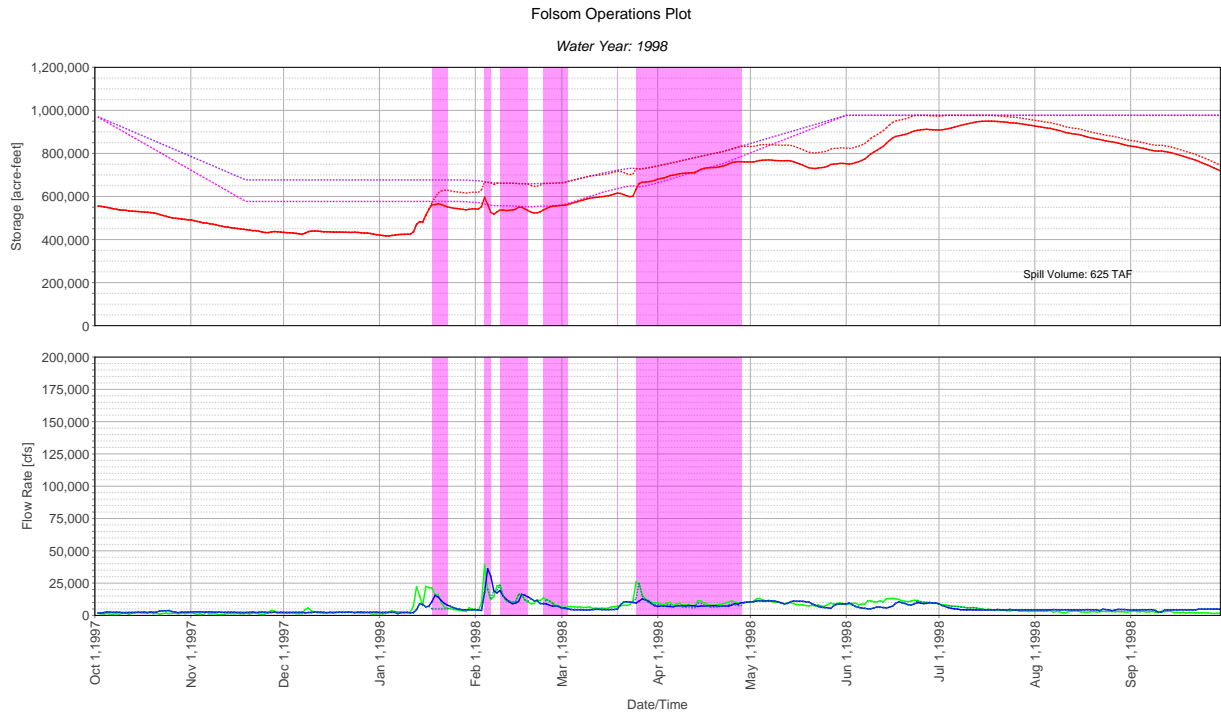
Folsom Operations Plot

Water Year: 1997

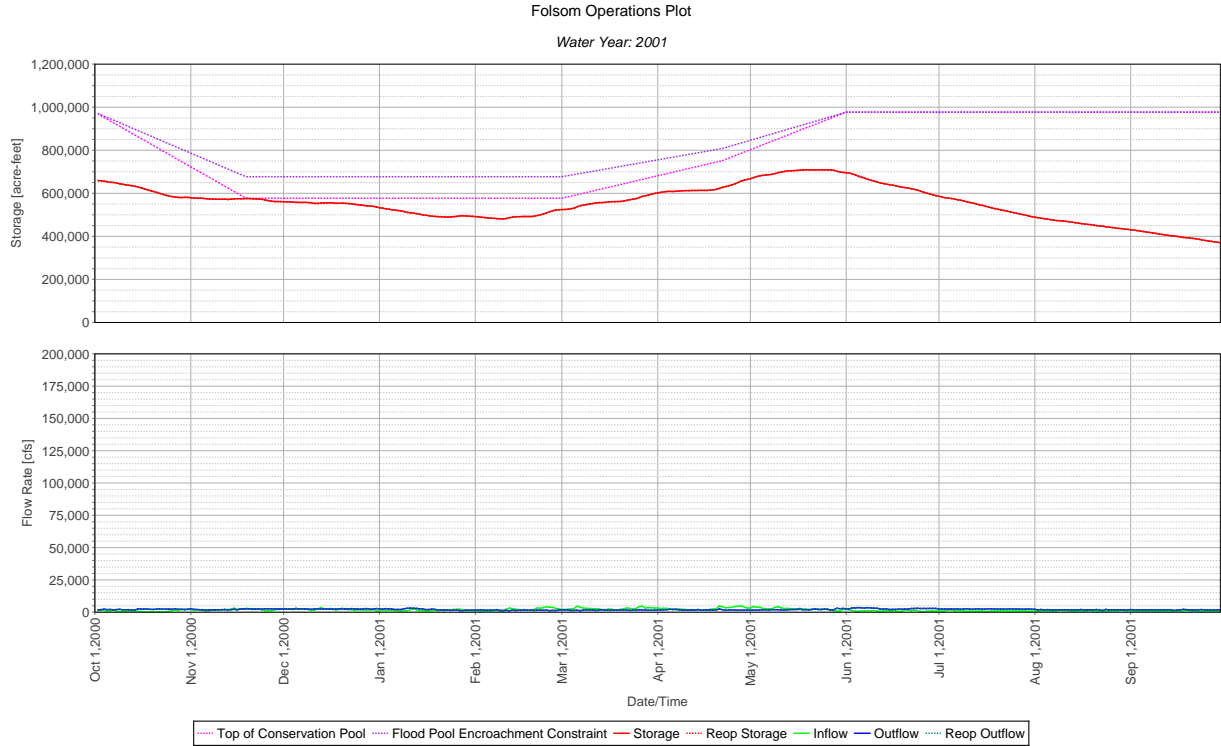
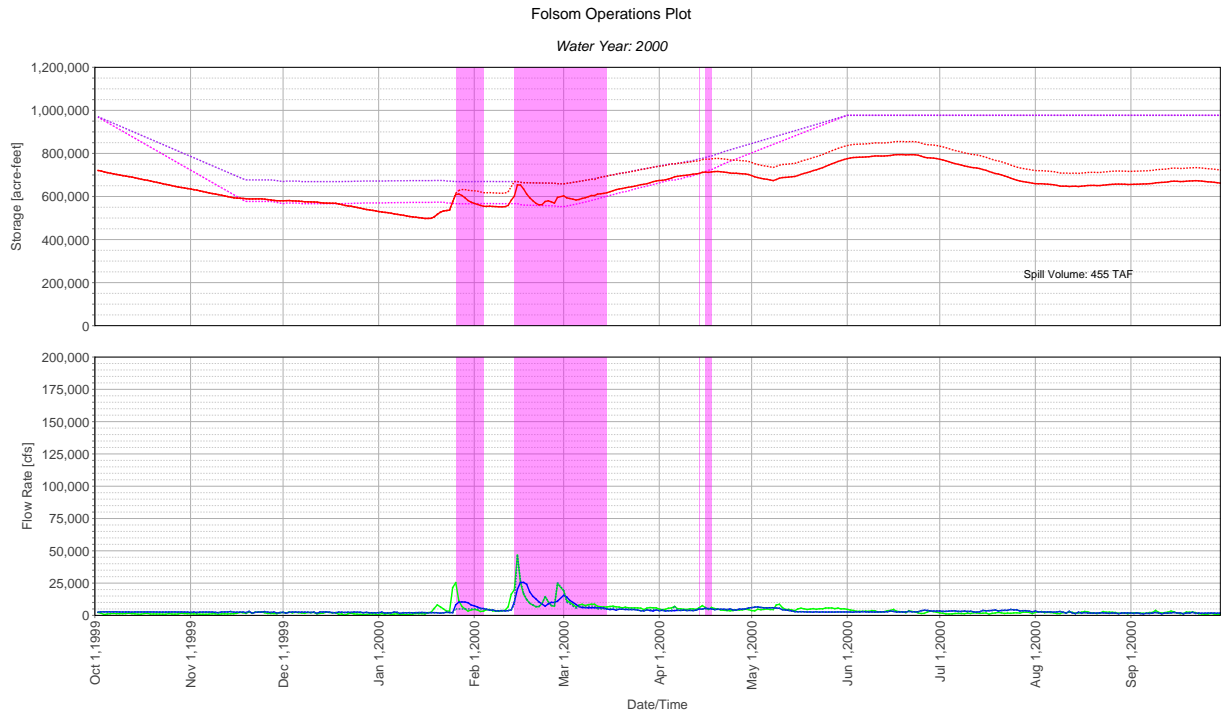


..... Top of Conservation Pool
 Flood Pool Encroachment Constraint
 ——— Storage
 Reop Storage
 ——— Inflow
 ——— Outflow
 Reop Outflow

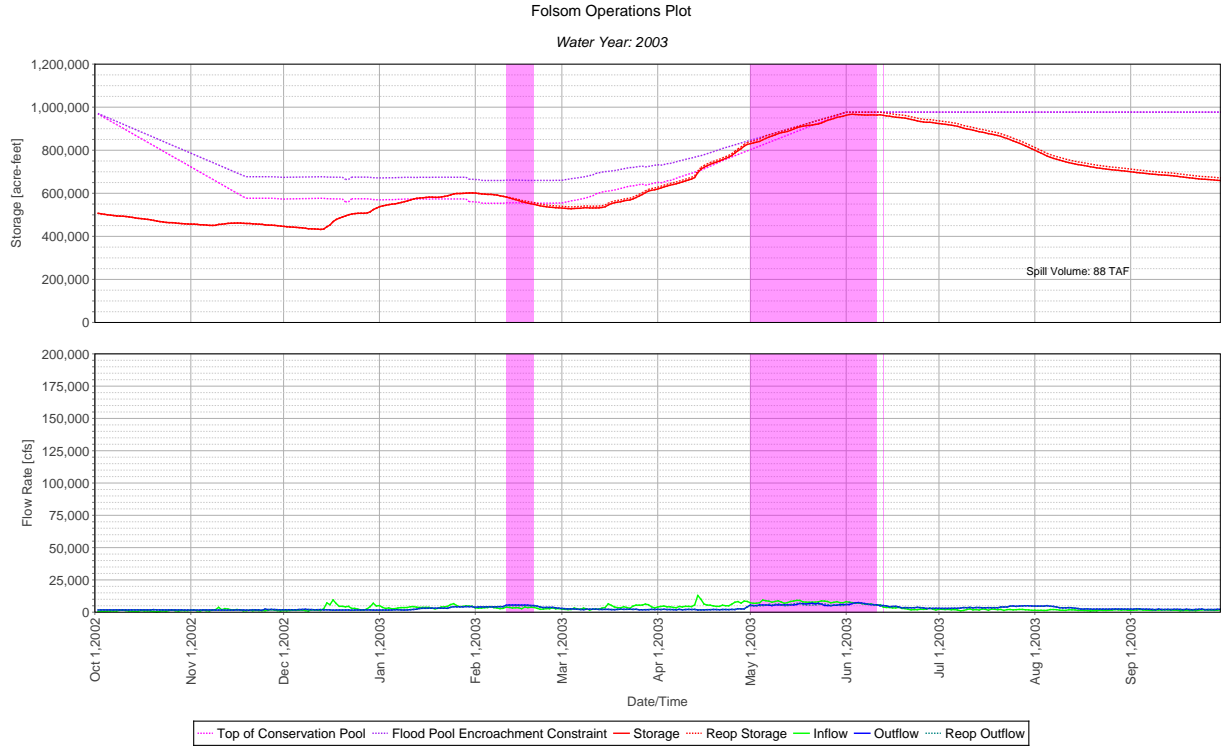
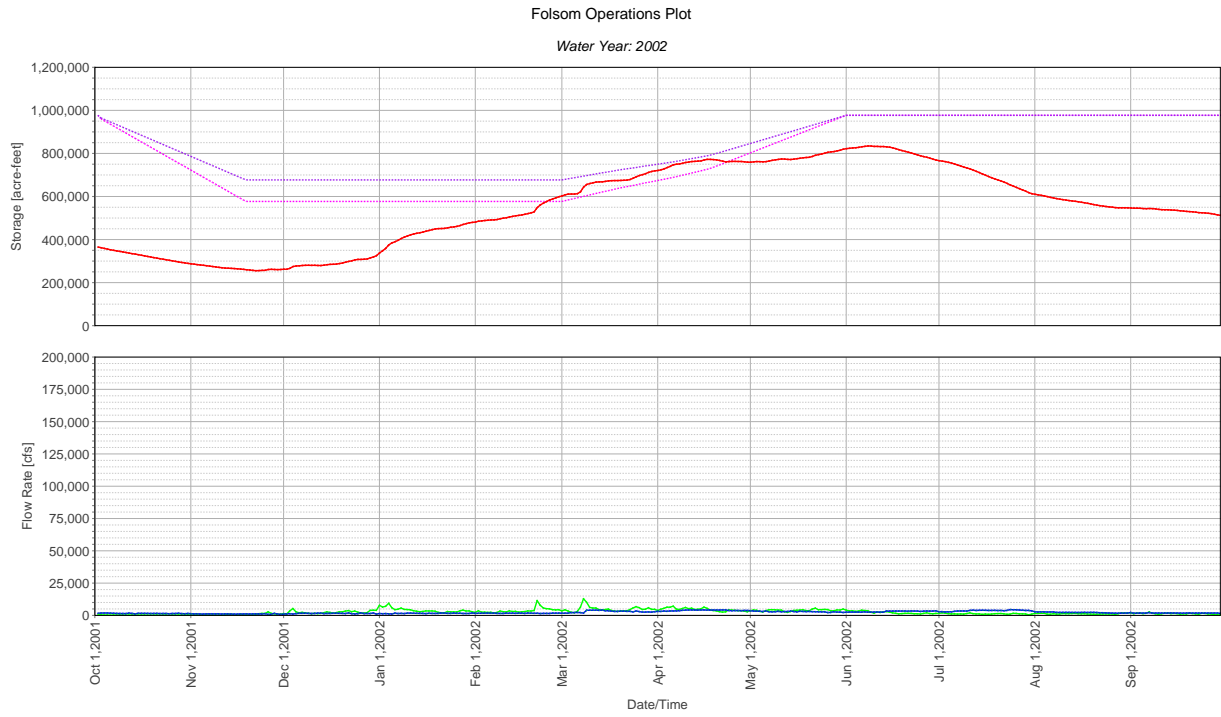
A: Reoperation Plots



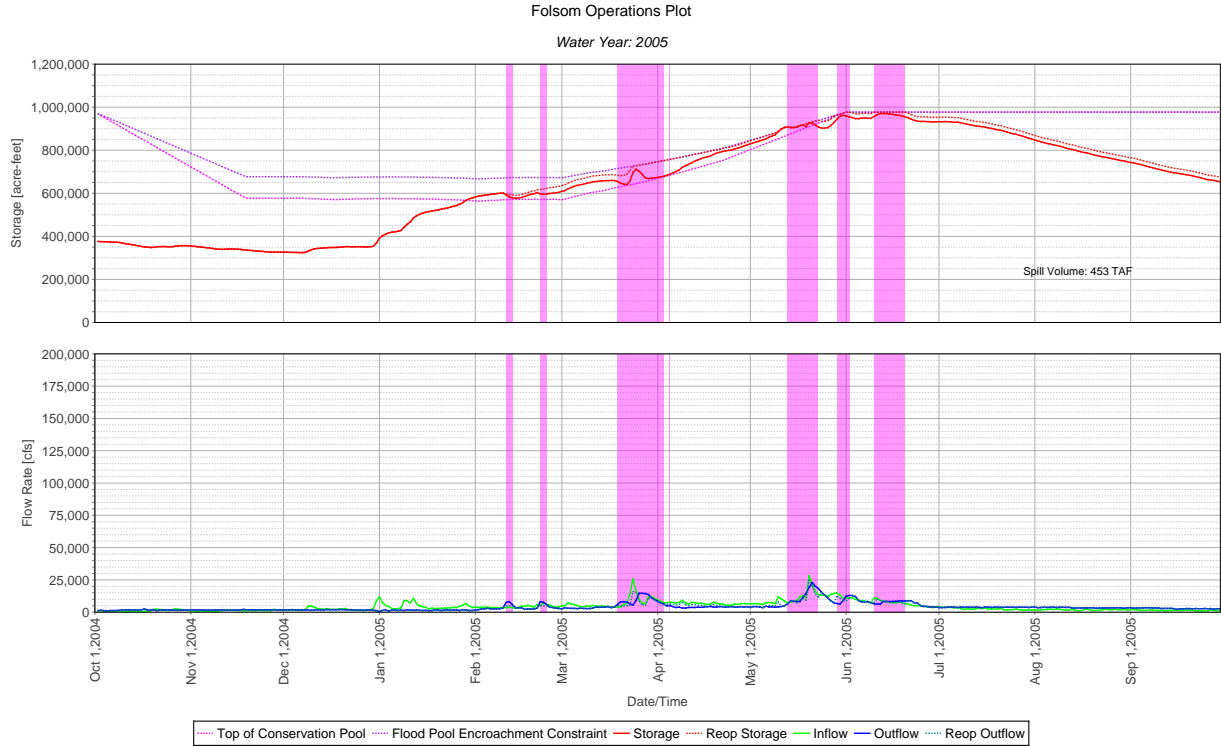
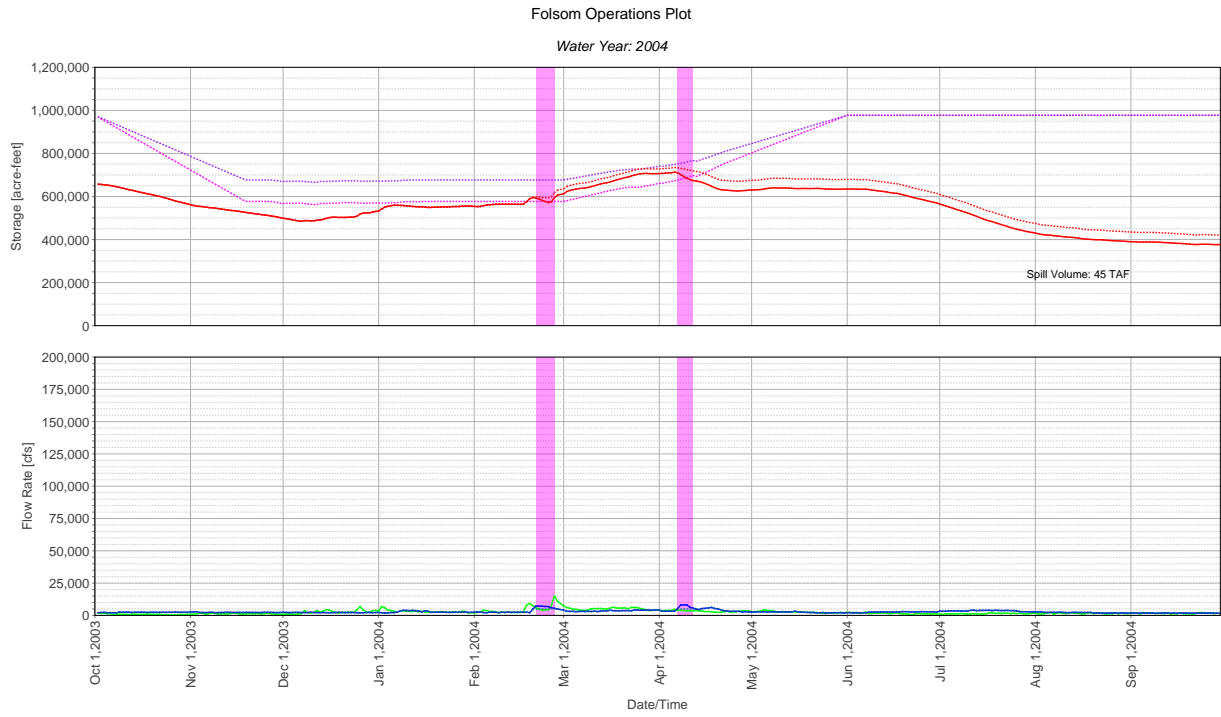
A: Reoperation Plots



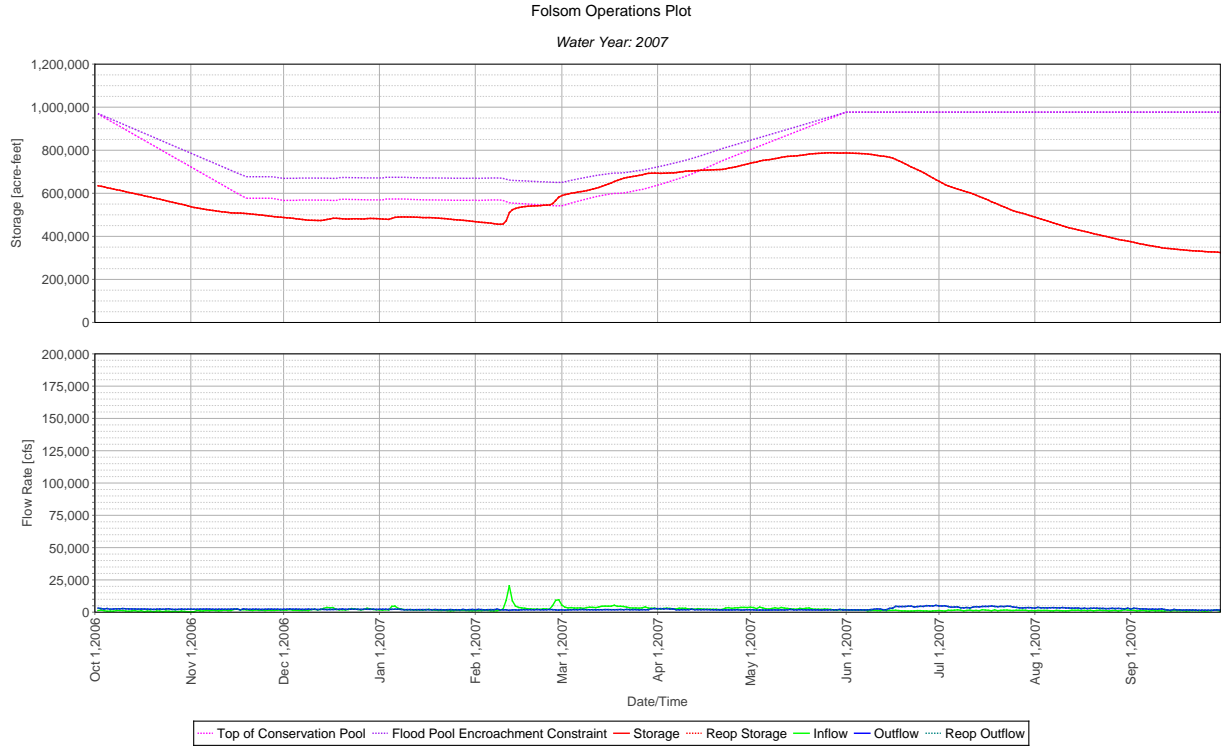
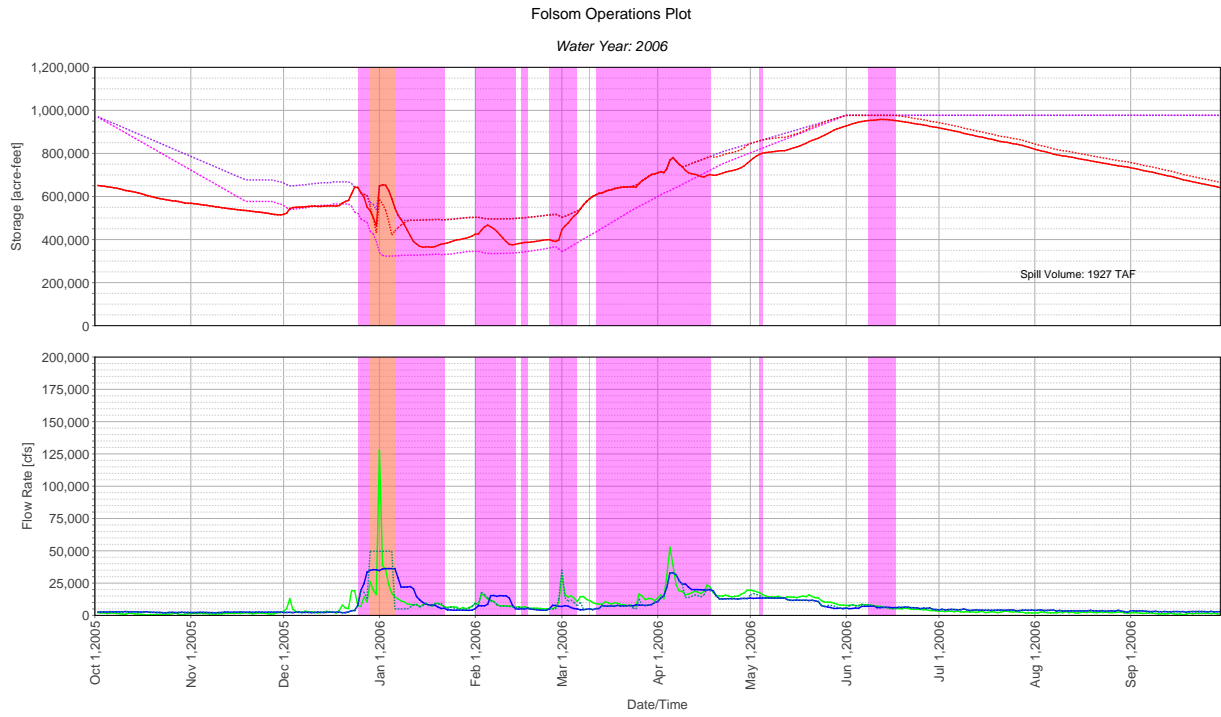
A: Reoperation Plots



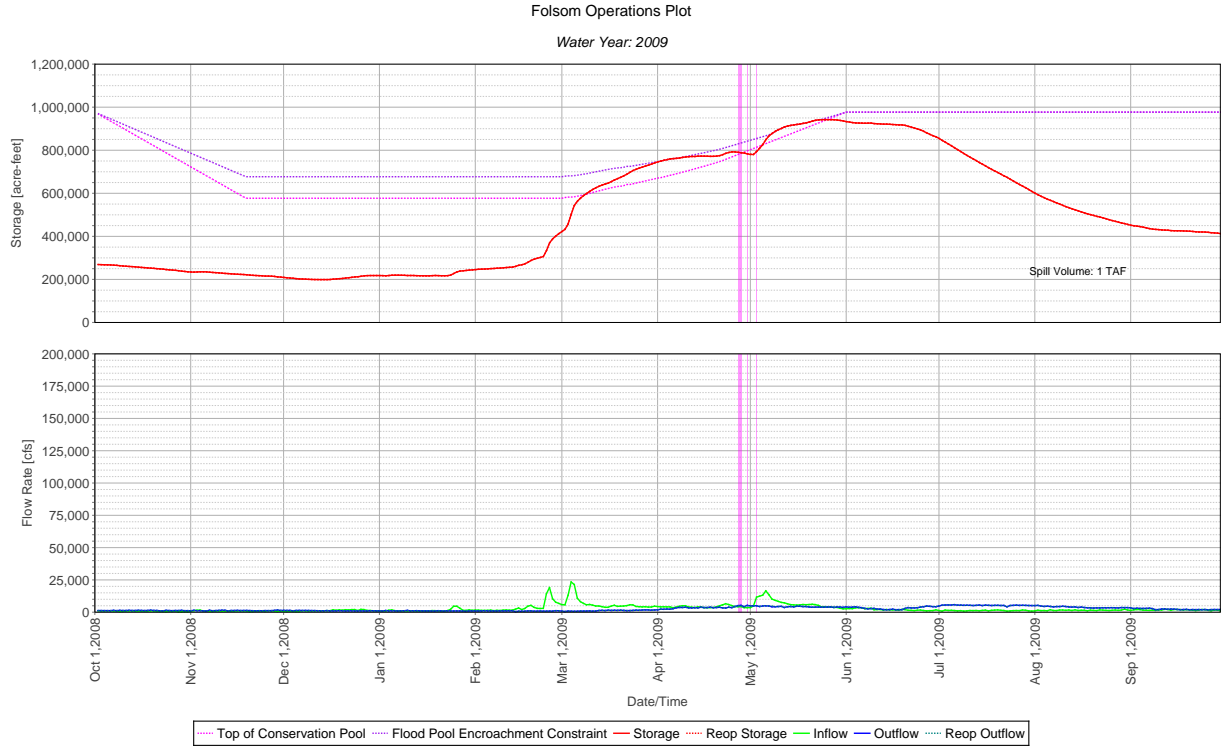
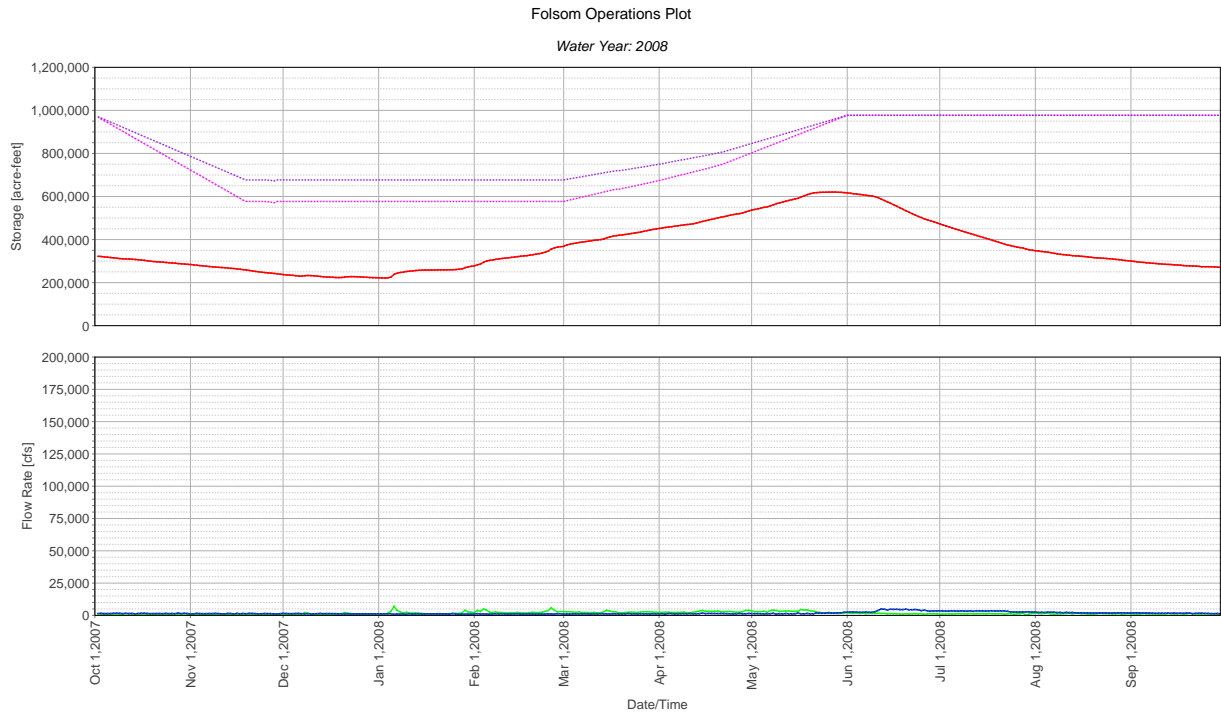
A: Reoperation Plots



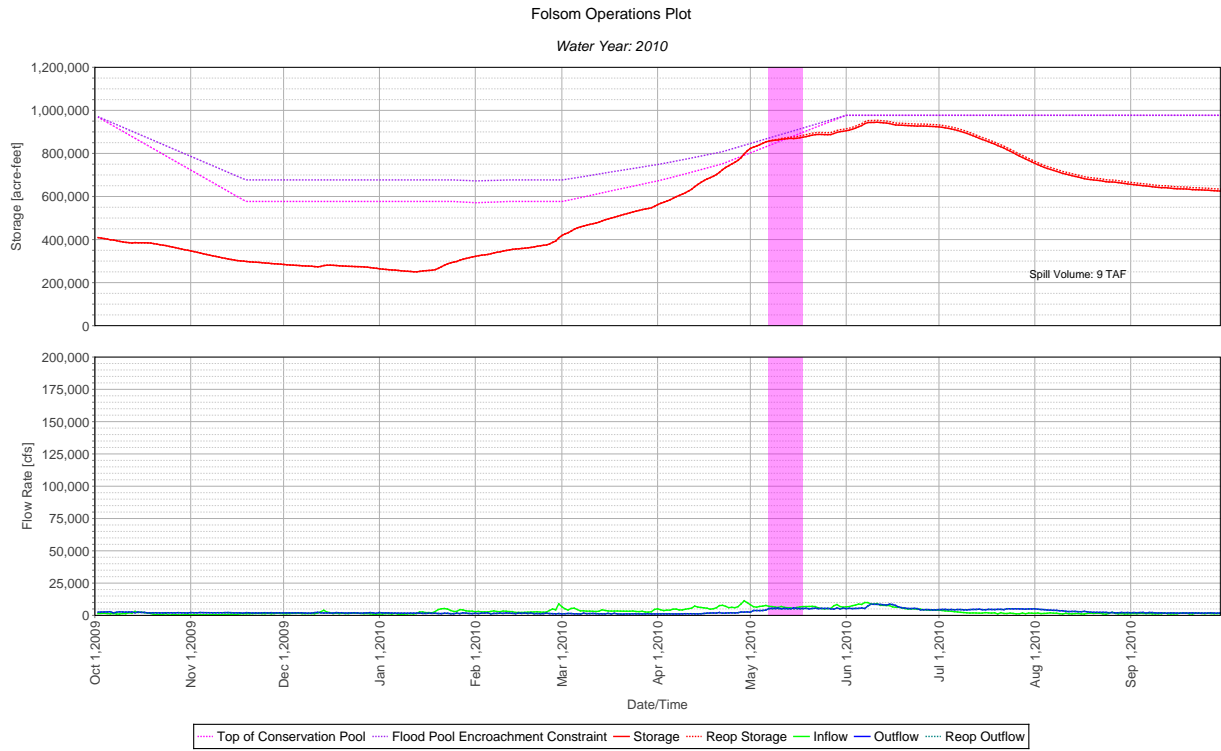
A: Reoperation Plots



A: Reoperation Plots



A: Reoperation Plots



B

Monthly Effects Evaluation

This appendix details how CalLite was used to perform an approximation of the daily reoperation to enhance water supply and the resulting effects of that reoperation systemwide.

B.1 Modeling the Reoperation

This appendix details the monthly flow data thresholds used in the CalLite reoperation routine that will cause that reoperation to cease (termed deactivation) in CalLite. The deactivation of reoperation in CalLite is compared with occurrences of deactivation from the daily reoperation performed on historical data to ensure the results are consistent between the daily analysis and CalLite modeling.

B.1.1 Parameters

Shasta

The monthly flow threshold for deactivation the reoperation at Shasta Dam was determined to be 30,000 cfs. A 5-day inflow forecast exceeding 650 TAF was used as the threshold for the daily investigation.

Oroville

The monthly flow threshold for deactivation the reoperation at Oroville Dam was determined to be 32,500 cfs. A 5-day inflow exceeding 700 TAF was used as the threshold for the daily investigation.

New Bullards Bar

New Bullards Bar is not dynamically modeled in CalLite. Thus, the effects of its reoperation were not assessed at this time.

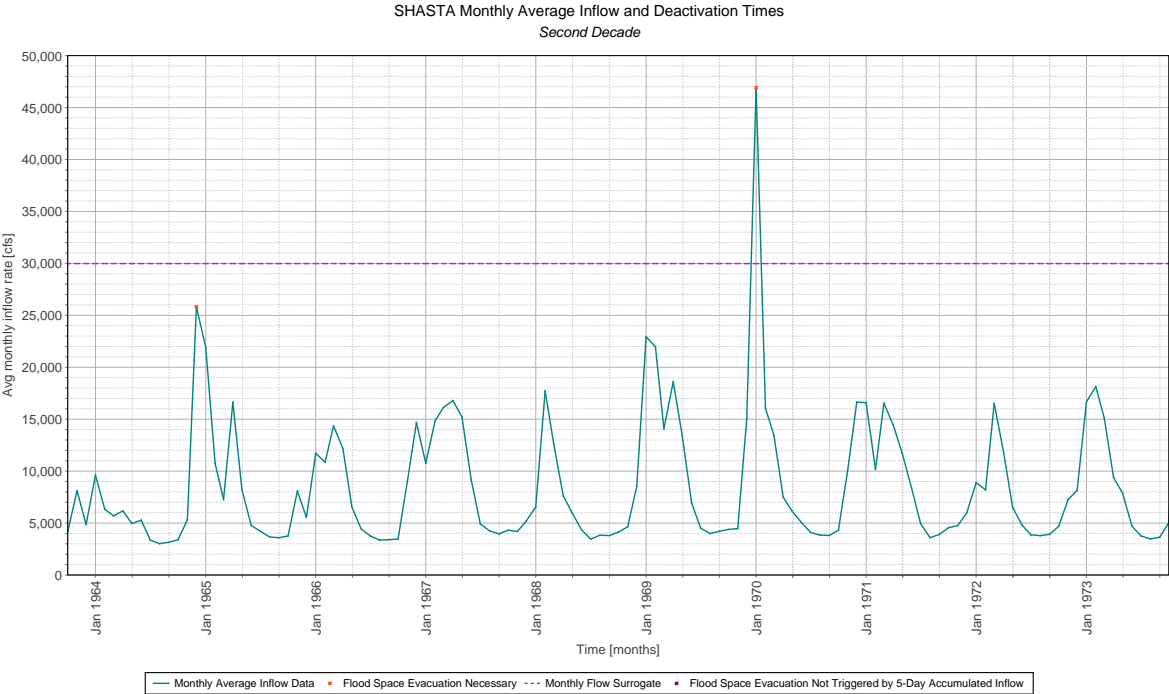
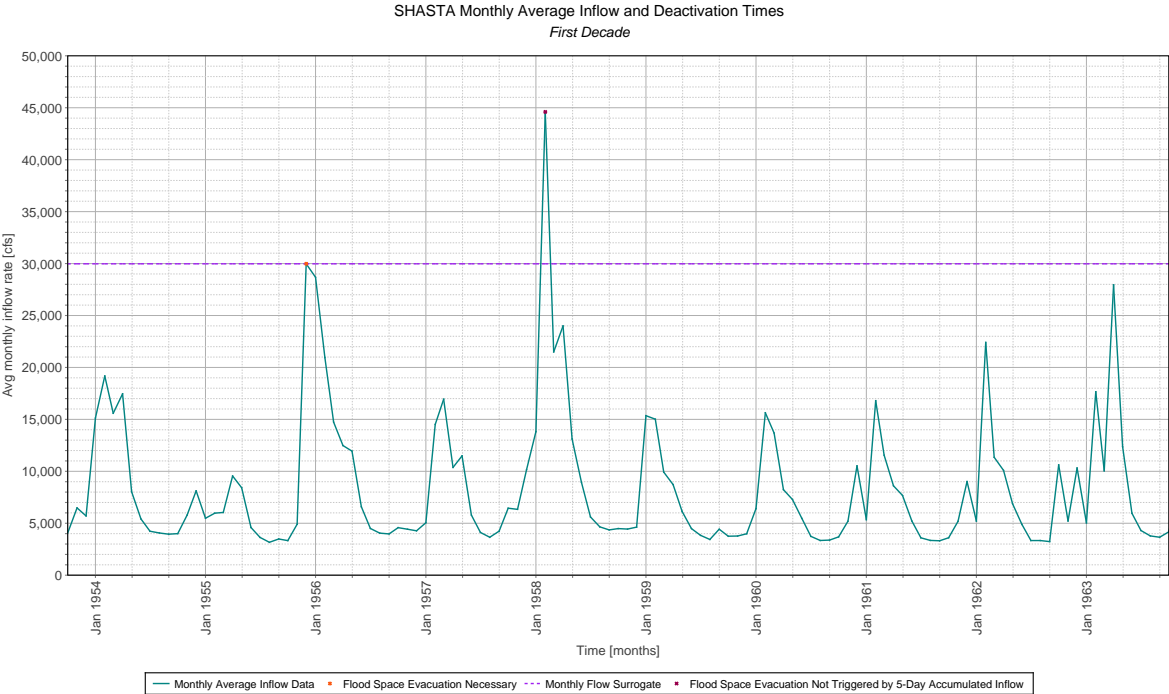
Folsom

The monthly flow threshold for deactivation the reoperation at Folsom Dam was determined to be 15,200 cfs. A 5-day inflow exceeding 400 TAF was used as the threshold for the daily investigation.

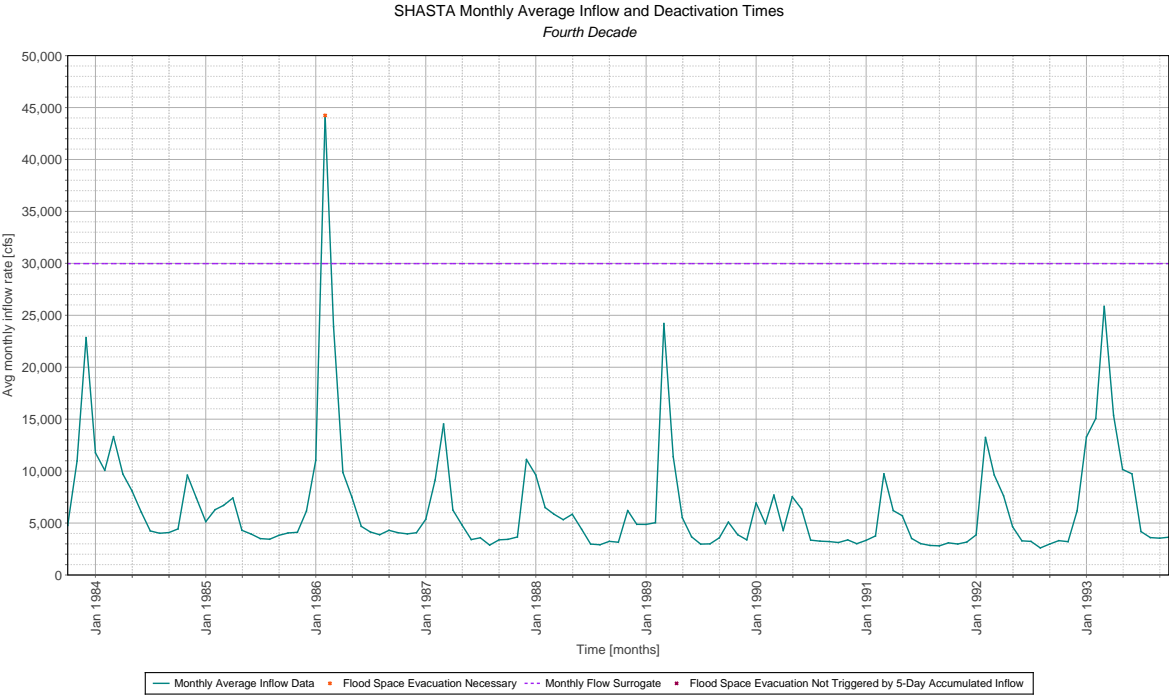
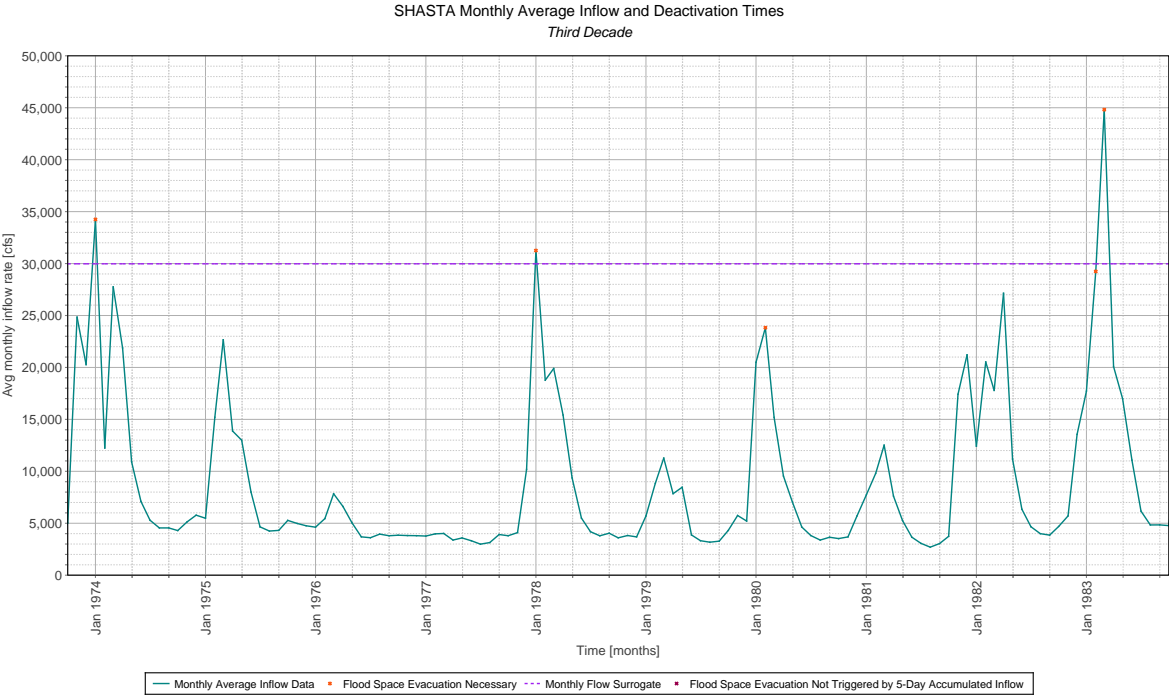
B.1.2 Plots

The following figures demonstrate the monthly flow data and deactivation thresholds for these reservoirs. Also shown are instances where the daily analysis of historical data for each reservoir indicated the cessation of reoperation was necessary due to an exceedingly large inflow forecast.

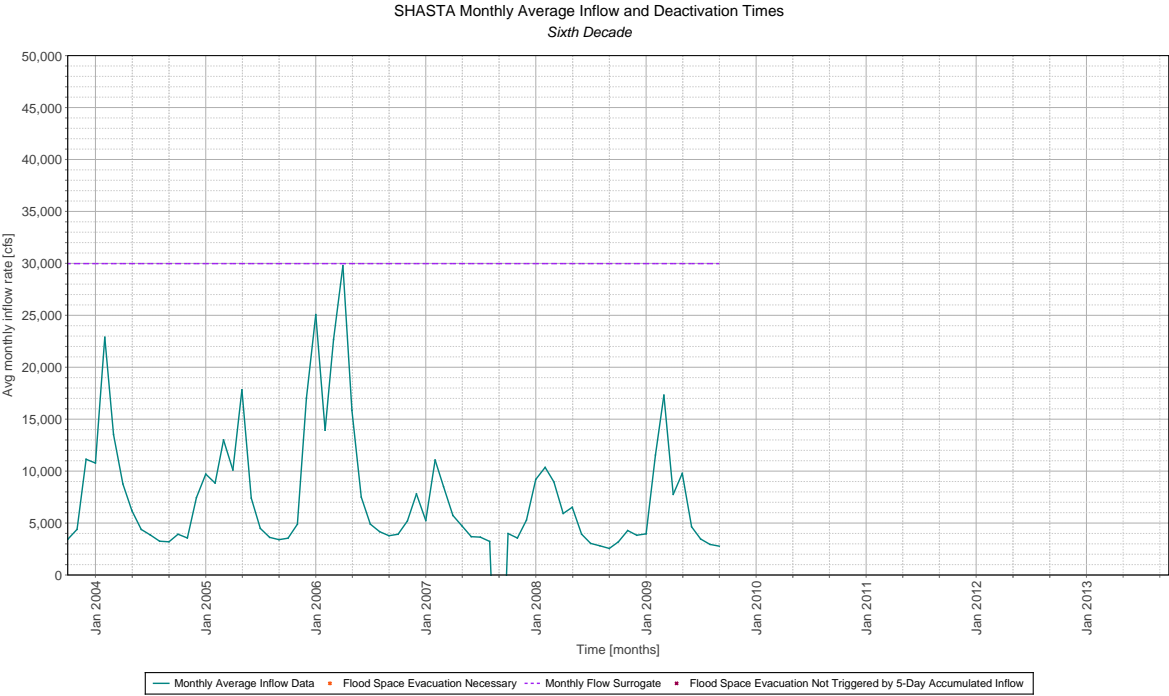
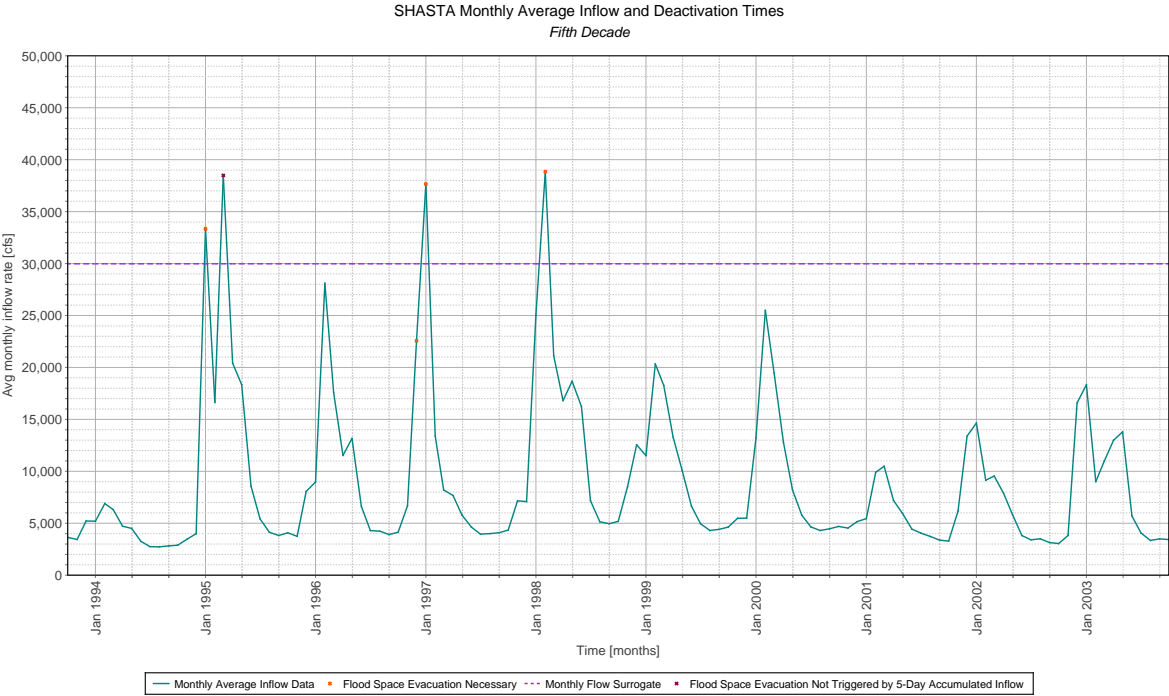
B: Monthly Effects Evaluation



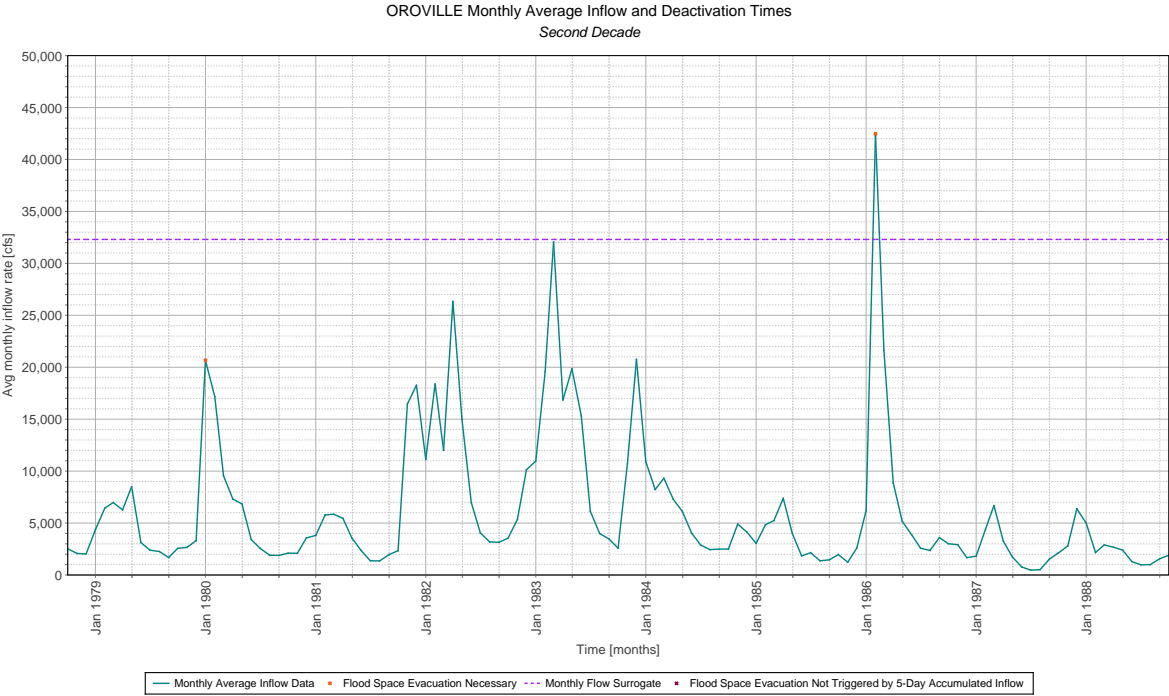
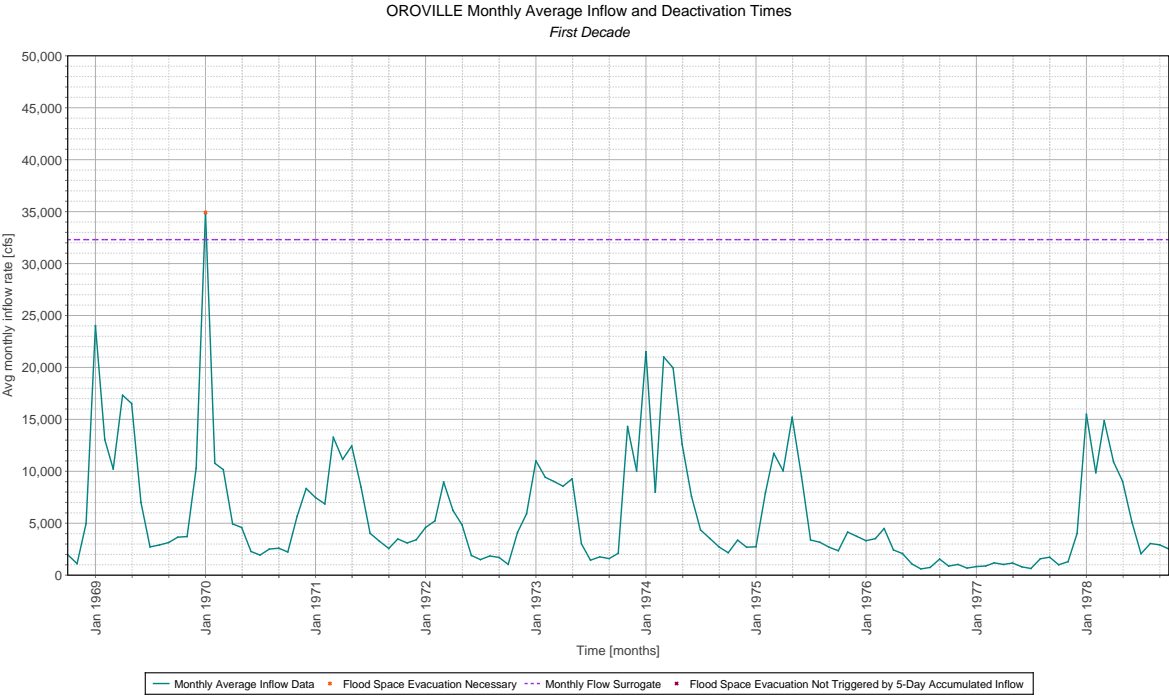
B: Monthly Effects Evaluation



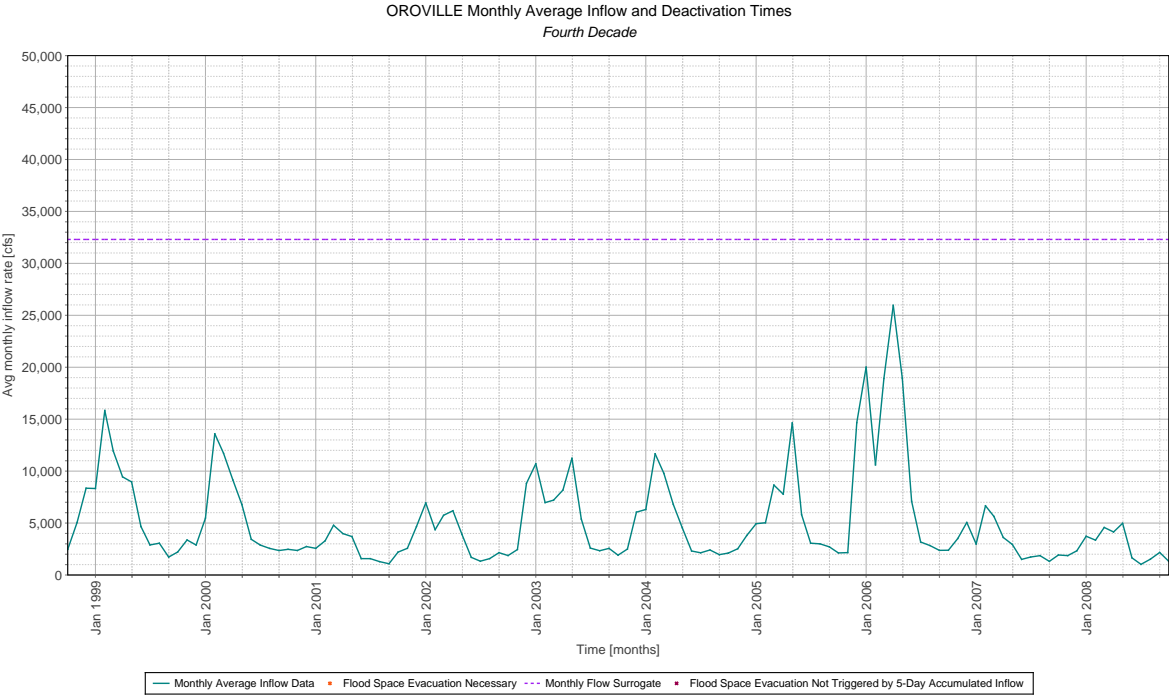
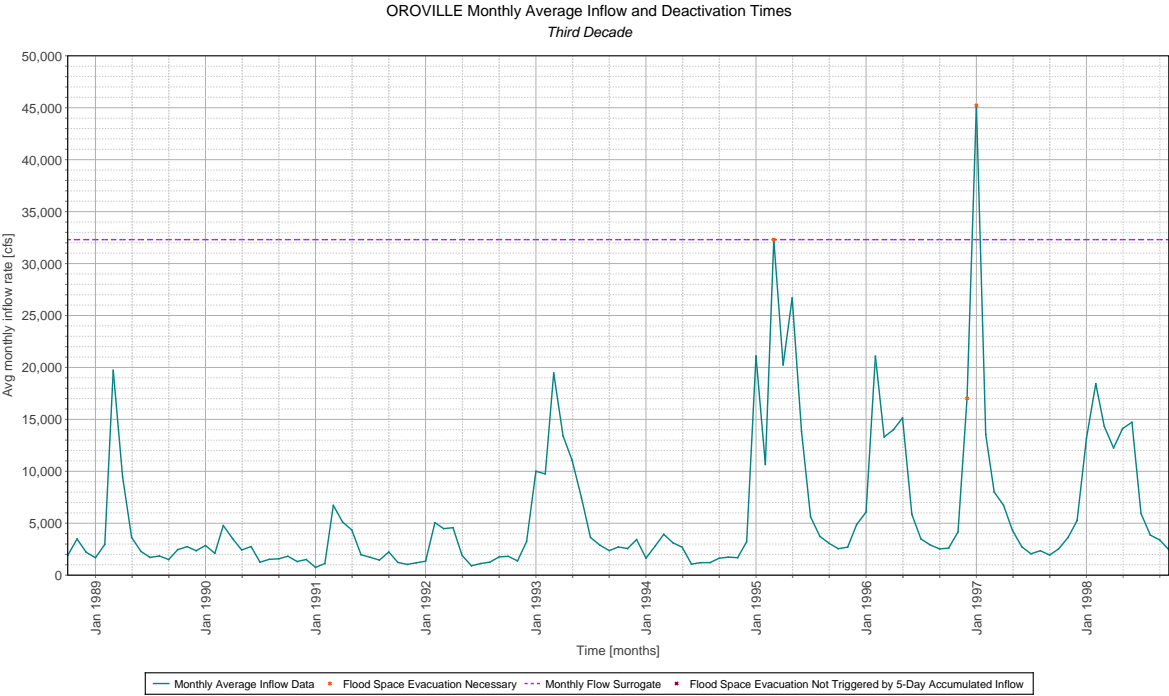
B: Monthly Effects Evaluation



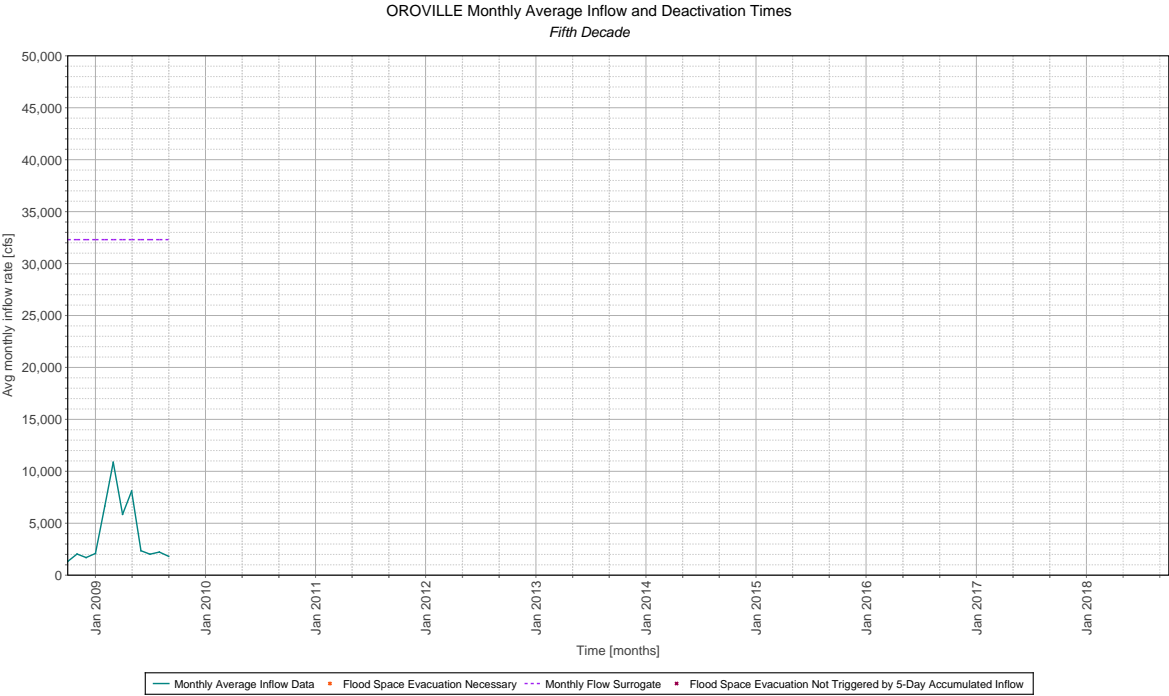
B: Monthly Effects Evaluation



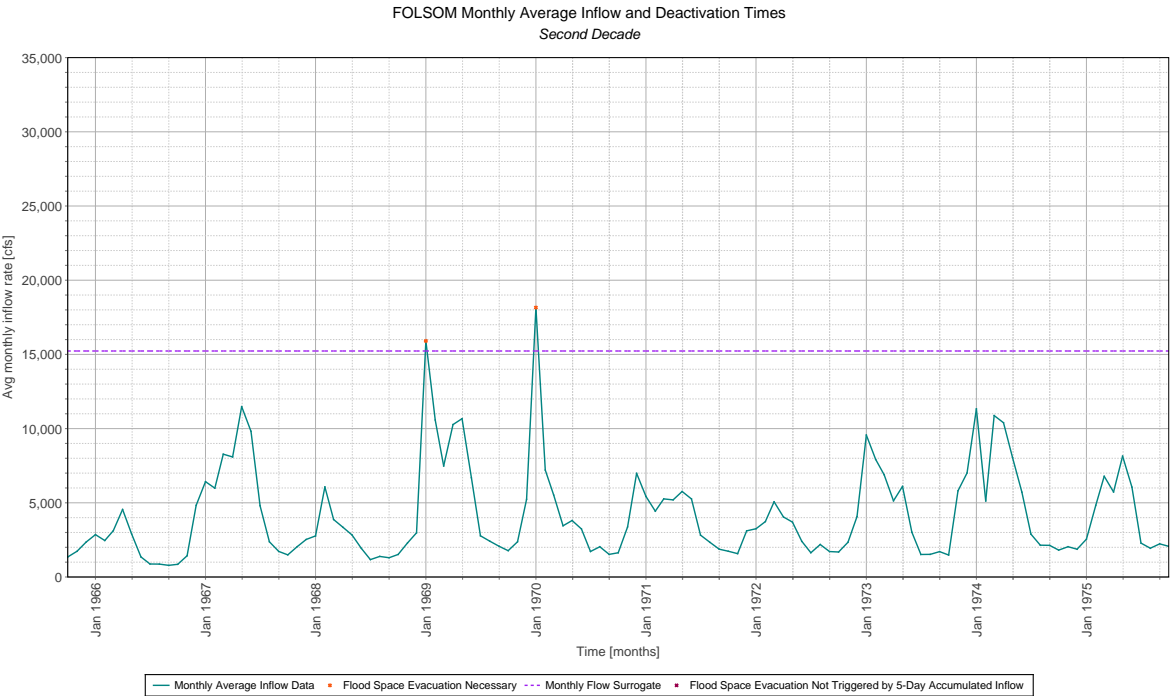
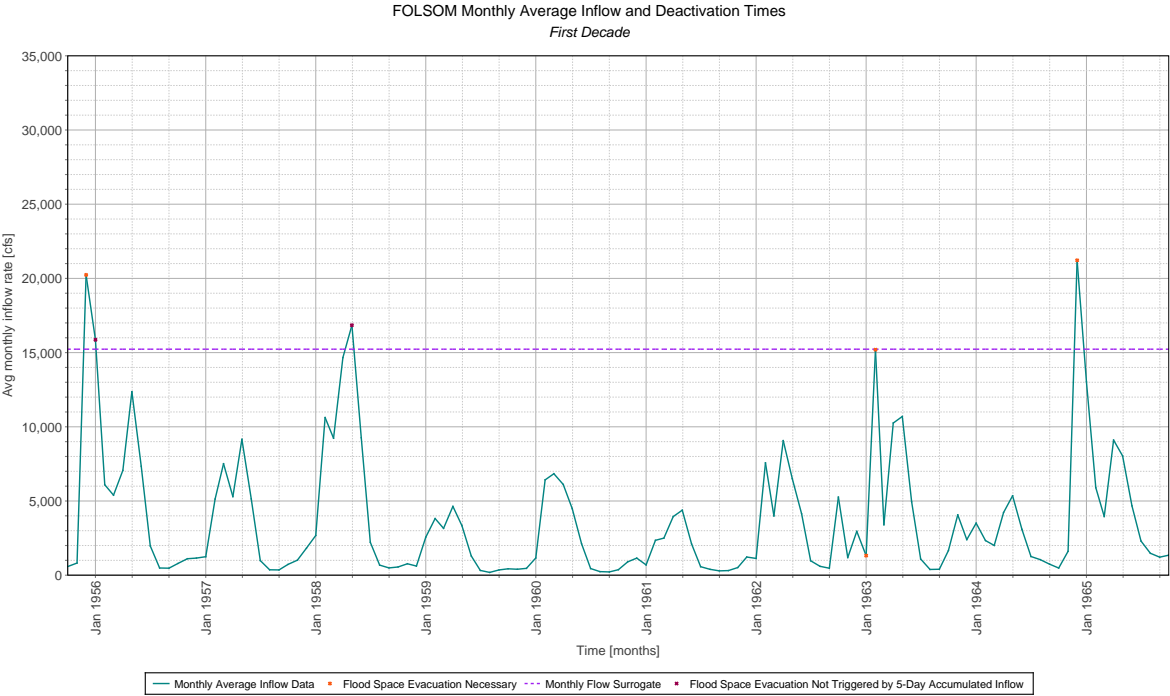
B: Monthly Effects Evaluation



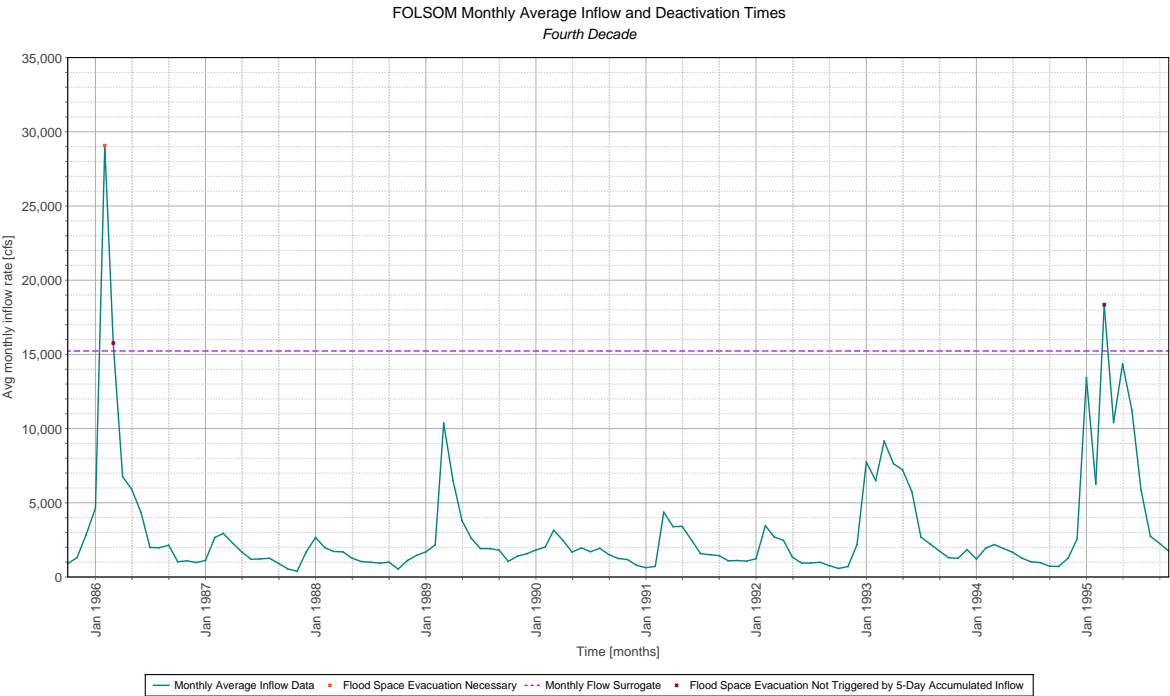
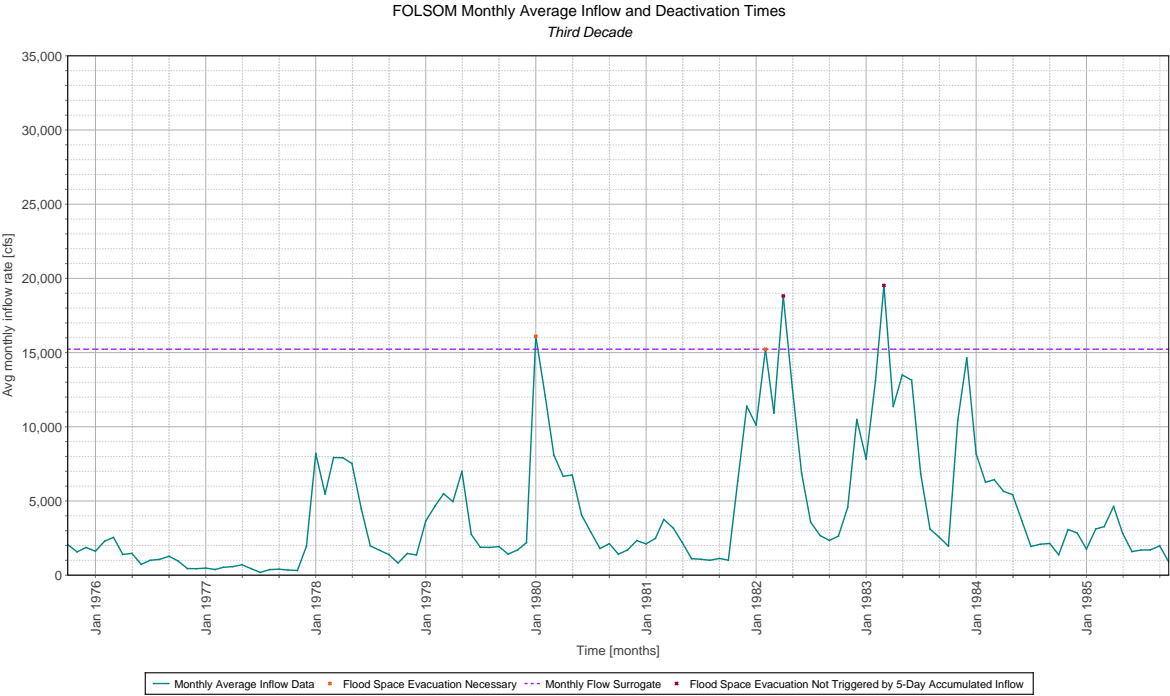
B: Monthly Effects Evaluation



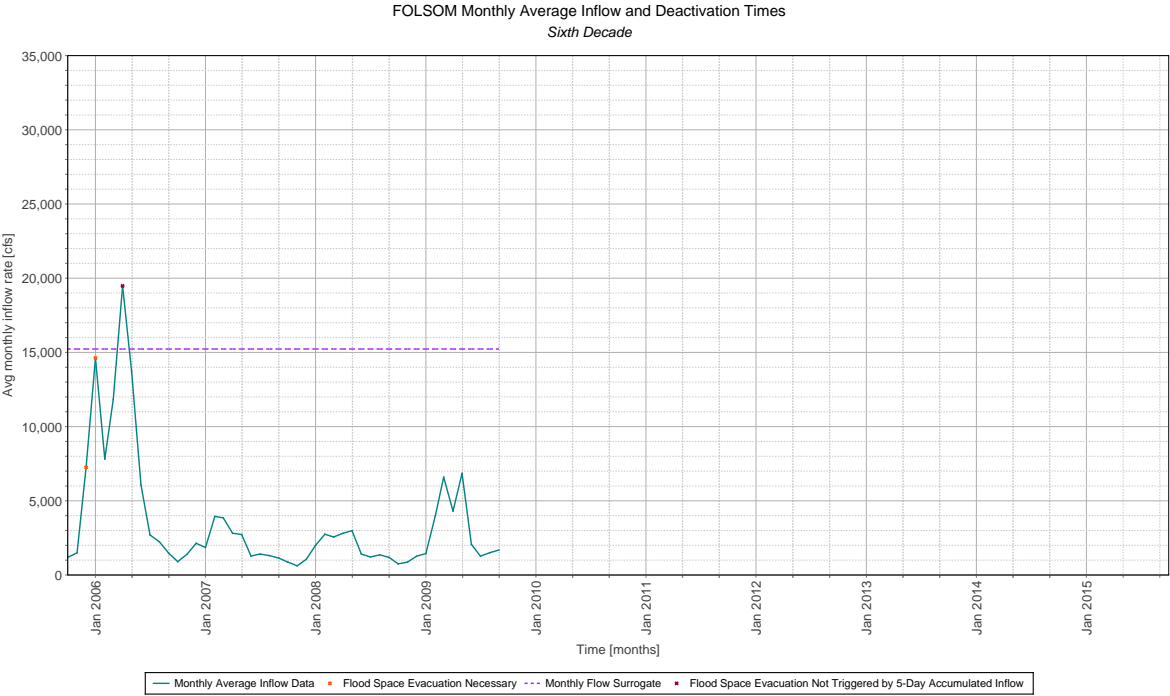
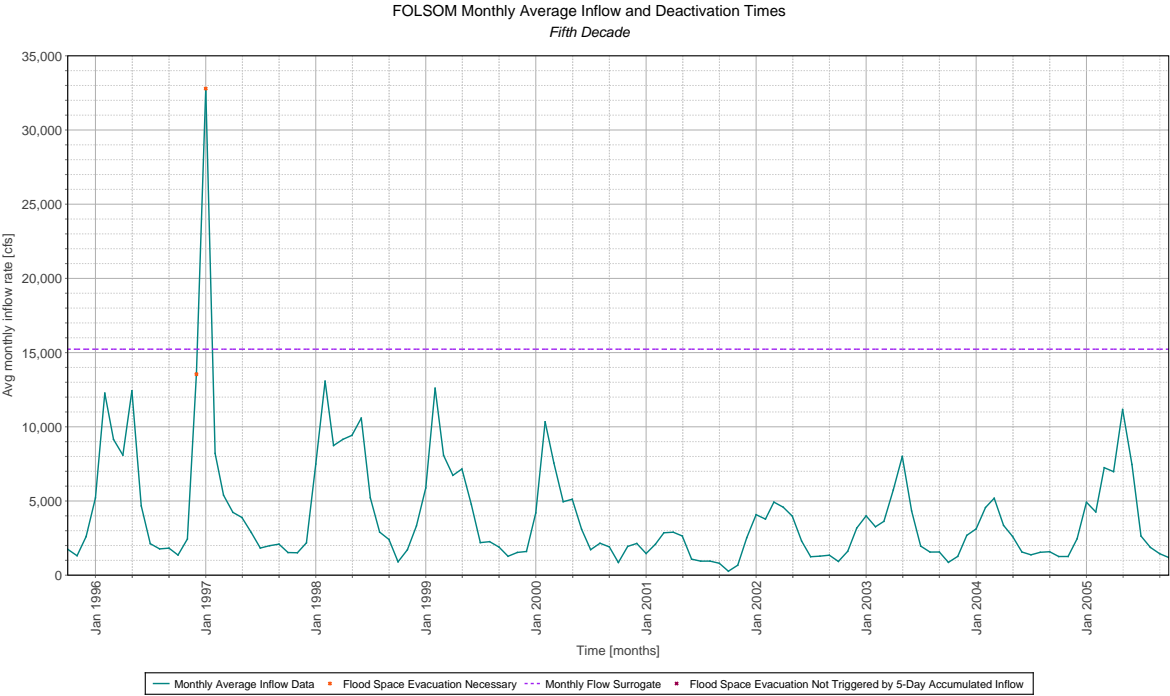
B: Monthly Effects Evaluation



B: Monthly Effects Evaluation



B: Monthly Effects Evaluation



B.2 Effects

The following section includes plots from the CalLite simulation which attempt to show the key effects of the reoperation.

Caryover Storages

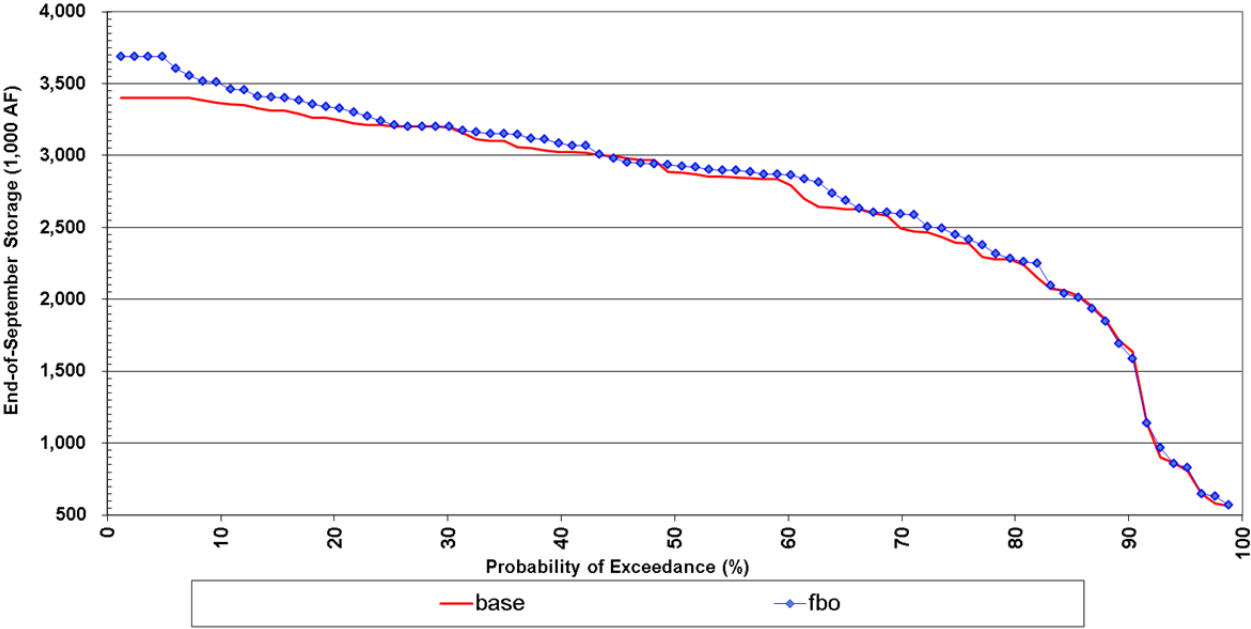


Figure 1. Shasta Reservoir End-of-September (FBO Scenario)

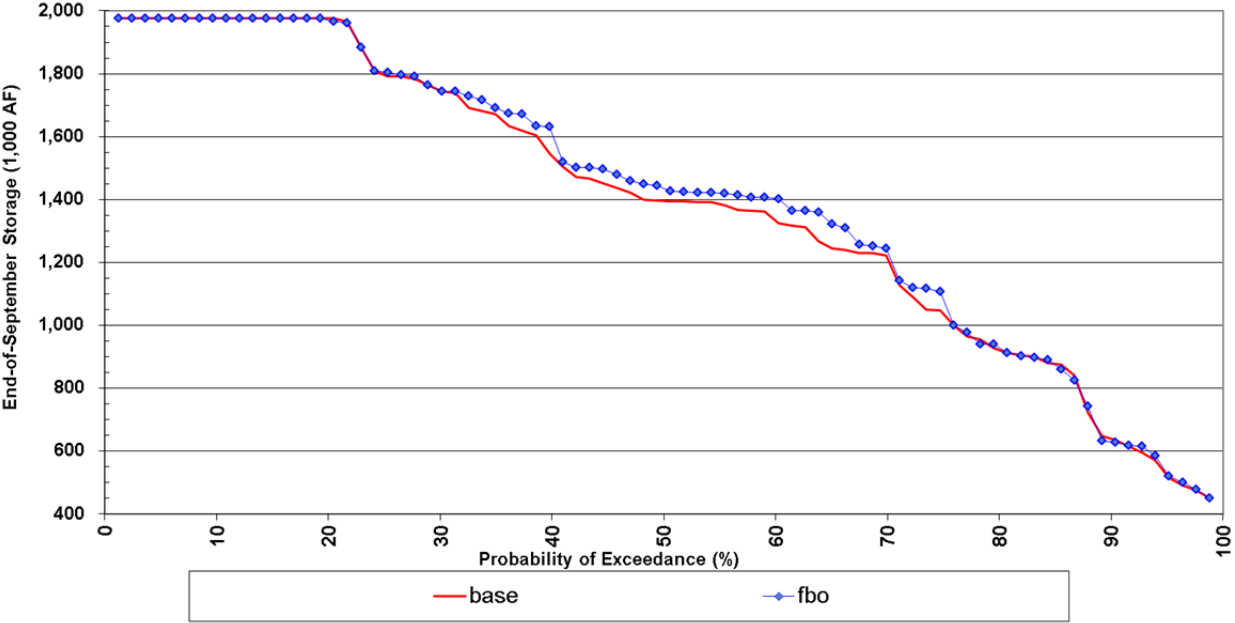


Figure 2. Trinity Reservoir End-of-September (FBO Scenario)

B: Monthly Effects Evaluation

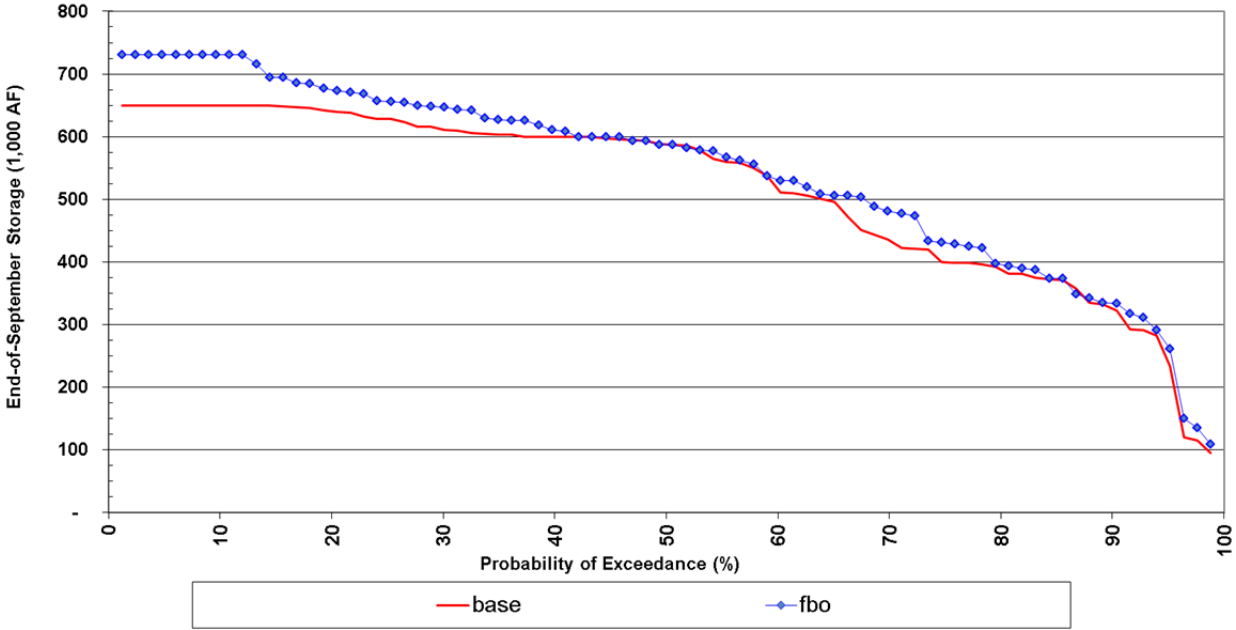


Figure 3. Folsom Reservoir End-of-September (FBO Scenario)

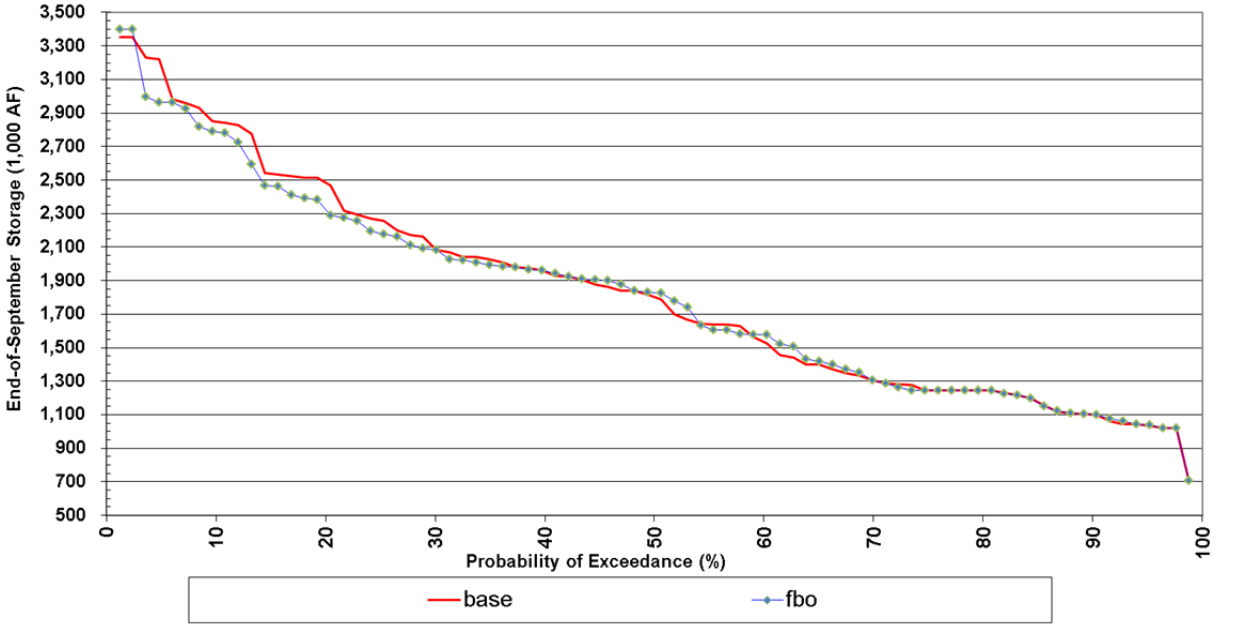


Figure 4. Oroville Reservoir End-of-September (FBO Scenario)

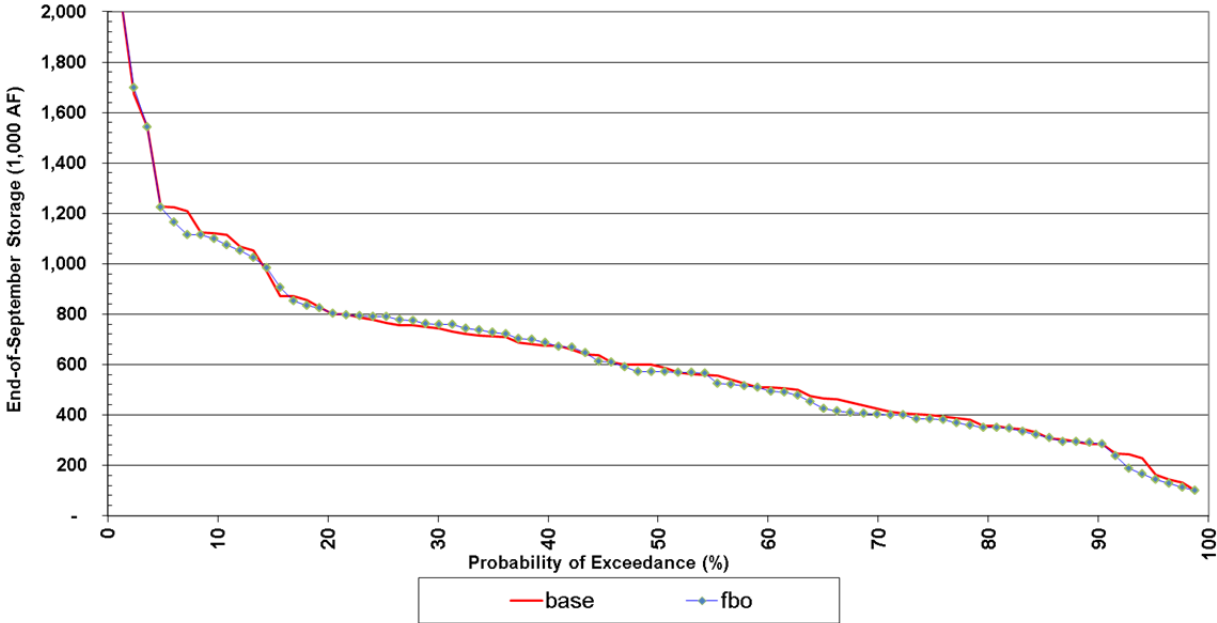


Figure 5. San Luis Reservoir End-of-September (FBO Scenario)

Shasta Flood Operations

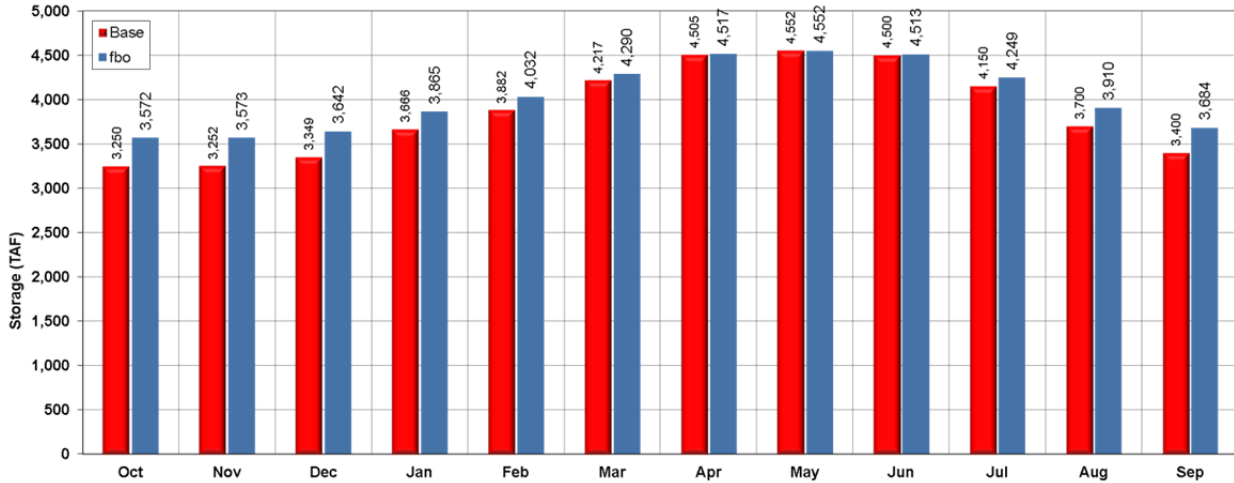


Figure 6. Shasta Top-of-Conservation Storage Level by Month

Upper Sacramento River System

B: Monthly Effects Evaluation

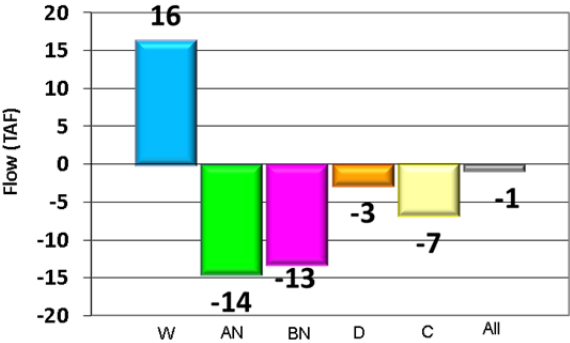


Figure 7. Changes in Average Annual Spring Creek Tunnel Diversions (Trinity Imports) by Water Year Type

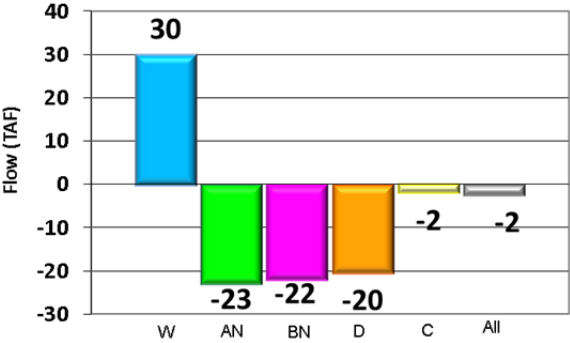


Figure 8. Changes in Average Annual Sacramento River Flow at Keswick by Water Year Type

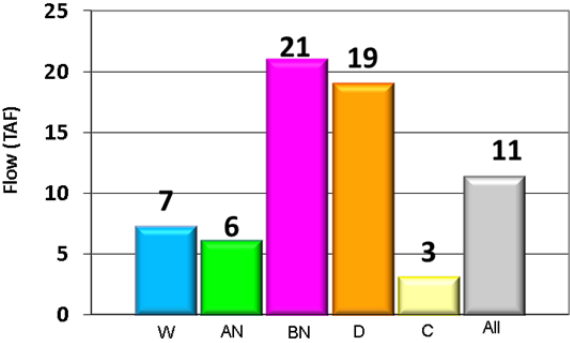


Figure 9. Changes in Average Annual TCCA Deliveries by Water Year Type

B: Monthly Effects Evaluation

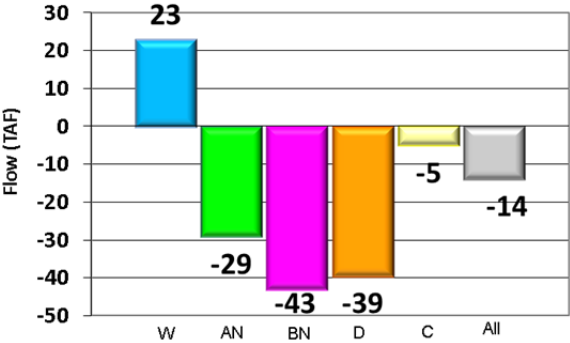


Figure 10. Changes in Average Annual Sacramento River Flow at Wilkins Slough by Water Year Type

Oroville Flood Operations

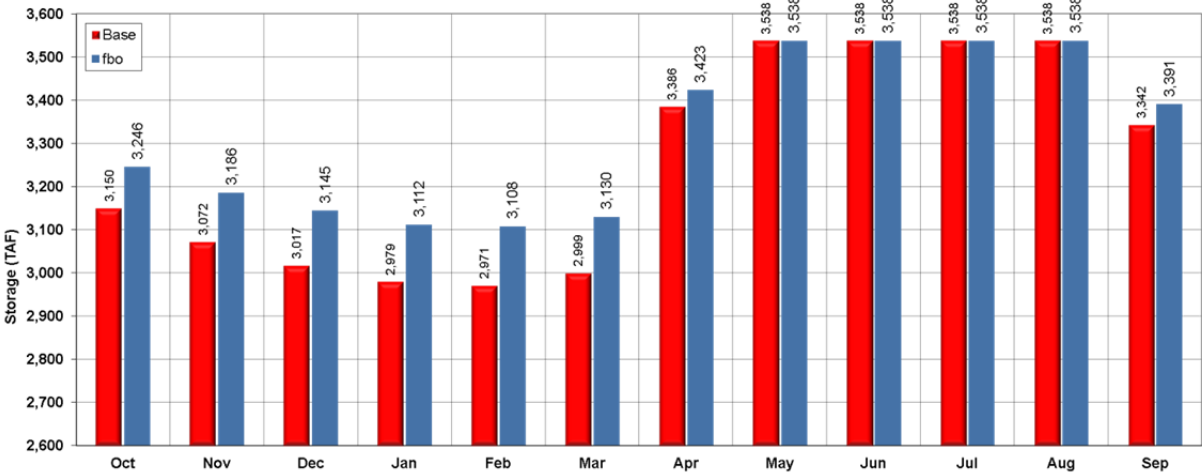


Figure 11. Oroville Top-of-Conservation Storage Level by Month

Feather River System

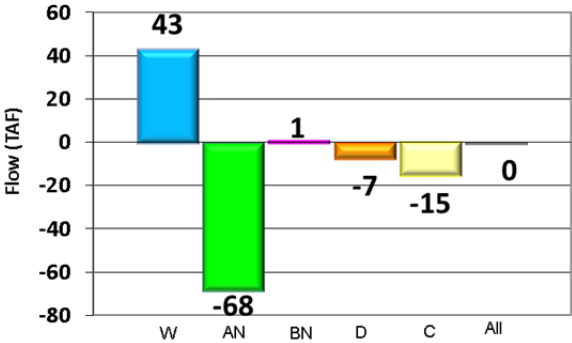


Figure 12. Changes in Average Annual Feather River Flow below Yuba River by Water Year Type

Folsom Flood Operations

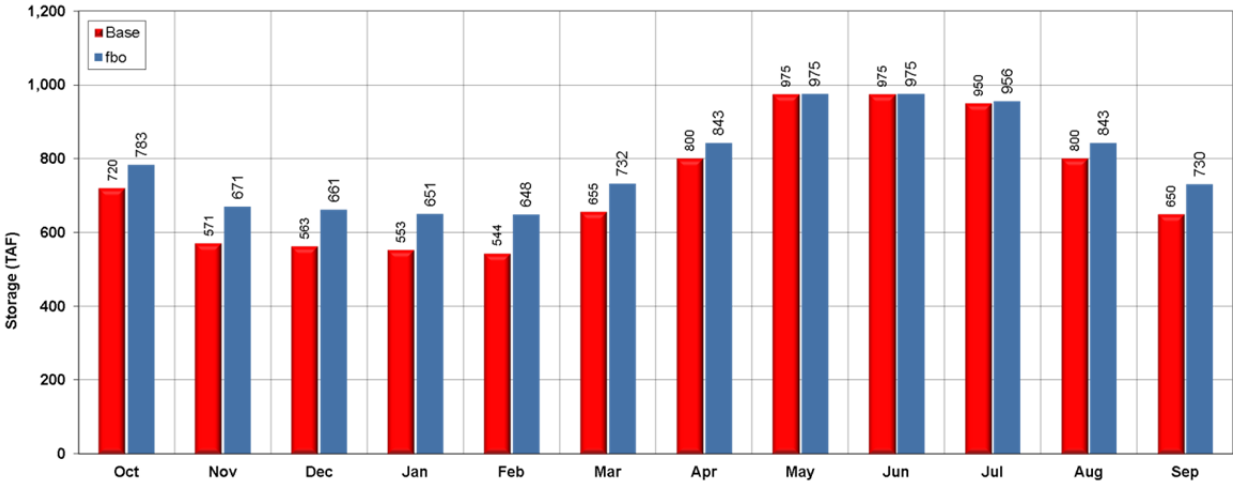


Figure 13. Folsom Top-of-Conservation Storage Level by Month

American River System

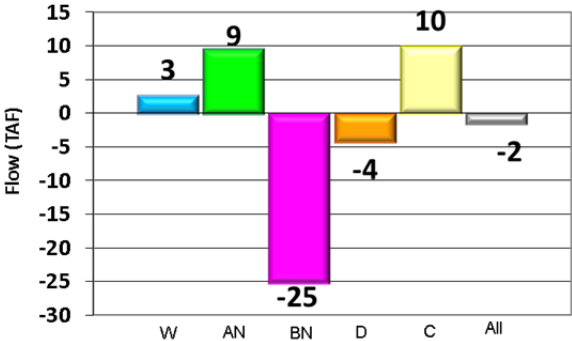


Figure 14. Changes in Average Annual American River Flow below Nimbus by Water Year Type

Lower Sacramento River System

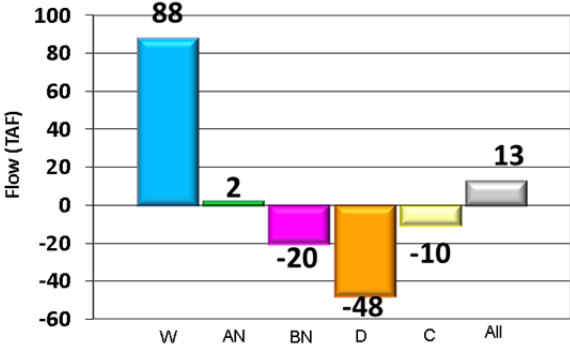


Figure 15. Changes in Average Annual Sacramento River Flow at Hood by Water Year Type

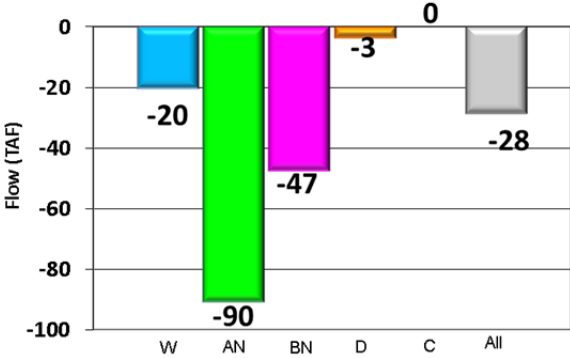


Figure 16. Changes in Average Annual Yolo Bypass Flows by Water Year Type

B: Monthly Effects Evaluation

Delta

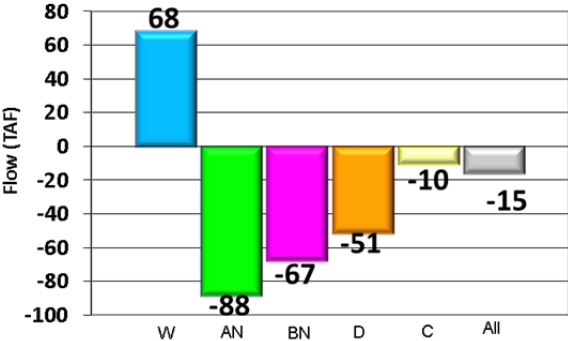


Figure 17. Changes in Average Annual Sacramento River at Hood and Yolo Bypass by Water Year Type

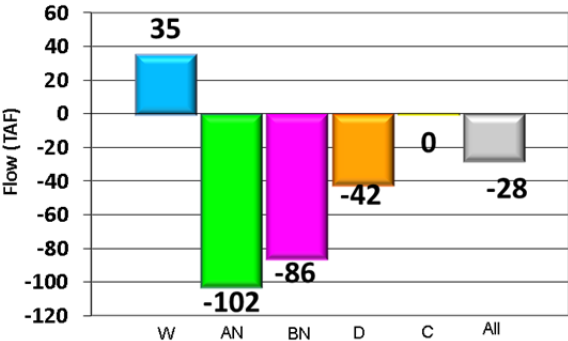


Figure 18. Changes in Average Annual Net Delta Outflow by Water Year Type

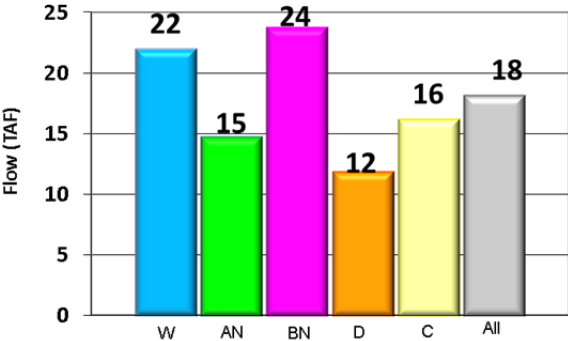


Figure 19. Changes in Average Annual CVP Exports by Water Year Type

B: Monthly Effects Evaluation

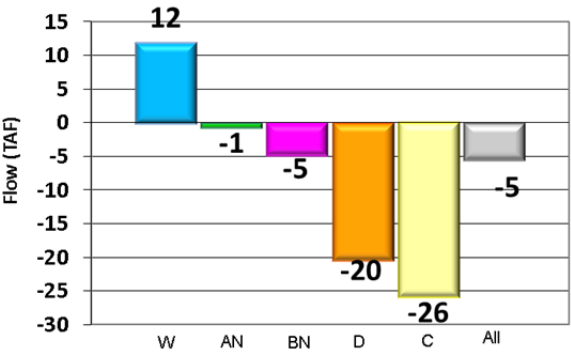


Figure 20. Changes in Average Annual SWP Exports by Water Year Type

Scenario	Change in Carryover Storage (TAF)				Average Annual Change in Flow (TAF)							
	Shasta EOS Storage	Trinity EOS Storage	Folsom EOS Storage	Oroville EOS Storage	Delta Inflow	Delta Outflow	Jones Pumping Plant	Banks Pumping Plant	NOD Ag Services Deliveries	North-of-Delta M&I	SWP Table A	Article 21
Base	2,680	1,396	517	1,836	17,913	15,802	2,252	2,630	233	85	2179	63
FBO-Base	60	20	28	-21	-16	-28	18	-5	11	1	-1	-3